

	WORK	POINT	SCHEDU	ILE	
WORK POINT	NORTHING	EASTING	STATION	OFFSET	ELEVATION
WP 1	6813599.524	593974.476	100+203.720	0.00	208.086
WP 2	6813638.714	593973.606	100+242.920	0.00	208.576
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Rev	Date	Description	Init
С	2024-03-25	ISSUED FOR 100% REVIEW	YL
В	2023-10-06	ISSUED FOR 80% REVIEW	YL
A	2023-06-19	ISSUED FOR 50% REVIEW	YL
		REVISIONS	

HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2

DESIGNEDY	L	DATE2024-03-25	_
CHECKEDJ	Z	DATE	_
DRAWNK	K	DATE 2024-03-25	_
SCALE	45.5	SHOWN	
	AD D	SHOWN	
		PREPARED UNDER THE DIRECTION OF	
		YING YI LI, P.ENG. ENGINEER OF RECORD	
		DATE 2024-03-25	
PROJECT No.	SHEET No.	DRAWING No.	
CE857700	23 OF 55	SC-INF01-6081-S001	С

GENERAL

READ THESE GENERAL NOTES IN CONJUNCTION WITH THE PROJECT SPECIFICATIONS, DRAWINGS, AND OTHER CONTRACT DOCUMENTS.

OBTAIN CLARIFICATION FROM THE ENGINEER IN THE EVENT THAT INFORMATION CONTAINED ELSEWHERE IN THE CONTRACT DOCUMENTS APPEARS TO CONFLICT WITH THESE GENERAL NOTES.

DO NOT SCALE DRAWINGS.

ALL DIMENSIONS ARE IN MILLIMETERS (mm OR MM) UNO.

ALL STATIONS, ELEVATIONS, EASTINGS, AND NORTHINGS ARE IN METERS (m OR M) UNO.

STATIONS, NORTHINGS, EASTINGS AND DIMENSIONS ARE GIVEN IN GRID COORDINATES AND VALUES UNO.

ANY UTILITY INFORMATION SHOWN ON THESE DRAWINGS CONCERNING TYPE AND LOCATION OF UNDERGROUND UTILITIES IS NOT GUARANTEED TO BE ACCURATE OR ALL-INCLUSIVE. THE CONTRACTOR SHALL DETERMINE THE TYPE AND LOCATION OF UNDERGROUND UTILITIES AND PROTECT AS REQUIRED. FOR RELOCATING EXISTING UNDERGROUND UTILITIES, REFER TO SPECIAL PROVISIONS.

ALL CONSTRUCTION SHALL CONFORM TO SPECIAL PROVISIONS, GOVERNMENT OF NORTHWEST TERRITORIES STANDARD SPECIFICATIONS FOR BRIDGE CONSTRUCTION (SSBC) (EDITION 1 2021), AND THESE DRAWINGS.

ALL CONSTRUCTION MATERIALS NOT SPECIFIED HEREIN AND IN THE DRAWINGS SHALL CONFORM TO THE REQUIREMENTS IN SSBC.

DESIGN CRITERIA & LOADING

DESIGN IS BASED ON:

- 1. CAN/CSA-S6-19 'CANADIAN HIGHWAY BRIDGE DESIGN CODE' (REFERRED TO HEREAFTER AS 'CSA S6').
- 2. DESIGN CRITERIA / STANDARDS BY GOVERNMENT OF NORTHWEST
- TERRITORIES (GNWT) WITH APPROVED DESIGN EXCEPTIONS (DE) 3. ALBERTA TRANSPORTATION BRIDGE STRUCTURES DESIGN CRITERIA, VERSION 9.0, JANUARY 2022

DESIGN LIVE LOAD: CL-800

BARRIERS DESIGNED FOR PERFORMANCE LEVEL: TL-4

CONSTRUCTION LIVE LOAD ON DECK = 1.5 kPa (NOT ON THE DECK CANTILEVER UNLESS BALANCED)

50-YEAR RETURN PERIOD MINIMUM AND MAXIMUM DAILY MEAN

TEMPERATURES = -48 AND 28 °C MINIMUM AND MAXIMUM EFFECTIVE TEMPERATURES = -46 AND 43 °C

LONGITUDINAL BRAKING FORCE = 228 kN

MEAN WIND PRESSURE FOR 50 YEAR RETURN PERIOD = 0.41 kPaMEAN WIND PRESSURE FOR 10 YEAR RETURN PERIOD (DURING CONSTRUCTION) = 0.30 kPa

DESIGN HORIZONTAL WIND PRESSURE = 1.64 kPa

DESIGN VERTICAL WIND PRESSURE = 0.82 kPa

SAFETY RAIL DESIGN LOAD: A CONCENTRATED LOAD OF 1.0 kN APPLIED AT ANY POINT SO AS TO PRODUCE THE MOST CRITICAL EFFECT. AS PER NATIONAL BUILDING CODE, CL 7.4.5.14 (B), APPLICABLE TO WHERE THE GATHERING OF MANY PEOPLE IS IMPROBABLE.

SEISMIC:

LIFELINE BRIDGE

SITE CLASS: C

SEISMIC PERFORMANCE CATEGORY: 1

S(0.2) = 0.169

SEISMIC CONNECTION HORIZONTAL LOAD AT ABUTMENTS: 455 kN, EACH DIRECTION

(TRANSVERSE DIRECTION RESTRAINED BY SHEAR BLOCKS: LONGITUDINAL DIRECTION RESTRAINED BY BACKFILL PASSIVE EARTH PRESSURE BEHIND ABUTMENT DIAPHRAGMS)

CAST-IN-PLACE CONCRETE

DRAIN TROUGHS AND SHEAR BLOCKS ON ABUTMENT SEATS: CLASS C, 28-DAY STRENGTH 35 MPa

PRECAST CONCRETE ELEMENT CONNECTIONS. POCKETS AND GIRDER HAUNCHES: APPROVED 120 MPa UHPC WITH 2% FIBER

ALL UHPC CASTING FORMWORK SHALL BE WATER-TIGHT TESTED PRIOR TO THE CONCRETE PLACEMENT.

PRECAST CONCRETE

DECK PANELS, DIAPHRAGMS AND APPROACH SLABS: CLASS HPC, 28-DAY STRENGTH 45 MPa

ALL OTHER PRECAST CONCRETE ELEMENTS: CLASS C, 28-DAY STRENGTH 35 MPa

REINFORCING STEEL SHALL HAVE 50 mm CLEAR CONCRETE COVER. EXCEPT THAT POCKET AND UHPC CONNECTION JOINT SURFACES TO HAVE MINIMUM 25 mm CLEAR CONCRETE COVER. UNLESS NOTED OTHERWISE.

ALL CORNERS (NOT INCLUDING CORNERS AT POCKET AND UHPC CONN SURFACES) SHALL BE CONSTRUCTED WITH A MINIMUM 20 mm CHAMF FILLET UNO.

THE CONTRACTOR IS RESPONSIBLE FOR ALL ASPECTS OF LIFTING (INC DESIGN OF LIFTING AND LEVELING DEVICES), HANDLING, ERECTION AND INSTALLATION OF PRECAST ELEMENTS AND TEMPORARY WORKS. THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER THE PLANS AND DRAW SIGNED AND SEALED BY A PROFESSIONAL ENGINEER IN THE NORTHWE TERRITORIES INDICATING ALL LOADS, AND ANY TEMPORARY SUPPORT S REQUIRED AT LEAST 2 WEEKS PRIOR TO THE FABRICATION. THE SUBM SHALL VERIFY THAT THE PRECAST ELEMENTS ARE CAPABLE OF RESIST ALL CONSTRUCTION LOADS SAFELY AND WITHOUT DAMAGE.

ALL JOINT AND POCKET SURFACES OF PRECAST ELEMENTS SHALL BE PRE-ROUGHENED (AT PLANT) BY ABRASIVE BLASTING TO ICRI CSP NO AS PER SSBC SECTION 7.2.5.15.

THE ESTIMATED MASS OF PRECAST CONCRETE ELEMENTS SHOWN IN T DRAWINGS ARE FOR THE GOVERNMENT OF NORTHWEST TERRITORIES US ONLY AND THE DEPARTMENT ASSUMES NO RESPONSIBILITY FOR ITS ACCURACY OR USE BY OTHERS.

REINFORCING STEEL

PLAIN REINFORCING STEEL SHALL CONFORM TO CAN/CSA-G30.18-M. 400W.

STAINLESS STEEL SHALL CONFORM TO THE REQUIREMENTS OF ASTM A AND A955/A955M (INCLUDING ANNEXES) WITH MINIMUM YIELD STRENG 420 MPa.

BAR MARK SUFFIX SS DENOTES SOLID STAINLESS STEEL BARS.

WELDING OF REINFORCEMENT IS NOT PERMITTED WITHOUT THE WRITTEN APPROVAL FROM THE ENGINEER.

STRUCTURAL STEEL & MISC METAL

PILES: STEEL SHALL CONFORM TO CAN/CSA-G40.21 GRADE 350W.

ALL STEEL FOR GIRDERS. STIFFENERS, SPLICE PLATES, AND ALL MATE WELDED TO THE GIRDERS, INCLUDING PLATES EMBEDDED IN PRECAST CONCRETE ABUTMENT DIAPHRAGM FOR GIRDER CONNECTIONS. SHALL CONFORM TO CSA G40.21M-350AT, CATEGORY 3, WITH CHARPY V-NOT STRENGTH OF 27 JOULES AT -45°C.

ALL OTHER STEEL INCLUDING DIAPHRAGM ELEMENTS AND LATERAL BRA SHALL CONFORM TO CSA G40.21M-350A.

SHEAR STUDS: ASTM STANDARD A108, GRADES 1015, 1018, OR 1020, AND CONFC TO AWS D1.5 TABLE 7.1 FOR TYPE B STUDS

BOLTS:

STRUCTURAL BOLTS TO WEATHERING STEEL APPLICATIONS SHALL BE A325M - TYPE 3; STRUCTURAL BOLTS TO GALVANIZED STEEL APPLICATIONS SHALL BE - TYPE 1 GALVANIZED IN ACCORDANCE WITH ASTM F2329

NUTS AND WASHERS FOR GIRDERS: MARKED WITH A "3" TO DENOTE WEATHERING GRADE

USE TWO WASHERS FOR ALL CONNECTIONS.

BRIDGERAIL AND APPROACH RAIL TRANSITION MATERIALS SHALL BE IN ACCORDANCE WITH ALBERTA TRANSPORTATION STANDARD DRAWING S-1642-20.

ALL MATERIALS AND CONNECTIONS ARE DESIGNED BASED ON METRIC CHANGING TO IMPERIAL UNITS SHALL NOT BE DONE WITHOUT APPROVA FROM THE ENGINEER. IMPERIAL SIZE COMPONENTS LARGER THAN METRI SPECIFIED SHALL NOT BE AN EXTRA COST TO THE CONTRACT.

GALVANIZED COMPONENTS:

ALL COMPONENTS, EXCEPT WEATHERING STEEL, SHALL BE HOT DIP GALVANIZED. IN ACCORDANCE WITH ASTM A123M WITH A MINIMUM NE RETENTION OF 600 q/m^2 , UNO.

GRIND ALL WELDS SMOOTH BEFORE GALVANIZING.

FIELD WELDS AND DAMAGED AREAS OF GALVANIZING SHALL BE METALLIZED IN ACCORDANCE WITH SSBC SECTION 6.2.7.3.3

EMBEDDED GALVANIZED STEEL ELEMENTS IN CONTACT WITH PLAIN OR STAINLESS REINFORCING STEEL SHALL BE SEPARATED BY PLASTIC ISOLATOR CLIPS OR APPROVED EQUIVALENT. WEATHERING STEEL COMPONENTS IN CONTACT WITH GALVANIZED ELEMENTS SHALL BE ISOLATED USING AN APPROVED COATING.

SEE SPECIFICATIONS AND THE DRAWINGS FOR FABRICATION AND INSTALLATION DETAILS, AND MATERIALS NOT SPECIFIED ABOVE.

STEEL GIRDERS

ALL BOLTED CONNECTIONS SHALL BE MADE WITH 22 mm DIAMETER UNO. BOLTED CONNECTIONS SHALL BE DETAILED WITH THREAD EXCLUDED FROM THE SHEAR PLANE.

THE ESTIMATED MASS OF THE STEEL GIRDER (GIRDERS AND DIAPHRAGMS BUT NOT BEARINGS, STUDS, BOLTS, ETC.) IS 23 TONNES. THIS ESTIMATE IS FOR THE GOVERNMENT OF NORTHWEST TERRITORIES USE ONLY AND THE DEPARTMENT ASSUMES NO RESPONSIBILITY FOR ITS ACCURACY OR USE BY OTHERS.

INECTION FER OR	DESIGN	G
CLUDING D	DISTRIBUTION FACTORS SHEAR (ULS, SLS) = 0.647 BENDING (ULS, SLS) = 0.578 SHEAR (FLS) = 0.611	D/
VINGS, EST SYSTEM MISSION TING	BENDING (FLS)= 0.398 DEAD LOAD:= $6.66 \text{ kN/m} (\text{MID-SPAN}) / 6.26 \text{ kN/m} (END SPAN) PER GIRDERDECK AND HAUNCH=18.3 \text{ kN/m} PER GIRDER$	TO 20 EI
-	FATIGUE: ACCORDING TO CAN/CSA-S6-19, HIGHWAY CLASS A	E
0.6,	ASSUMED CONSTRUCTION LOADS = 1.5 kPa	BI
THE JSE	THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW CONSTRUCTION PLANS, SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE NORTHWEST TERRITORIES INDICATING ALL LOADS, PROPOSED METHODS AND SEQUENCES OF CONSTRUCTION, AND ANY TEMPORARY SUPPORT SYSTEMS REQUIRED. THE SUBMISSION SHALL VERIFY THAT THE GIRDERS ARE CAPABLE OF RESISTING THE ACTUAL LOADS SAFELY AND WITHOUT DAMAGE.	AS DI TO OI RI AI
GRADE	FABRICATION	AI El F(
A276	FABRICATION AND ERECTION SHALL BE IN ACCORDANCE WITH SSBC.	GI G
GTH OF	ALL GIRDER DIMENSIONS ARE CORRECT AT 20°C AND GIRDER LENGTHS ARE MEASURED ALONG THE BOTTOM FLANGE.	_
	ALL WELDING, CUTTING AND PREPARATION SHALL BE IN ACCORDANCE WITH THE AWS BRIDGE WELDING CODE, D1.5.	_
EN	BEARING STIFFENERS UNDER FULL BRIDGE DEAD LOAD SHALL BE VERTICAL. INTERMEDIATE WEB STIFFENERS AND DIAPHRAGM CONNECTION STIFFENERS SHALL BE INSTALLED PERPENDICULAR TO THE GIRDER FLANGES.	-
	THE CONTRACTOR SHALL SUBMIT WELD SIZES AND WELDING PROCEDURE TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION.	_
ERIALS	ALL WELD METAL DEPOSITS SHALL HAVE CHARPY V NOTCH IMPACT STRENGTH OF AT LEAST 27 JOULES AT -45° C. ALL WELD METAL DEPOSITS SHALL BE SUBJECT TO THE APPROVAL OF THE ENGINEER AND SHALL PRODUCE BOTH THE CORROSION RESISTANCE AND THE COLOUR PROPERTIES OF THE BASE METAL.	_
RACINGS	ALL BOLT HOLES SHALL BE DRILLED 2 mm LARGER THAN THE SPECIFIED BOLT DIAMETER UNLESS NOTED OTHERWISE.	_
ORMING	TIGHTENING OF ALL HIGH STRENGTH BOLTS SHALL BE DONE BY THE TURN OF NUT METHOD ONLY IN ACCORDANCE WITH SSBC. BEFORE FINAL TIGHTENING THERE SHALL BE A SUFFICIENT NUMBER OF BOLTS BROUGHT TO A SNUG TIGHT CONDITION TO ENSURE THAT PARTS OF THE JOINT ARE BROUGHT INTO FULL CONTACT WITH EACH OTHER.	_
E ASTM	GIRDERS SHALL MEET THE CAMBER REQUIREMENTS AS SHOWN ON GIRDER	_
E A325M	CAMBER DIAGRAM. ALL STEEL SHALL BE BLAST CLEANED AFTER FABRICATION IN ACCORDANCE WITH SSBC SECTION 6.2.7.1	_ _
	TEMPORARY SUPPORT	
	THE CONTRACTOR SHALL INSTALL TEMPORARY SUPPORTING STRUCTURES TO MAINTAIN THE GIRDER'S STABILITY SOON AFTER THE GIRDERS ARE ERECTED.	_
UNITS. /AL IRIC	GIRDERS SHALL BE TEMPORARILY RESTRAINED SOON AFTER BEING PLACED ON THE BEARINGS AND PRIOR TO RELEASING THE LIFTING DEVICE TO AVOID SLIDING. THE RESTRAINING SYSTEM SHALL REMAIN IN PLACE UNTIL BACKFILL WITH COMPACTION BEHIND ABUTMENT DIAPHRAGMS IS MINIMUM 1.2 m ABOVE THE BOTTOM OF THE DIAPHRAGMS TO RETAIN THE SLIDING FORCE.	-
	INSPECTION & NON-DESTRUCTIVE TESTING	_
NET	WELD INSPECTION AND TESTING SHALL BE CARRIED OUT IN ACCORDANCE WITH SSBC.	IN C(D I
	BRIDGE ELEMENTS LIFTING & ERECTION	PI A(
TALLIZED	THE CONTRACTOR SHALL SUBMIT GIRDER AND PRECAST CONCRETE ELEMENT	

THE CONTRACTOR SHALL SUBMIT GIRDER AND PRECAST CONCRETE ELEMENT LIFTING AND ERECTION PROCEDURES TO THE ENGINEER FOR REVIEW AT LEAST 2 WEEKS PRIOR TO THE WORKS AS PER THE CONTRACT SPECIAL PROVISION AND SSBC.

THE LIFTING AND ERECTION PROCEDURES FOR ALL BRIDGE ELEMENTS SHALL BE DESIGNED. STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE NORTHWEST TERRITORIES.

HYDROTECHNICAL DATA

HYDROTECHNICAL DESIGN REPORT PREPARED BY MATRIX SOLU DATED JANUARY 16, 2024.

DESIGN FLOOD RETURN PERIOD: 1:100 YEAR DESIGN FLOW: 210 m^3/s DESIGN WATER LEVEL: 205.10 m DESIGN VELOCITY: 2.1 m/s

GEOTECHNICAL

GEOTECHNICAL REPORT WAS COMPLETED BY MASKWA ENGINEE DATED OCTOBER 30, 2023.

SURVEY

FOPOGRAPHICAL SURVEY WAS COMPLETED BY STANTEC CONSU 2019. SUPPLEMENTAL TOPOGRAPHICAL SURVEY WAS COMPLET ENGINEERING LTD. IN JUNE 2023.

BRIDGE CONSTRUCTION SEQUEN

BELOW IS A GENERAL DESCRIPTION OF THE CONSTRUCTION ASSUMED FOR THE PURPOSE OF THE DESIGN OF THE WORKS DETAILED WORK PLAN AND HIGHLIGHTS ONLY CERTAIN ACTIVIT TO COMPLETE THE WORK. SPECIFIC WORK PLANNING AND DE CONSTRUCTION EXECUTION AND SCHEDULING ARE THE SOLE I OF THE CONTRACTOR AND THE CONTRACTOR'S PLAN IS SUBJ REVIEW AND ACCEPTANCE OF THE ENGINEER AS WELL AS OTH AUTHORITIES HAVING JURISDICTION.

AN ALTERNATE SEQUENCE PLAN WILL BE GIVEN CONSIDERATIO ENGINEER SO LONG AS THE CONTRACTOR SUBMITS A COMPLE FOR REVIEW. SUCH AN ALTERNATIVE SEQUENCE PLAN MAY OR GIVEN ACCEPTANCE BY THE ENGINEER AND THE ENGINEER'S GOVERN.

- SURVEY EXISTING BRIDGE PILE ROW LOCATIONS AS PER D AND S006 (SHEET 27 AND 28);
- BUILD DETOUR ROAD AND TEMPORARY BRIDGE;
- REMOVE THE EXISTING BRIDGE;
- INSTALL PILES;
- INSTALL ABUTMENT SEATS AND UHPC CONNECTION JOINTS
- INSTALL SLEEPER BEAM (FLEXIBLE STAGE AS DETERMINED BY THE CONTRACTOR);
- INSTALL HEAD SLOPE AND RIPRAP (FLEXIBLE STAGE AS DETERMINED BY THE CONTRACTOR):
- INSTALL BEARINGS, BUT DO NOT GROUT THE BEARING POCKETS;
- INSTALL GIRDERS AND PROVIDE GIRDER TEMPORARY STABILITY SUPPORTS. ALSO INCLUDING THE SUPPORT SYSTEM TO PREVENT GIRDER LONGITUDINAL SLIDING. THE LONGITUDINAL SUPPORT SYSTEM TO STAY IN PLACE UNTIL BACKFILL BEHIND ABUTMENTS IS COMPLETED;
- INSTALL GIRDER INTERMEDIATE DIAPHRAGMS:
- GROUT BEARING POCKETS:
- INSTALL PRECAST CONCRETE ABUTMENT DIAPHRAGMS:
- INSTALL PRECAST CONCRETE WINGWALLS WITH TEMPORARY SUPPORTS (LATERALLY AND VERTICALLY) AND UHPC CONNECTION JOINTS;
- BACKFILL BEHIND ABUTMENTS UP TO 1.2 m ABOVE BOTTOM OF ABUTMENT DIAPHRAGMS WITH BALANCED METHOD SPECIFIED IN THE DRAWINGS:
- INSTALL PRECAST CONCRETE DECK PANELS AND UHPC CONNECTION JOINTS:
- COMPLETE BACKFILL AND INSTALL APPROACH SLAB ELEMENTS AND UHPC CONNECTION JOINTS;
- INSTALL BRIDGERAILS AND APPROACH RAIL POSTS:
- INSTALL CIP CONCRETE DRAIN TROUGHS

IN ADDITION TO SUBMITTING EXISTING BRIDGE PILE ROW SURVEY DATA. THE CONTRACTOR SHALL HAVE THE BEARINGS DESIGNED AND SUBMIT THE SHOP DRAWINGS AT AN EARLIEST POSSIBLE TIME FOR THE ENGINEER'S REVIEW PRIOR TO THE FABRICATION OF BRIDGE ELEMENTS THAT ARE RELATED TO ACTUAL BEARING ASSEMBLY SIZES.

LEGEND & ABBREVIATIONS

UTIONS INC.	BH	BOREHOLE	GALV	GALVANIZED
	Ę	CENTRELINE	GALV GALVANIZED HPC HIGH PERFORMANCE CONCORNANCE CONCORNET NT HORIZ HORIZONTAL HWY HIGHWAY HIGHWAY INT INTERIOR INTERIOR LG LONG MIN MIN MINIMUM MISC MISC MISCELLANEOUS MIN NOM NOMINAL NTS NTS NOT TO SCALE PL PL PLATE PROJ PROJ PROJECTION QTY QUANTITY SIM SIMILAR SS STAINLESS STEEL BAR STA STA STATION TYP TYP TYPICAL UHPC ULTRA HIGH PERFORMANCE CONCRETE UNO UNLESS NOTED OTHERWISH VERT VERTICAL WSC WATER SURVEY OF CANAD MATER SURVEY OF CANAD	HIGH PERFORMANCE CONCRETE
		SURVEY CONTROL POINT	HORIZ	HORIZONTAL
	Ţ	WATER LEVEL	HWY	HIGHWAY
	◆ ^{WP}	WORK POINT	INT	INTERIOR
ERING LTD.,	ABUT	ABUTMENT	LG	LONG
	AIFB	ASPHALT IMPREGNATED FIBREBOARD	MIN	МІЛІМИМ
	APP	APPROACH	MISC	MISCELLANEOUS
SULTING LTD. IN TED BY MASKWA	B/W	BETWEEN	NOM	NOMINAL
	BRG	BEARING	NTS	NOT TO SCALE
NCE	BOTT	воттом	PL	PLATE
SEQUENCE	C/W	COMPLETE WITH	PROJ	PROJECTION
KS. IT IS NOT A TIES REQUIRED ETAILS OF THE	CIP	CAST-IN-PLACE	QTY	QUANTITY
RESPONSIBILITY JECT TO THE	ℓ CENTRELINE HPC HIGH PERFORMANCE CONCRETE ▲ SURVEY CONTROL POINT HORIZ HORIZONTAL ↓ WATER LEVEL HWY HIGHWAY ↓ WORK POINT INT INTERIOR ABUT ABUTMENT LG LONG AFB ASPHALT IMPREGNATED FIBREBOARD MIN MINIMUM APP APPROACH MISC MISCELLANEOUS INA B/W BETWEEN NOM NOMINAL BRG BEARING NTS NOT TO SCALE OTO COMPLETE WITH PROJ PROJECTION INA CIP CAST-IN-PLACE QTY QUANTITY CIP CAST-IN-PLACE QTY QUANTITY CLR CLEAR SS STAINLESS STEEL BAR CLR CLEAR STA STAINLESS STEEL BAR DIAM DIAMETER TYP TYPICAL ON DIAMETER TYP TYPICAL DIA DIAMETER UHPC ULTRA HIGH PERFOR			
THER	●BHBOREHOLEGALVGALVANIZED€CENTRELINEHPCHIGH PERFORMANCE CONCRETE▲SURVEY CONTROL POINTHORIZHORIZONTAL↓WATER LEVELHWYHIGHWAY↓WORK POINTINTINTERIORABUTABUTMENTLGLONGAIFBASPHALT IMPREGNATEDMINMINIMUMAPPAPPROACHMISCMISCELLANEOUSB/WBETWEENNOMNOMINALBRGBEARINGNTSNOT TO SCALEBOTTBOTTOMPLPLATEC/WCOMPLETE WITHPROJPROJECTIONCIPCAST-IN-PLACEQTYQUANTITYCJCONSTRUCTION JOINTSIMSIMILARCLRCLEARSSSTAINLESS STEEL BARCONTCONTINUOUSSTASTATIONCSPCORRUGATED STEEL PIPESYMSYMMETRICALDIAMDIAMETERTYPTYPICALDIAPHRAGMUHPCULTRA HICH PERFORMANCEELELEVATIONUNOUNLESS NOTED OTHERWISEEXEXISTINGVERTVERTICALEXEXTERIORWERTVERTICAL			
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R MAY NOT BE RULING SHALL	CSP	CORRUGATED STEEL PIPE	SYM	SYMMETRICAL
	DIA	DIAMETER	TYP	TYPICAL
DRAWING S005	DIAPH	DIAPHRAGM	UHPC	
	EL	ELEVATION	UNO	UNLESS NOTED OTHERWISE
	EX	EXISTING	VERT	VERTICAL
0	EXT	EXTERIOR	WSC	WATER SURVEY OF CANADA
S;	FO	FIBRE OPTIC CABLE (BURIED)	WW	WINGWALL

Consultant Logo

A 2023-06-19





Rev	Date	Description	Init
С	2024-03-25	ISSUED FOR 100% REVIEW	YL
В	2023-10-06	ISSUED FOR 80% REVIEW	YL

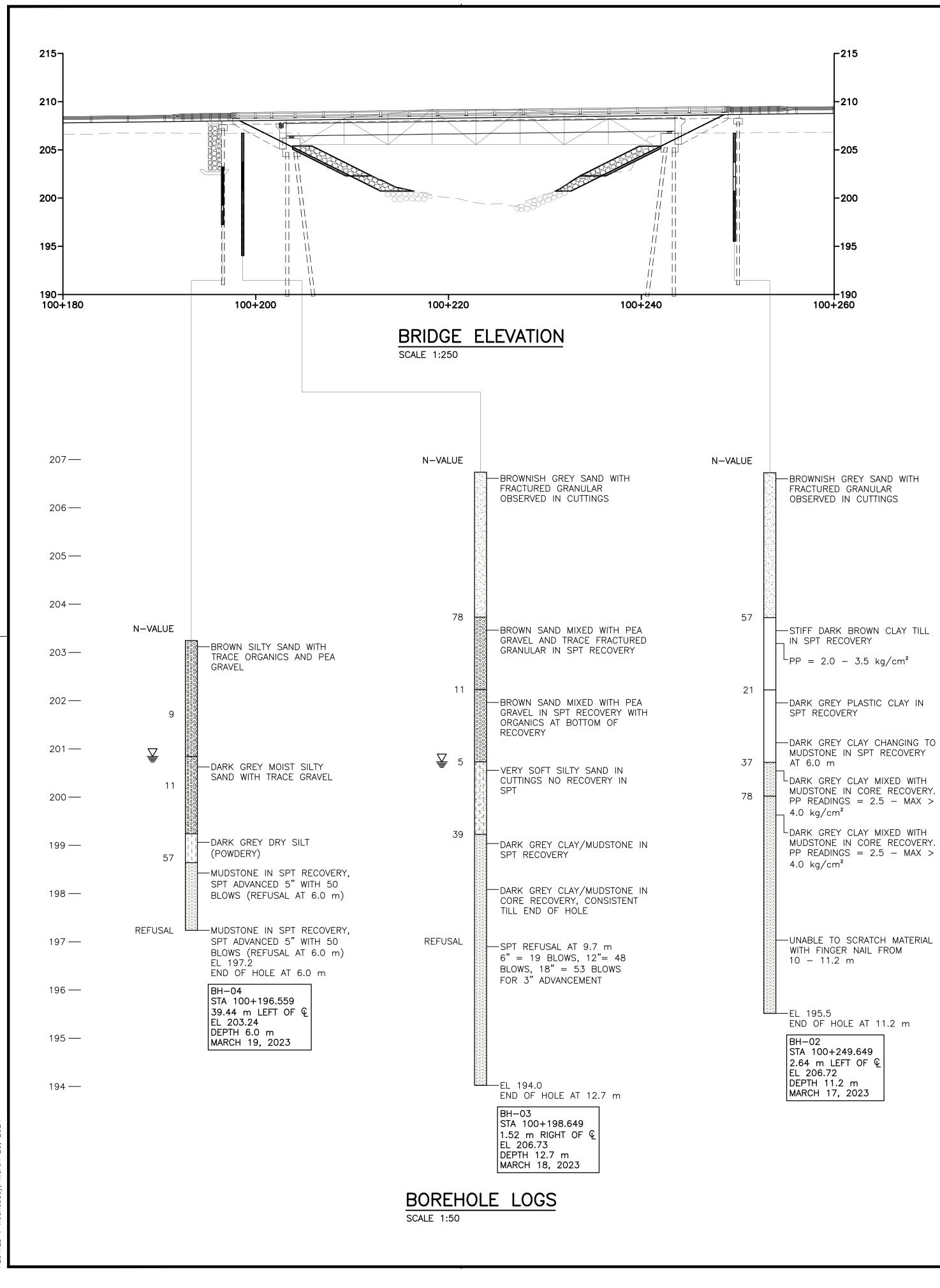
ISSUED FOR 50% REVIEW

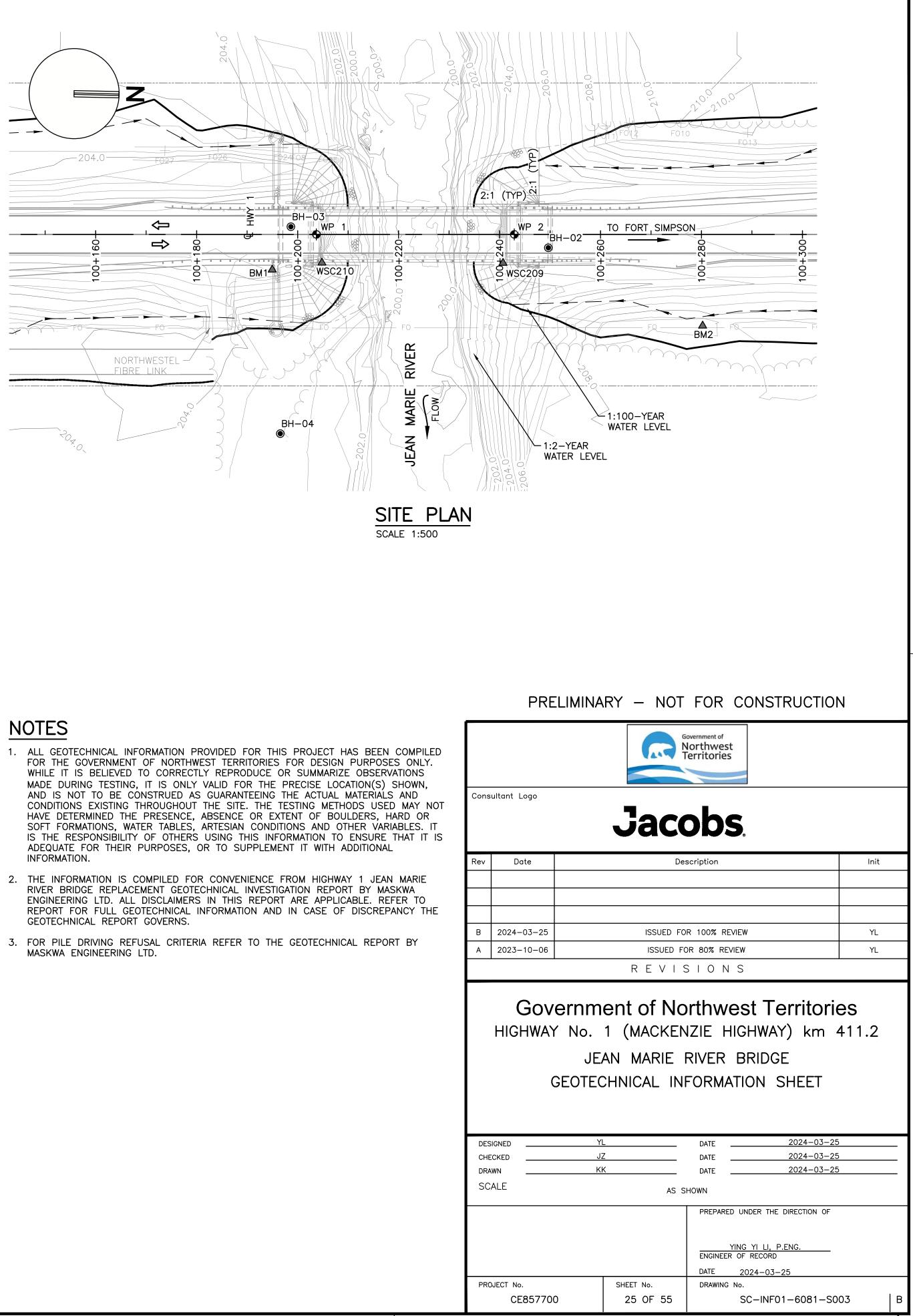
REVISIONS

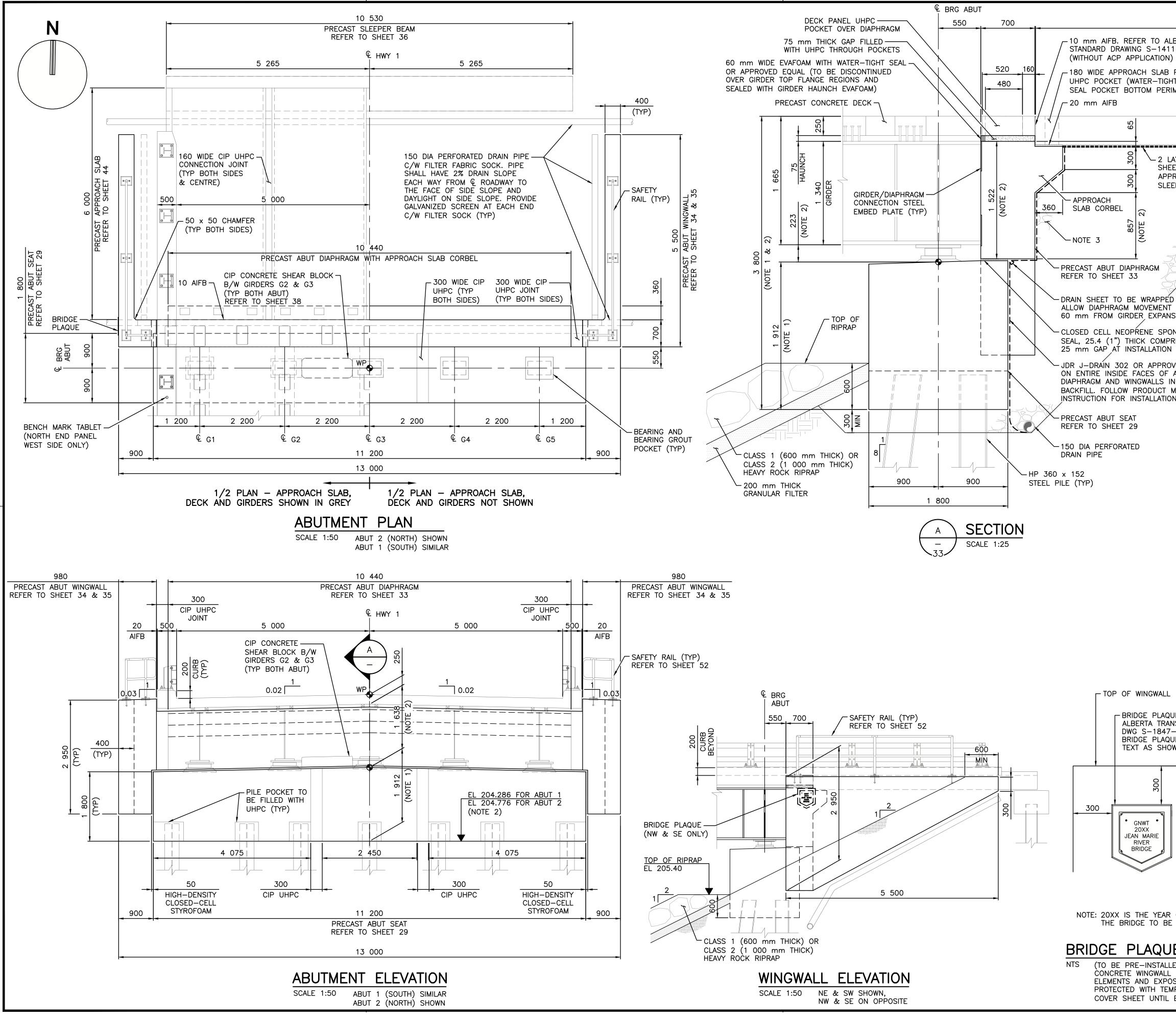
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Government of Northwest Territories								
HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411	.2							
JEAN MARIE RIVER BRIDGE								
GENERAL NOTES								

DESIGNEDYI	<u> </u>	DATE2024-03-25	_
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PROJECT No.	SHEET No.	DRAWING No.	
CE857700	24 OF 55	SC-INF01-6081-S002	С

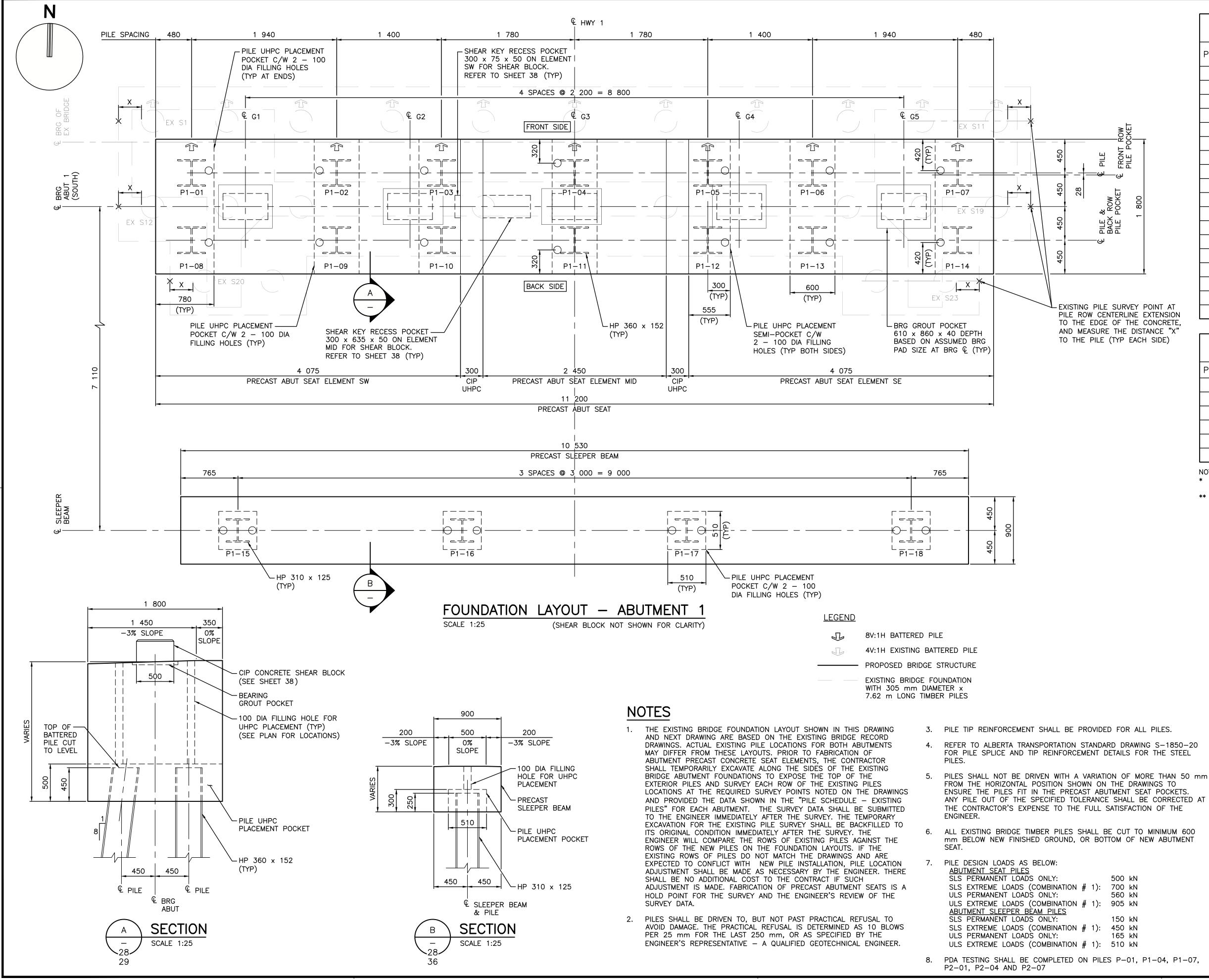






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PANEL - PRI	ECAST TRANSF	n AIFB. REFER TO PORTATION STANDA	RD DRAWING \ \				
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BACKFILL		STEEL PILE	125 - BEAM 900				
) TO OF SION	NOTES		, - - − − 1				
	NOTES						
RESSED TO FIT	 DIMENSIONS ARE TAKE ABUTMENT SEAT, PILE 	-	ABUTMENT. ONS, BEARING GROUT POCKET S				
VED ALTERNATE	BLOCK AND BEARING	ANCHORAGE SOCK	ET LOCATIONS, ABUTMENT DIAPHI HEIGHTS AND SIZES OF THE BE	RAGM			
ABUT SEAT, N CONTACT WITH	HEIGHTS AND SIZES S	HALL BE ADJUSTEI	EARING DRAWINGS. RELATED ELED TO ACCOMMODATE THE ACTUAL	L BEARING			
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			ASSOCIATED COSTS INCURRED I LY BY THE CONTRACTOR.	DUE TO			
			S SHALL BE PLACED AND COMP TRICAL FILL DEPTH TO AVOID UN				
	EARTH PRESSURE ON	THE BRIDGE SYST	EM. THE FILL HEIGHT DIFFERENCE 0 mm AT ANY TIME DURING TH	E BETWEEN			
	OPERATION.						
	WINGWALLS, INCLUDE	UHPC AREAS, EXTE	ABUTMENT DIAPHRAGMS, SEATS END TO 600 mm BELOW FINISH SEALER, COLOUR OF CONCRET	ED GROUND			
		ARY — NUI	FOR CONSTRUCTION				
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	Governm	nent of No	rthwest Territorie	es			
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	JEAN MARIE RIVER BRIDGE						
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IPORARY PLASTIC ELEMENT INSTALLED)	PROJECT No. CE857700	SHEET No. 26 OF 55	DRAWING No. SC-INF01-6081-S00)4 C			
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PILE SCHEDULE – ABUTMENT 1						
PILE MARK	NORTHING	EASTING	CUT OFF ELEVATION	APPROXIMATE TIP ELEVATION	PILE LENGTH	
P1-01	6813599.86	593969.35	204.736	189.800	14.936	
P1-02	6813599.90	593971.29	204.736	189.800	14.936	
P1-03	6813599.93	593972.69	204.736	189.800	14.936	
P1-04	6813599.97	593974.47	204.736	189.800	14.936	
P1-05	6813600.01	593976.25	204.736	189.800	14.936	
P1-06	6813600.04	593977.65	204.736	189.800	14.936	
P1-07	6813600.09	593979.58	204.736	189.800	14.936	
P1-08	6813598.96	593969.37	204.736	189.800	15.052	
P1-09	6813599.00	593971.31	204.736	189.800	15.052	
P1-10	6813599.03	593972.71	204.736	189.800	15.052	
P1-11	6813599.07	593974.49	204.736	189.800	15.052	
P1-12	6813599.11	593976.27	204.736	189.800	15.052	
P1-13	6813599.14	593977.67	204.736	189.800	15.052	
P1-14	6813599.19	593979.60	204.736	189.800	15.052	
P1-15	6813592.32	593970.13	207.247	191.000	16.247	
P1-16	6813592.38	593973.13	207.247	191.000	16.247	
P1-17	6813592.45	593976.13	207.247	191.000	16.247	
P1-18	6813592.52	593979.13	207.247	191.000	16.247	

PILE SCHEDULE - EXISTING PILES

PILE MARK	NORTHING*	EASTING*	NORTHING**	EASTING**	OFFSET "X"
EX S1	6813600.54	593968.81			
EX S11	6813600.79	593980.09			
EX S12	6813599.39	593968.84			
EX S19	6813599.64	593980.11			
EX S20	6813598.42	593969.55			
EX S23	6813598.64	593979.45			

NOTE:

* THEORETICAL NORTHING/EASTING AT CENTRE OF PILES BASED ON GEOMETRIC

INFORMATION FROM RECORD DRAWINGS ** SURVEY POINT NORTHING/EASTING AND OFFSET DISTANCE "X" TO BE PROVIDED BY THE CONTRACTOR

PRELIMINARY - NOT FOR CONSTRUCTION

Territories

Government of Northwest



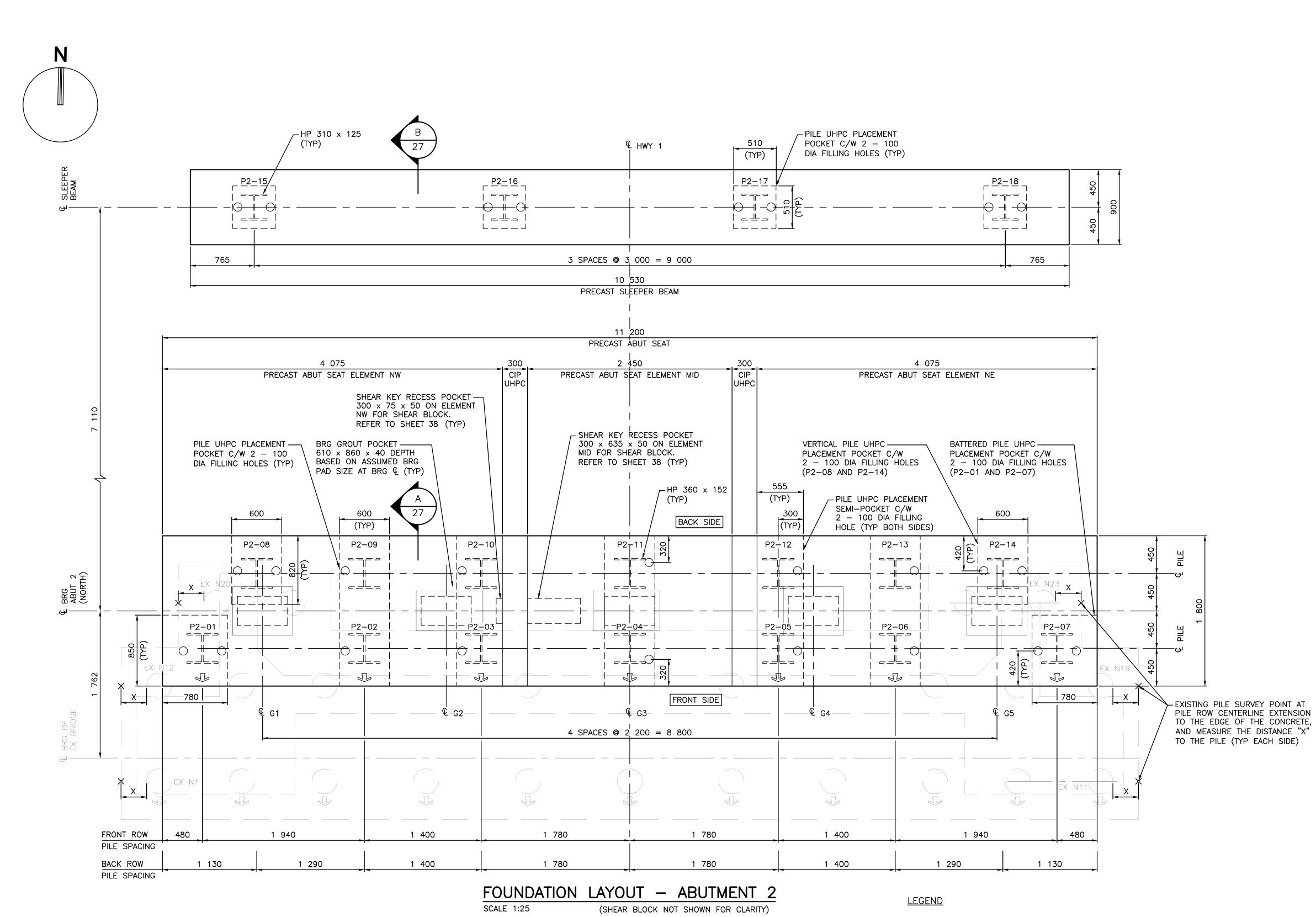


Rev	Date	Description	Init
С	2024-03-25	ISSUED FOR 100% REVIEW	YL
В	2023-10-06	ISSUED FOR 80% REVIEW	YL
А	2023-06-19	ISSUED FOR 50% REVIEW	YL
		REVISIONS	

Government of Northwest Territories
HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2
JEAN MARIE RIVER BRIDGE
FOUNDATION LAYOUT

SHEET 1

	DESIGNED	YL	DATE	2024-03-25	
	CHECKED	JZ	DATE _	2024-03-25	
	DRAWN	К	DATE _	2024-03-25	
	SCALE	AS S	GHOWN		
			PREPARED	UNDER THE DIRECTION OF	
				NG YI LI, P.ENG. OF RECORD 2024–03–25	
	PROJECT No.	SHEET No.			
P1-07,	CE857700	27 OF 55		SC-INF01-6081-S005	C



- 8V:1H BATTERED PILE J
- 4V:1H EXISTING BATTERED PILE J - PROPOSED BRIDGE STRUCTURE
- EXISTING BRIDGE FOUNDATION WITH 305 mm DIAMETER x 7.62 m LONG TIMBER PILES

PILE SCHEDULE – ABUTMENT 2									
PILE MARK	NORTHING	EASTING	CUT OFF ELEVATION	APPROXIMATE TIP ELEVATION	PILE LENGTH				
P2-01	6813638.15	593968.50	205.226	189.800	15.426				
P2-02	6813638.19	593970.44	205.226	189.800	15.426				
P2-03	6813638.22	593971.84	205.226	189.800	15.426				
P2-04	6813638.26	593973.62	205.226	189.800	15.426				
P2-05	6813638.30	593975.40	205.226	189.800	15.426				
P2-06	6813638.33	593976.80	205.226	189.800	15.426				
P2-07	6813638.38	593978.73	205.226	189.800	15.426				
P2-08	6813639.06	593969.13	205.226	189.800	15.546				
P2-09	6813639.09	593970.42	205.226	189.800	15.546				
P2-10	6813639.12	593971.82	205.226	189.800	15.546				
P2-11	6813639.16	593973.60	205.226	189.800	15.546				
P2-12	6813639.20	593975.38	205.226	189.800	15.546				
P2-13	6813639.23	593976.78	205.226	189.800	15.546				
P2-14	6813639.26	593978.06	205.226	189.800	15.546				
P2-15	6813645.72	593968.95	207.915	191.000	16.915				
P2-16	6813645.79	593971.95	207.915	191.000	16.915				
P2-17	6813645.86	593974.95	207.915	191.000	16.915				
P2-18	6813645.92	593977.94	207.915	191.000	16.915				

PILE SCHEDULE - EXISTING PILES

PILE MARK	NORTHING*	EASTING*	NORTHING**	EASTING**	OFFSET "X"
EX N1	6813636.55	593968.01			
EX N11	6813636.80	593979.29			
EX N12	6813637.70	593967.99			
EX N19	6813637.95	593979.26			
EX N20	6813638.70	593968.65			
EX N23	6813638.92	593978.55			

NOTE:

* THEORETICAL NORTHING/EASTING AT CENTRE OF PILES BASED ON GEOMETRIC

INFORMATION FROM RECORD DRAWINGS ** SURVEY POINT NORTHING/EASTING AND OFFSET DISTANCE "X" TO BE PROVIDED

BY THE CONTRACTOR

PRELIMINARY - NOT FOR CONSTRUCTION

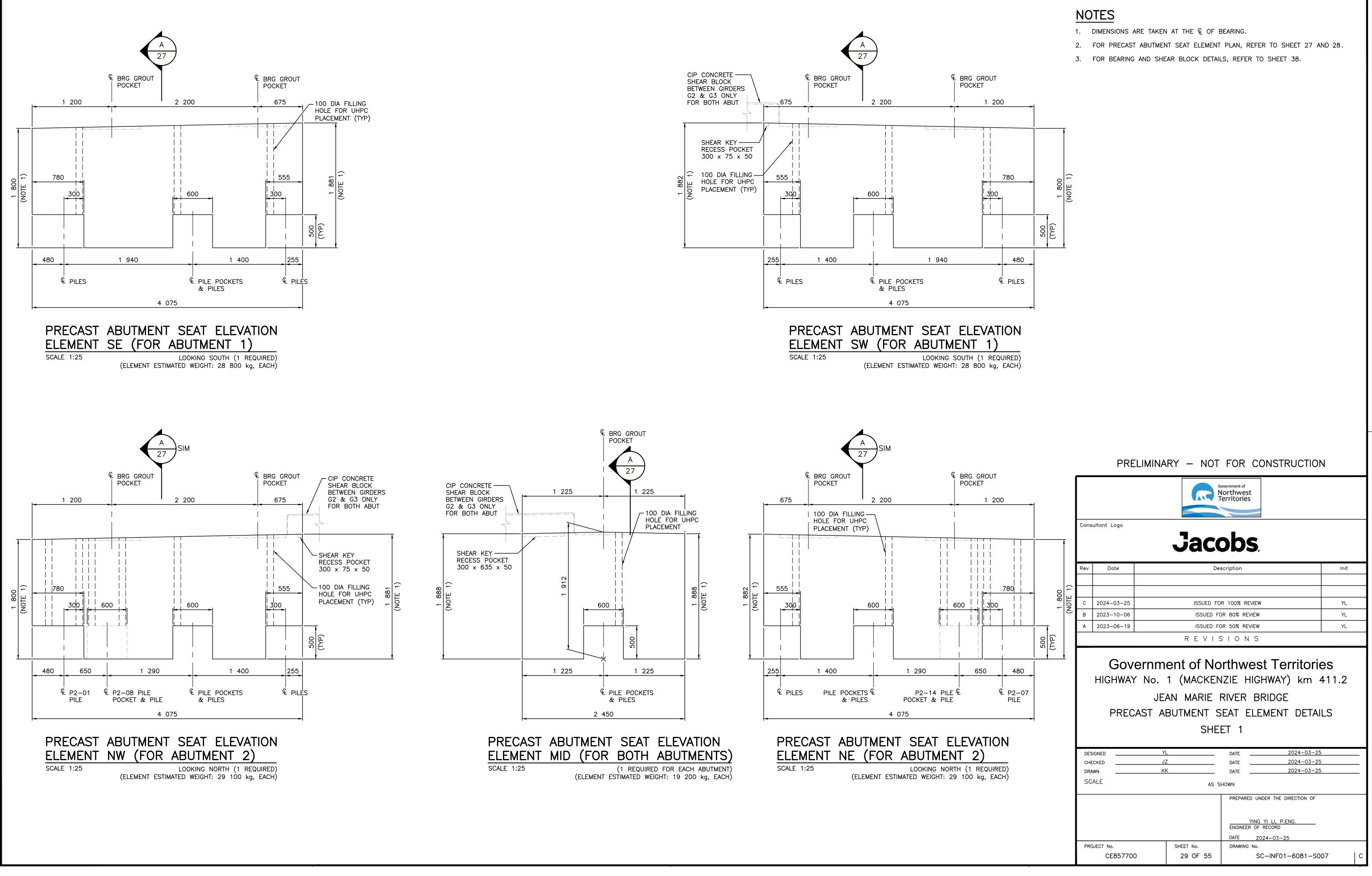


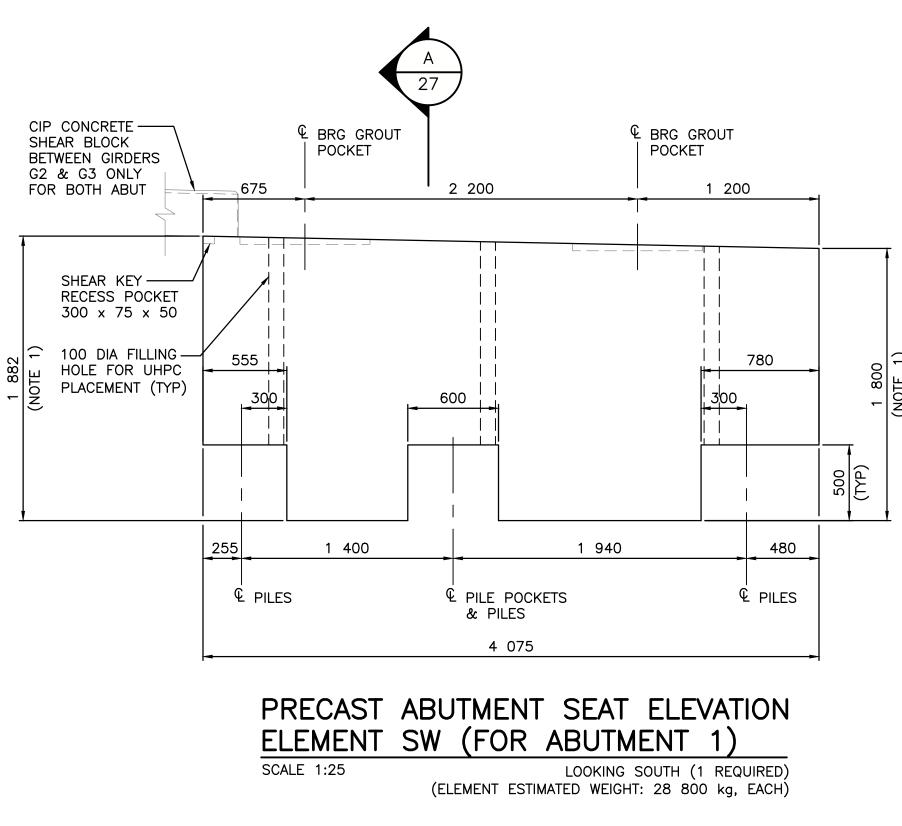
Consultant Logo

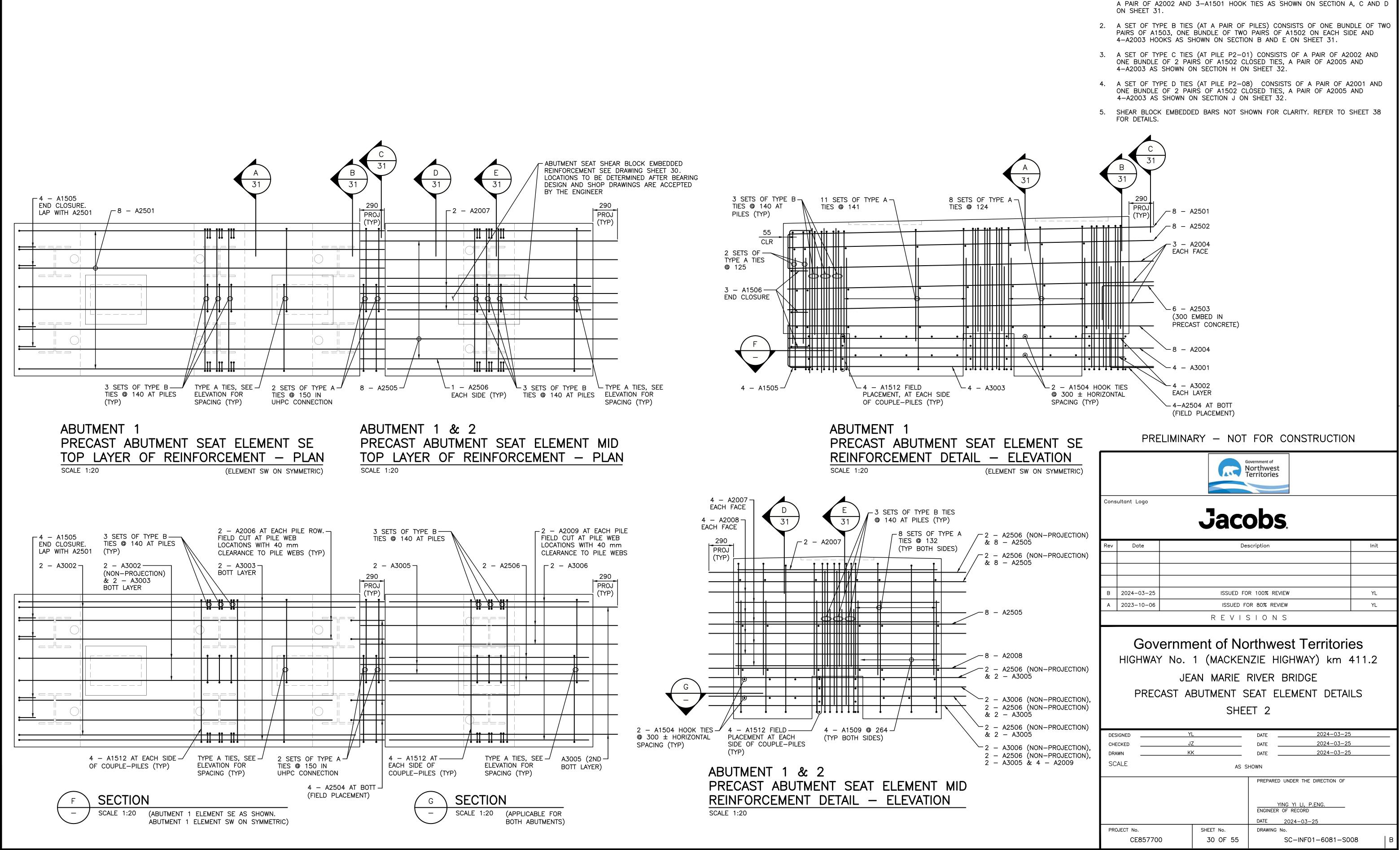


Rev	Date	Description	Init
С	2024-03-25	ISSUED FOR 100% REVIEW	YL
В	2023-10-06	ISSUED FOR 80% REVIEW	YL
А	2023-06-19	ISSUED FOR 50% REVIEW	YL
		REVISIONS	

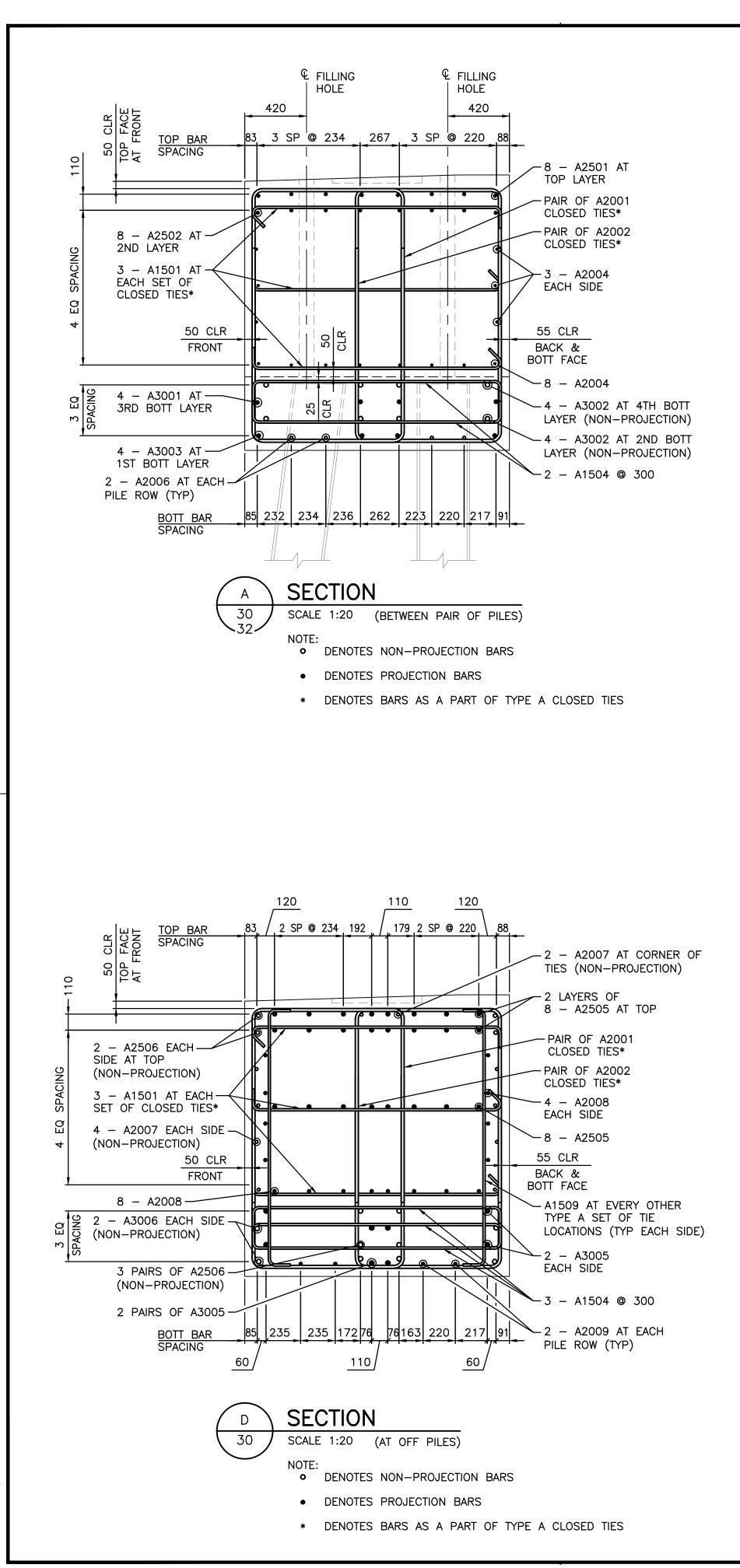
Government of Northwest Territories HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2									
JEAN MARIE RIVER BRIDGE									
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		ENGINEER O	FRECORD						
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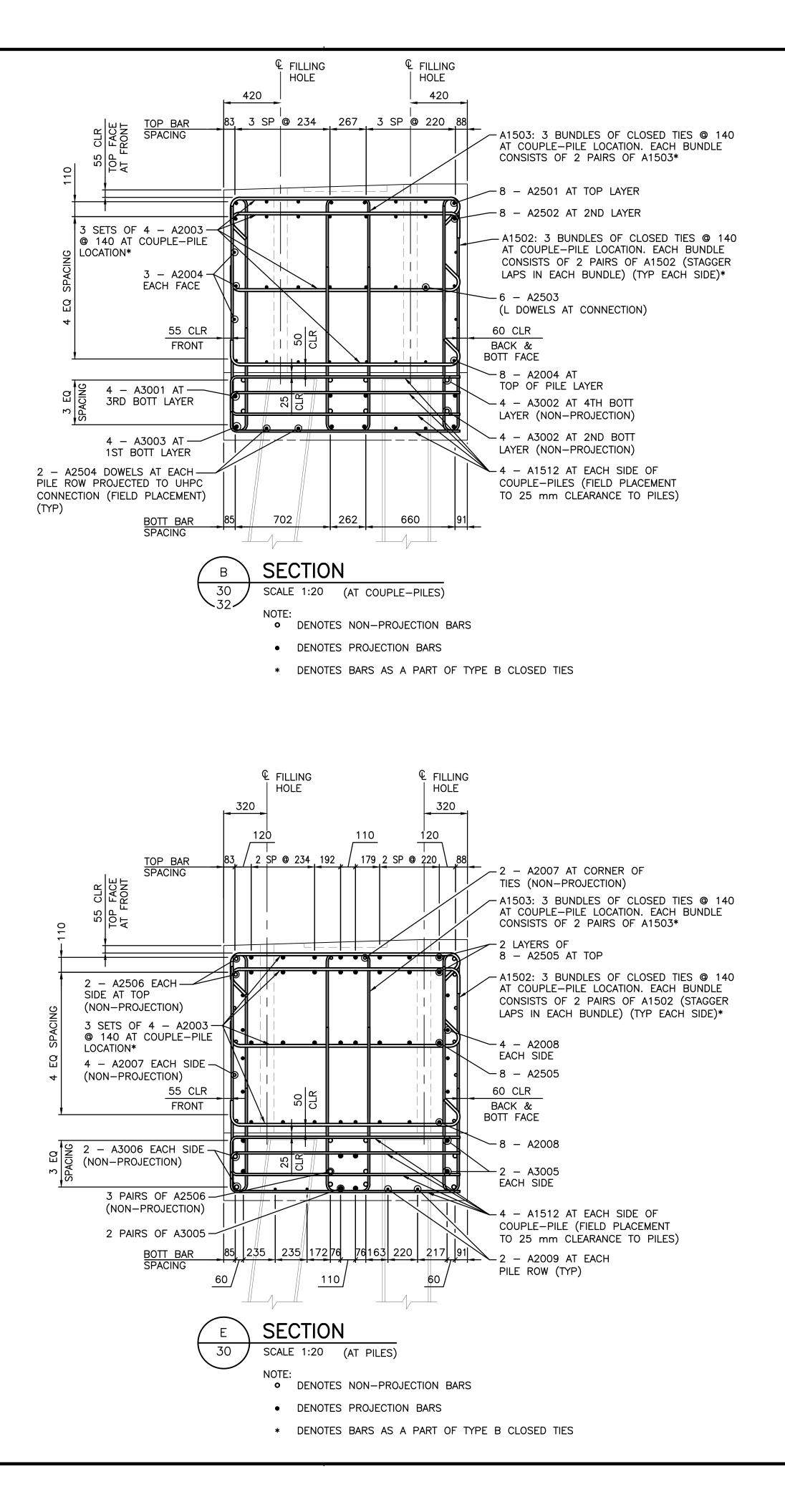




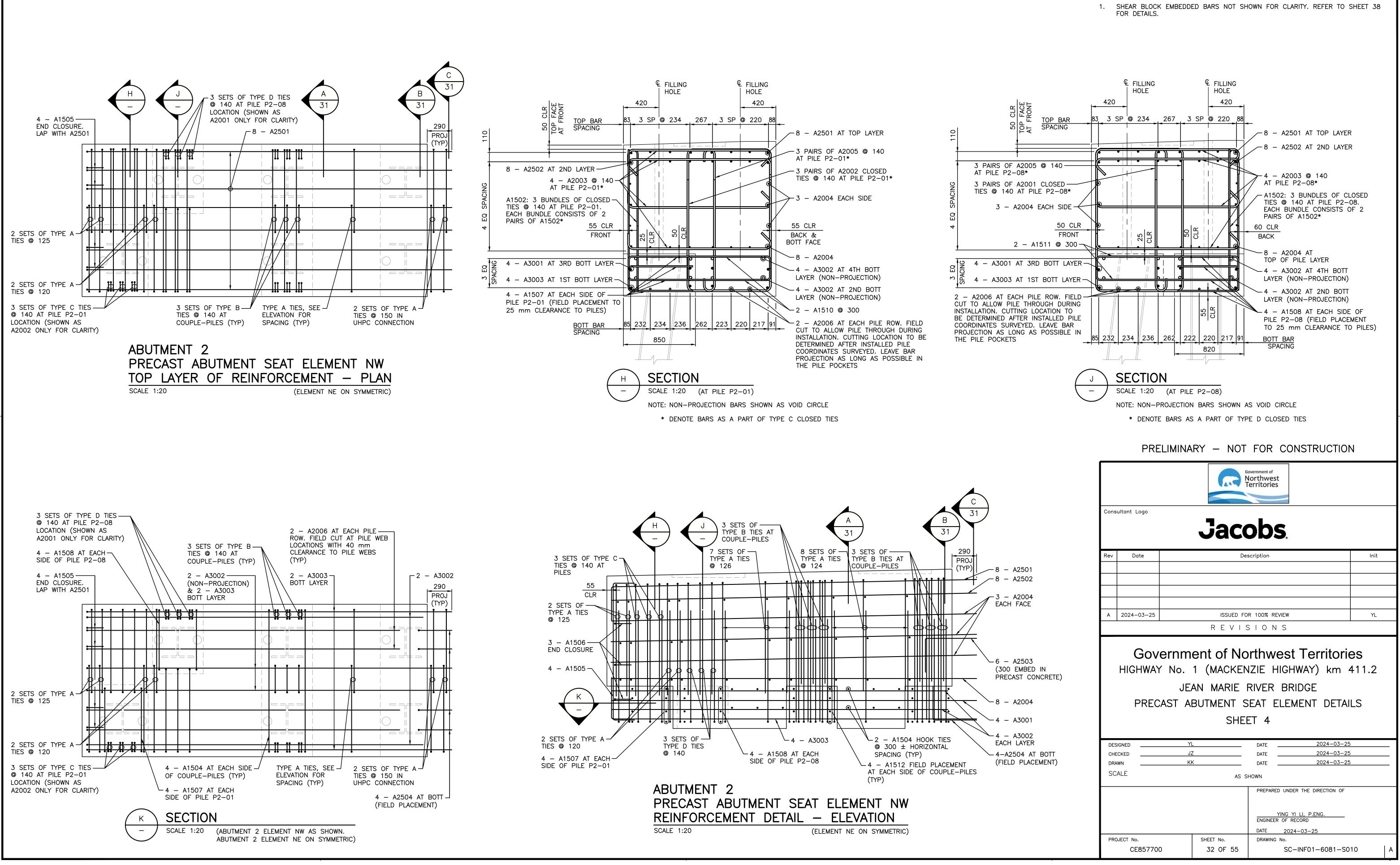
- 1. A SET OF TYPE A TIES (BEYOND PILE LOCATIONS) CONSISTS OF A PAIR OF A2001

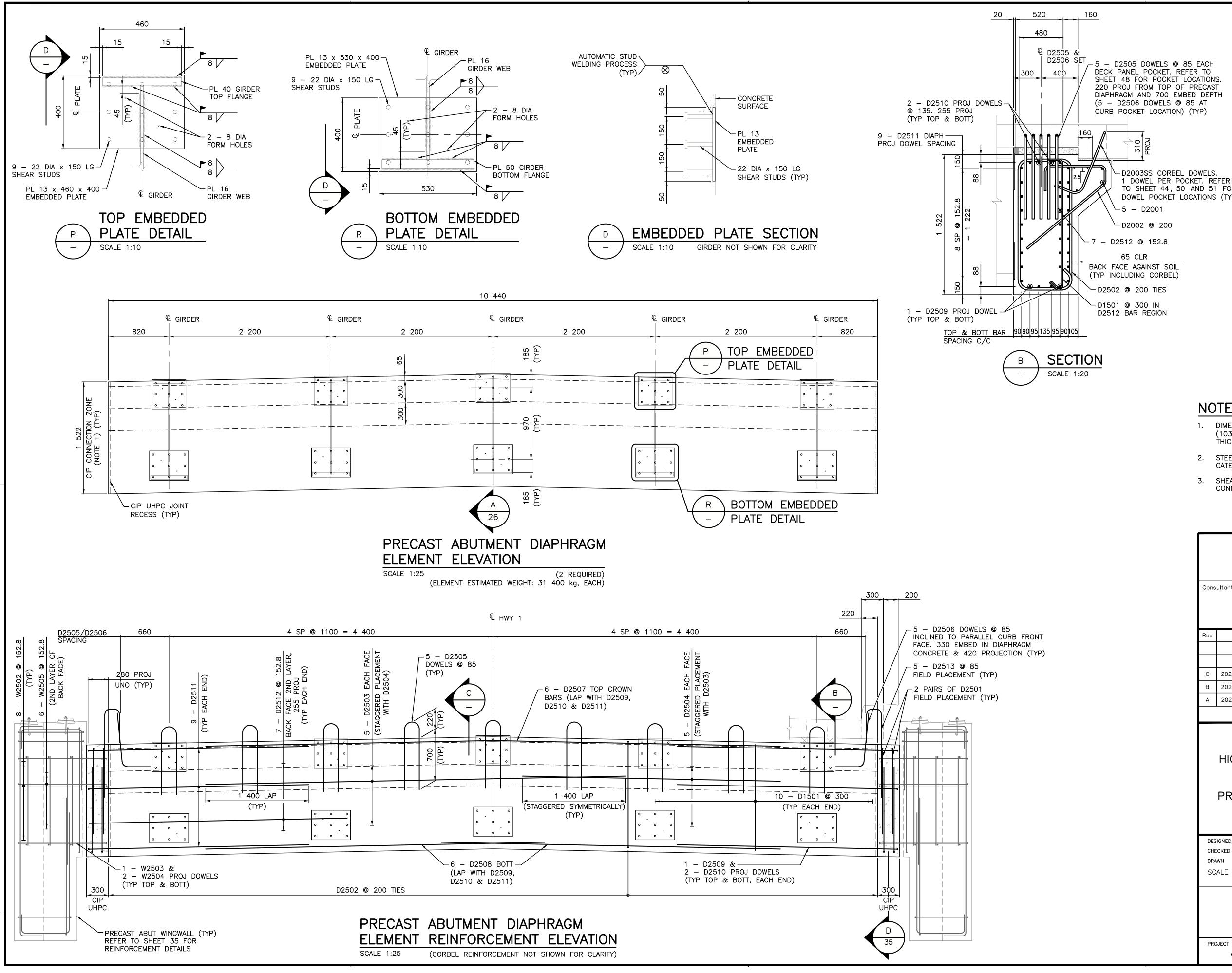


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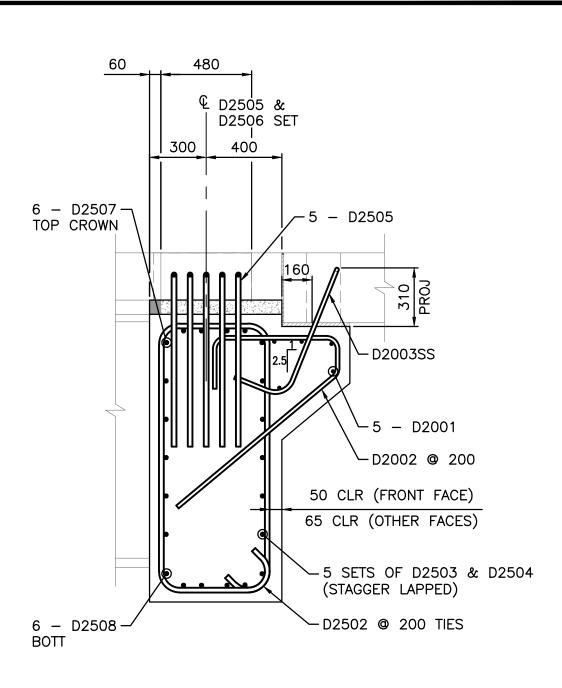


NOTES 1. SHEAR BLC FOR DETAIL		DED BARS NOT SH	OWN FOR CLARIT	TY. REFER TO S	SHEET 38
			CLOSED 2 PAIRS CLOSED BAR PRO END ELE AS SOLI	G OF A2002 TIES OJECTED FROM EMENT SHOWN D (TYP) OJECTED FROM F MID SHOWN	
C 30 32 SCALE	(, ,,	UHPC CONNECTIO LICABLE TO BOTH			
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TO SHEET 44, 50 AND 51 FOR DOWEL POCKET LOCATIONS (TYP)

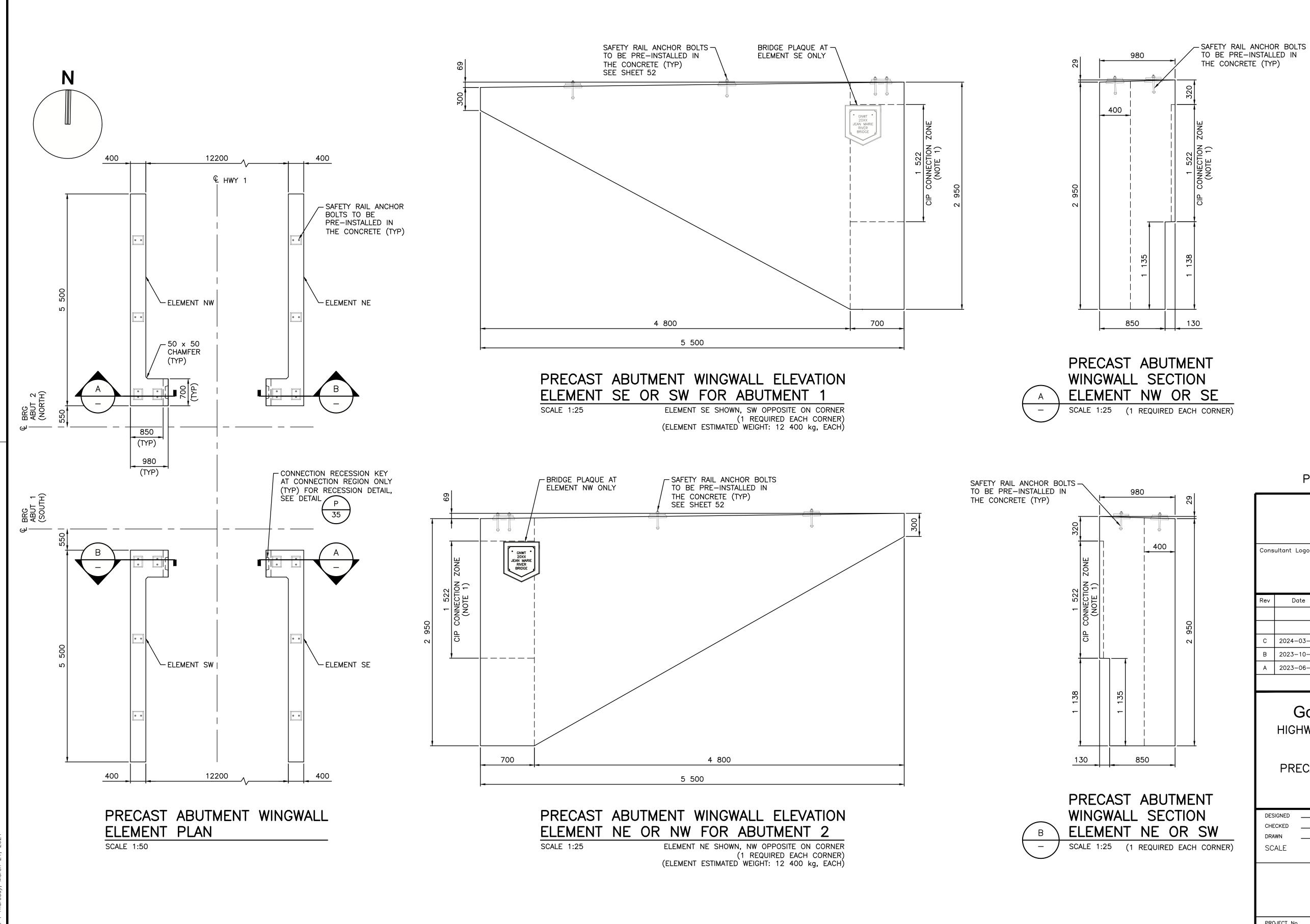




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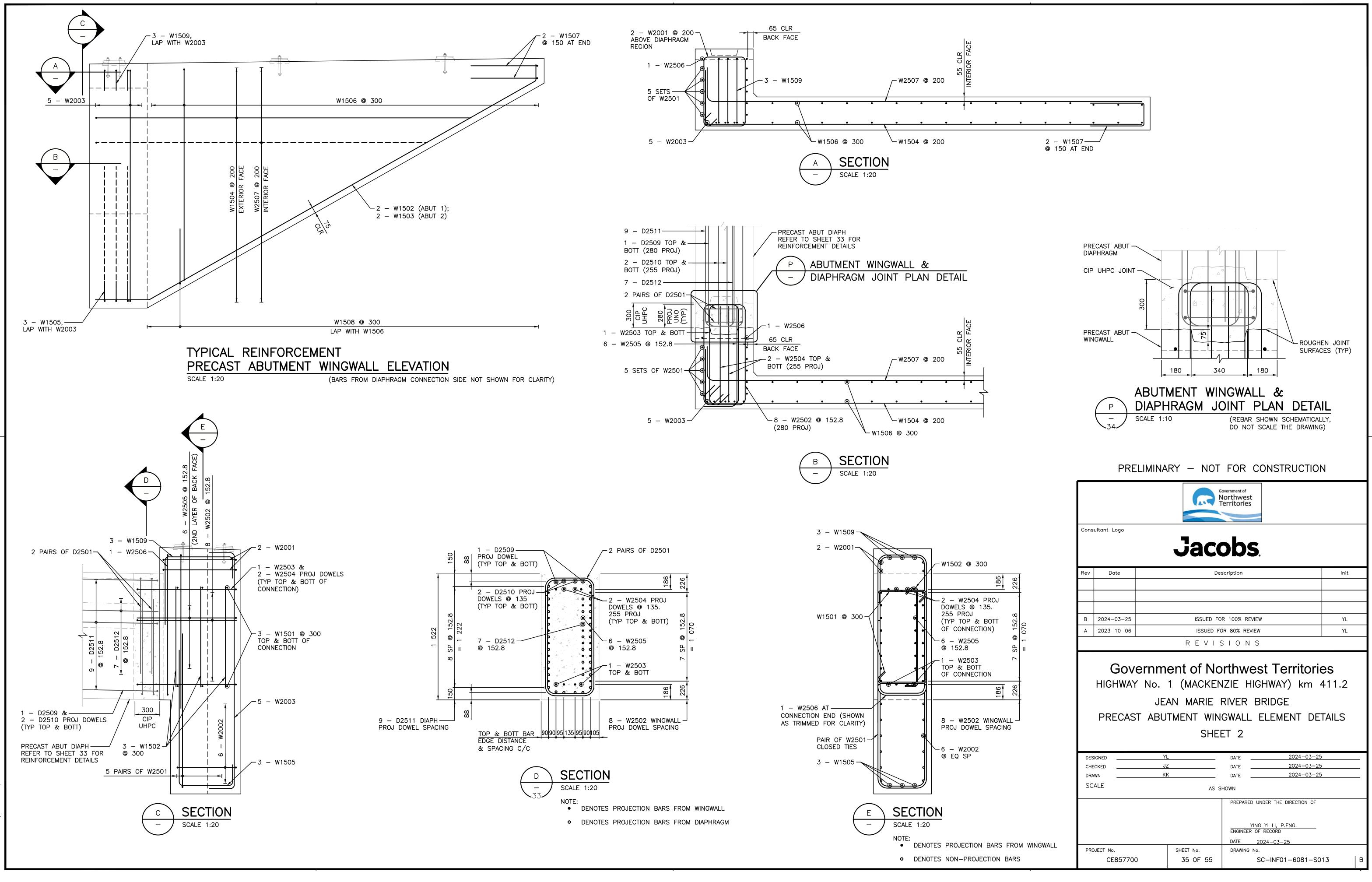
- 1. DIMENSIONS ARE BASED ON THE ASSUMED HEIGHT OF ELASTOMERIC BEARING PAD (103 mm) AND NEED TO BE ADJUSTED BASED ON ACTUAL BEARING PAD THICKNESS.
- 2. STEEL EMBEDDED PLATES SHALL CONFORM TO CAN/CSA G40.21M-350AT, CATEGORY 3, WITH CHARPY V-NOTCH STRENGTH OF 27 JOULES AT -45°C.
- 3. SHEAR STUDS SHALL BE CONFORM TO SUBSECTION 6.2.4.3, STUD SHEAR CONNECTORS, OF STANDARD SPECIFICATIONS FOR BRIDGE CONSTRUCTION.

Government of Northwest Territories									
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	HIGHWAY	Y No. JE	ent of No 1 (Macken AN MARIE MENT DIAP	IZIE HIGHW RIVER BRIE	VAY) km 4 DGE	411.2			
DES	GIGNED	Y		DATE	2024-03-25				
		J; Ki		DATE	<u> 2024–03–25</u> 2024–03–25				
DRA	ALE	<u> </u>		DATE	2024-00-20				
			AS S	HOWN					
	PREPARED UNDER THE DIRECTION OF YING YI LI, P.ENG. ENGINEER OF RECORD DATE 2024–03–25								
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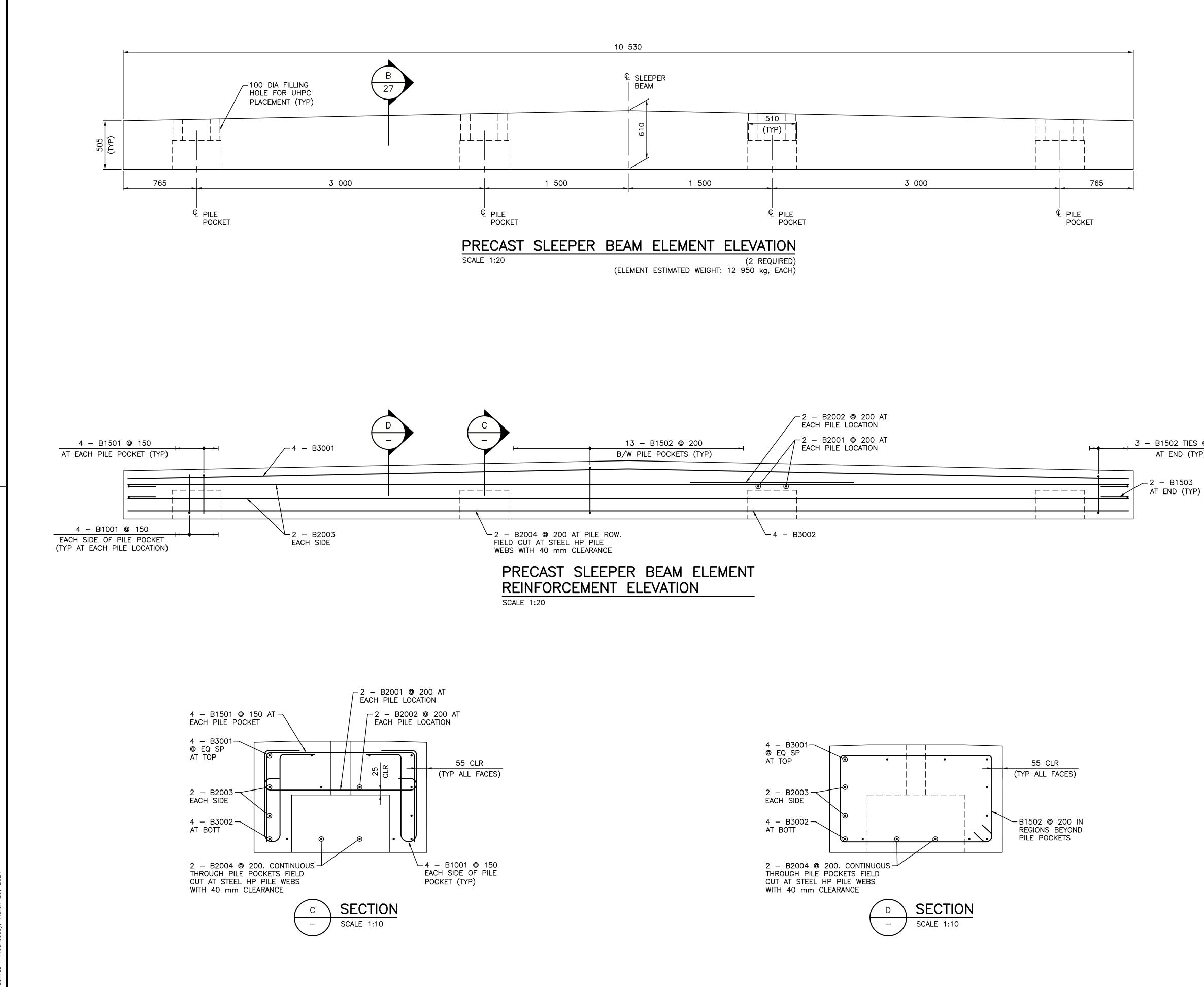


1. DIMENSIONS ARE BASED ON THE ASSUMED HEIGHT OF ELASTOMERIC BEARING PAD (103 mm) AND NEED TO BE ADJUSTED BASED ON ACTUAL BEARING PAD THICKNESS.

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MENT		HIGHWAY N	o. 1 (MACKEN JEAN MARIE ABUTMENT WIN	orthwest Territorie IZIE HIGHWAY) km RIVER BRIDGE GWALL ELEMENT DE ET 1	411.2
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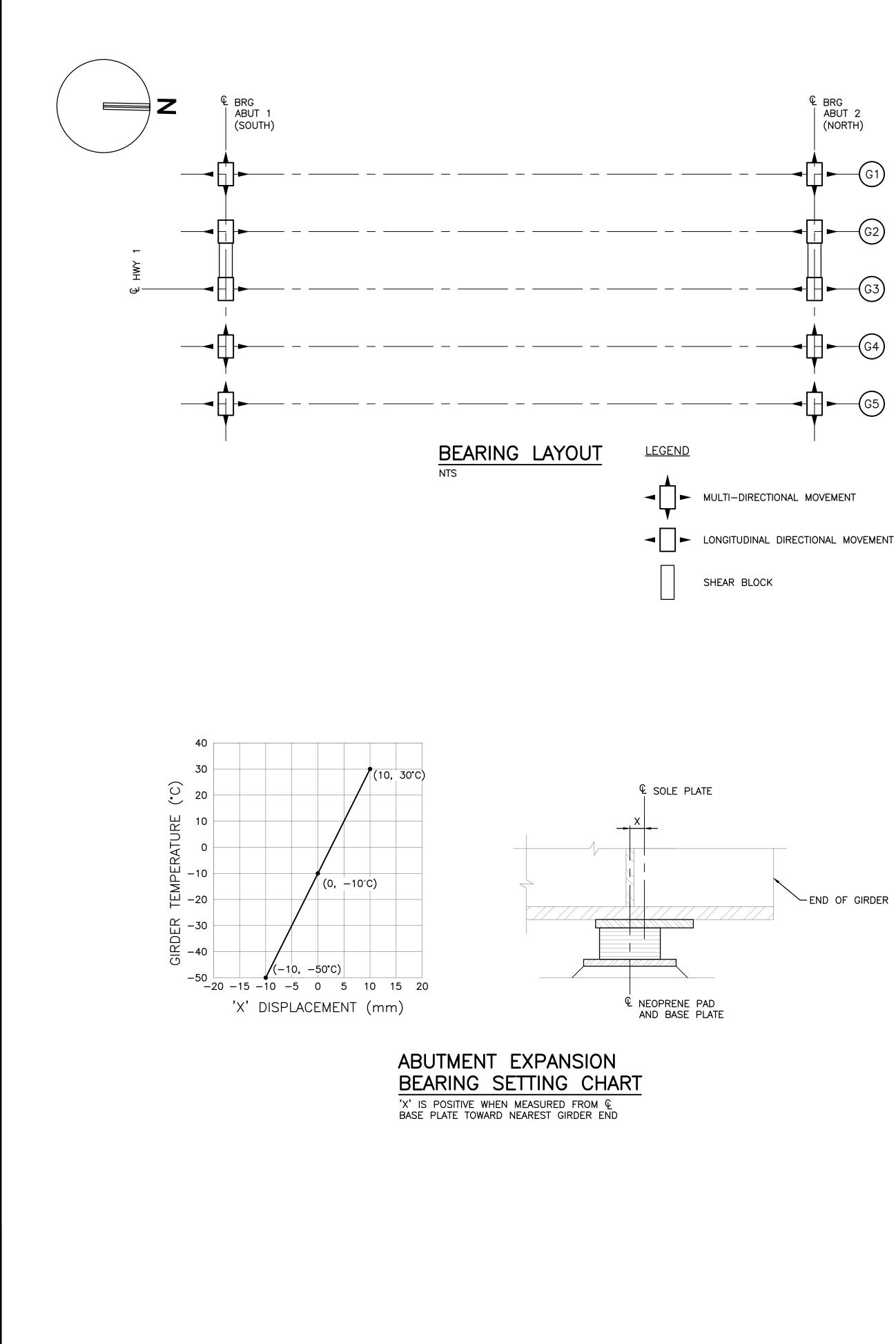


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			REVI	SIONS			
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		′No.	1 (MACKE	NZIE HIG	s t Territor HWAY) km RIDGE		
	HIGHWAY	′No. JE/ CAST S	1 (MACKE AN MARIE SLEEPER E	NZIE HIGI RIVER BI	HWAY) km RIDGE MENT DETA	411.2	
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PRELIMINARY - NOT FOR CONSTRUCTION

Government of Northwest Territories

3 – B1502 TIES @ 200 AT END (TYP)



BEARING NOTES

GENERAL

- THE NORTHWEST TERRITORIES.
- ANCHOR RODS SHALL BE DESIGNED IN ACCORDANCE WITH CAN/CSA-S6-19 AND THE ALBERTA TRANSPORTATION STRUCTURES DESIGN CRITERIA V9.0 APPENDIX D.
- 3. ALL REQUIREMENTS OF GNWT STANDARD SPECIFICATIONS FOR STRUCTURAL STEEL AND SECTION 8 SHALL BE MET.
- 4. ABUTMENT SEAT AND PILE CUT-OFF ELEVATIONS, AND RELATED DRAWINGS. THE CONTRACTOR SHALL ENSURE THAT THE IS TO PROVIDE WRITTEN NOTIFICATION OF ANY CHANGE TO ELEVATIONS AND PRECAST CONCRETE ELEMENT DIMENSIONS INCLUDING ANY SUBSEQUENT CHANGES TO THE BRIDGE CHANGES SHALL BE BORNE SOLELY BY THE CONTRACTOR.
- 5. FABRICATION OF BEARINGS SHALL NOT COMMENCE UNTIL SHOP DRAWINGS HAVE BEEN REVIEWED AND ACCEPTED BY THE ENGINEER.
- 6. THE CONTRACTOR SHALL ENGAGE A QUALIFIED INDEPENDENT TESTING COMPANY AT THE CONTRACTOR'S EXPENSE TO IN ACCORDANCE AS DESCRIBED IN SSBC.

INSTALLATION

- ERECTED AND INTERMEDIATE DIAPHRAGMS ARE INSTALLED. PRECAST CONCRETE DECK AND ABUTMENT DIAPHRAGM
- 2. ALL BEARINGS SHALL BE SUPPLIED COMPLETE WITH SOLE PLATES, BASE PLATES AND BOLTS AS DETAILED.
- 3. ALL WELDING SHALL CONFORM TO CURRENT AWS BRIDGE WELDING CODE D1.5.

BEARING SCHEDULE								
				G1, G4, G5 G2 & G3			G3	
	BEARING					VALUE	LC	
			MAX	1320		1320		
		VERT	PERM	825		825		
	SLS		MIN	825		825		
		LONG		135		135		
DESIGN		TRANS		135		135		
BEARING REACTION		VERT	MAX	1840	ULS1	1840	ULS1	
(kN)	ULS		PERM	910	ULS1	910	ULS1	
			MIN	750	ULS1	750	ULS1	
		LONG			455 (L	ILS 5)*		
		TRANS		455 (ULS 5)*				
	FLS	VERT		420		420		
DESIGN BEARING		LONG		±60		±60		
MOVEMENT (mm)		TRANS		±12		±2		
DESIGN BEARING	SLS	LONG		0.025		0.025		
ROTATION (rad)		TRANS		_		_		

NOTE: * BEARING TRAVERSE LOAD IS RESISTED BY EACH SHEAR BLOCK; LONGITUDINAL LOAD IS RESISTED BY BACKFILL SOIL BEHIND EACH ABUTMENT DIAPHRAGM.

GIRDER AND BEARIN	NG ELE	VATIONS	SAT 🖗	OF BE	ARINO
ABUT 1 (SOUTH)	G1	G2	G3	G4	G5
TOP OF GROUT PAD ELEVATION (m)					
BEARING HEIGHT (mm)					
UNDERSIDE OF GIRDER ELEVATION (m)	206.333	206.377	206.421	206.377	206.33
ABUT 2 (NORTH)	G1	G2	G3	G4	G5
TOP OF GROUT PAD ELEVATION (m)					
BEARING HEIGHT (mm)					
UNDERSIDE OF GIRDER ELEVATION (m)	206.823	206.867	206.911	206.867	206.82
NOTE: TOP OF GROUT PAD ELEVATIONS A DRAWINGS ARE APPROVED AT CON				AFTER BEARI RING HEIGHTS	

1. ALL BEARINGS SHALL BE DESIGNED BY BEARING MANUFACTURER AND STAMPED BY A PROFESSIONAL ENGINEER REGISTERED IN

2. LAMINATED ELASTOMERIC BEARING PADS AND ALL OTHER COMPONENTS INCLUDING BUT NOT LIMITED TO TOP PLATES. STAINLESS STEEL SLIDING PLATES, BASE PLATES, CONNECTIONS,

BRIDGE CONSTRUCTION (SSBC) SECTION 6 FOR THE SUPPLY OF

BRIDGE ELEMENT SIZES SHALL BE ADJUSTED TO ACCOMMODATE THE PROPOSED BEARING ASSEMBLIES AS PER ACCEPTED SHOP ROADWAY PROFILE IS MAINTAINED AS SHOWN. THE CONTRACTOR STRUCTURE. ANY ASSOCIATED COSTS INCURRED DUE TO SUCH

PERFORM TESTING OF THE BEARINGS. THE TESTING SHALL BE

1. BEARING GROUT PADS SHALL BE GROUTED AFTER GIRDERS ARE ELEMENTS INSTALLATION SHALL NOT BE STARTED UNTIL GROUT PADS HAVE A MINIMUM COMPRESSIVE STRENGTH OF 35 MPa.

- 1. GROUT PADS AND ANCHOR BOLT VOIDS SHALL BE GROUTED WITH SIKA 212, FLOWABLE GROUT OR APPROVED EQUIVALENT. GROUT SHALL HAVE A MINIMUM 28 DAYS COMPRESSIVE STRENGTH OF 45 MPa AND SHALL NOT BE DRY PACKED.
- 2. SOLE PLATE STEEL SHALL CONFORM TO CSA G40.21M-350 AT CATEGORY 3 WITH CHARPY V-NOTCH STRENGTH OF 27 JOULES AT -45°C. THE STEEL FOR BASE PLATE, KEEPER BARS, PINTELS AND SHIMS SHALL CONFORM TO THE REQUIREMENTS OF CSA G40.21 GRADE 300W.

GALVANIZING

1. GALVANIZING SHALL BE HOT DIP METHOD AFTER FABRICATION IN ACCORDANCE WITH THE CURRENT EDITION OF ASTM A123/A123M STANDARD SPECIFICATION FOR ZINC (HOT-DIP GALVANIZED) COATINGS ON IRON AND STEEL PRODUCTS AND ASTM F2329 STANDARD SPECIFICATION FOR ZINC COATING (HOT-DIP).

JACKING LOADS

PERMANENT DEAD LOAD:

825 kN (UN-FACTORED)

PERMANENT DEAD LOAD + 1 LANE OF CL 800 TRUCK (AT CENTER OF ROADWAY):

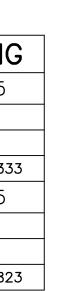
990 kN (UN-FACTORED)

(ABOVE JACKING LOADS PROVIDED BY ASSUMING NO BACKFILL SOIL FRICTION ON DIAPHRAGMS AND WINGWALLS DURING THE JACKING)

Consultant Logo Consul			Government of Northwest Territories	
Rev Date Description Init B 2024-03-25 ISSUED FOR 100% REVIEW YL A 2023-10-06 ISSUED FOR 80% REVIEW YL R E VISIO NS YL Government of Northwest Territories HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2 JEAN MARIE RIVER BRIDGE BEARING LAYOUT	Cons	sultant Logo	Jacobs	
A 2023-10-06 ISSUED FOR 80% REVIEW YL R E V I S I O N S R E V I S I O N S Government of Northwest Territories HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2 JEAN MARIE RIVER BRIDGE JEAN MARIE RIVER BRIDGE BEARING LAYOUT DESIGNED YL DATE 2024-03-25 GRAWN KK DATE 2024-03-25 DATE 2024-03-25 OATE 2024-03-25 DATE 2024-03-25 DATE 2024-03-25 DATE 2024-03-25 OATE 2024-03-25	Rev	Date		Init
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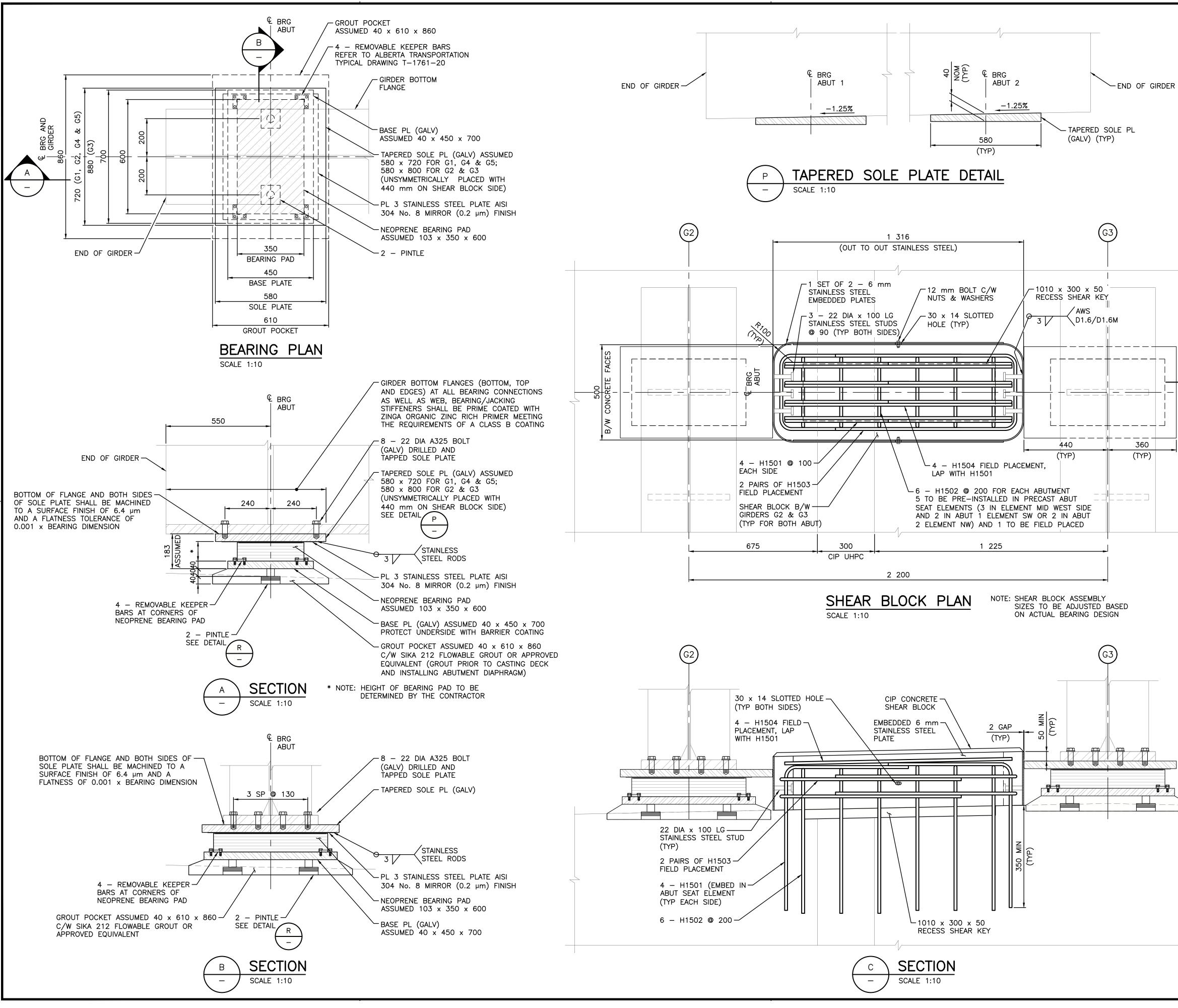
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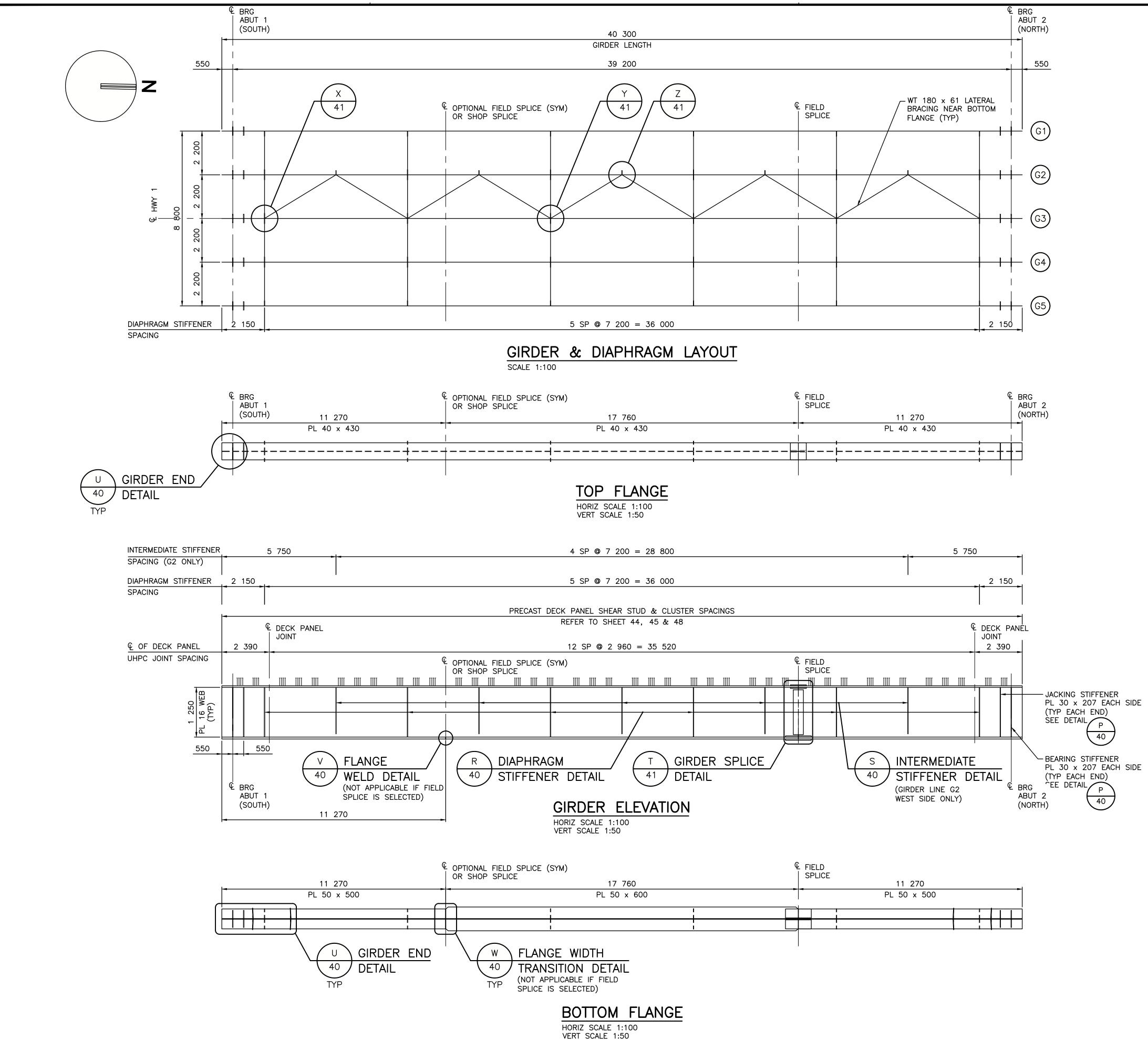


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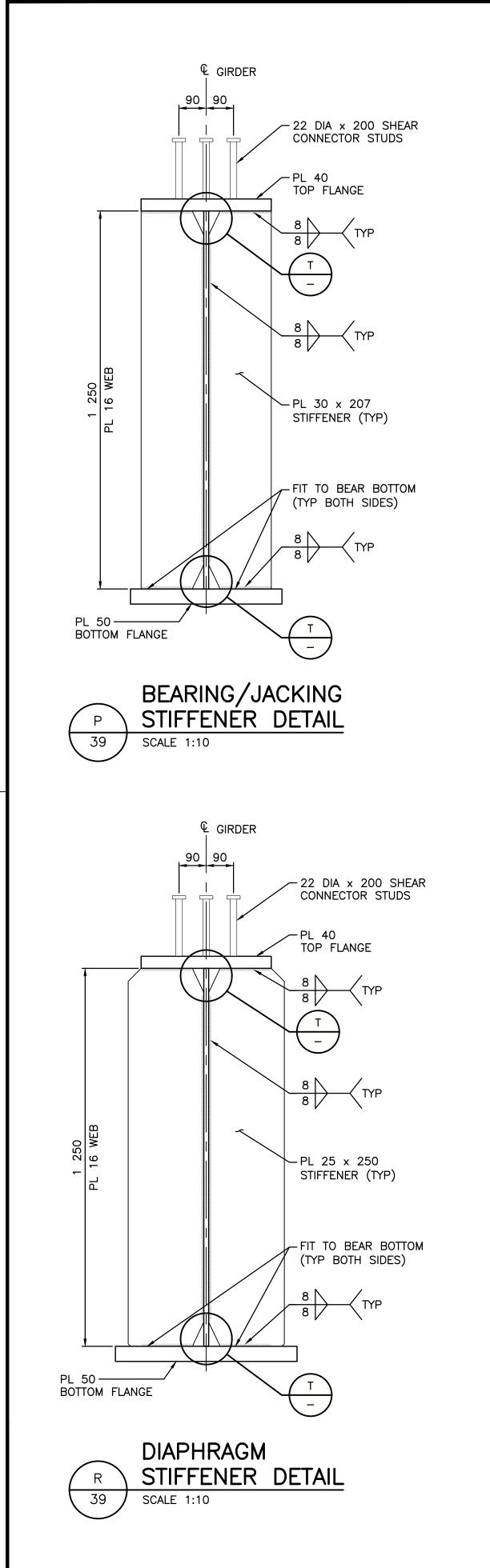
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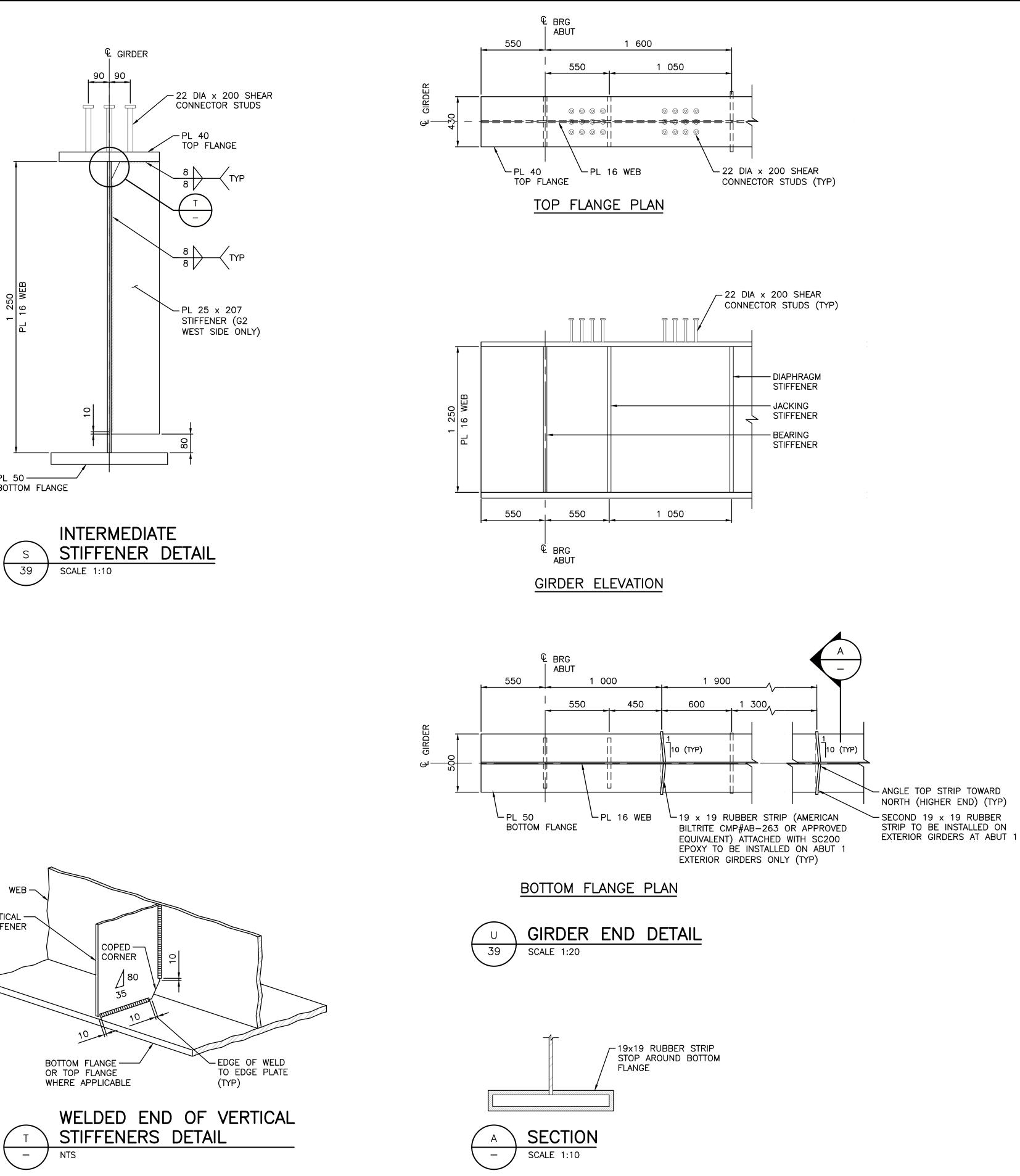


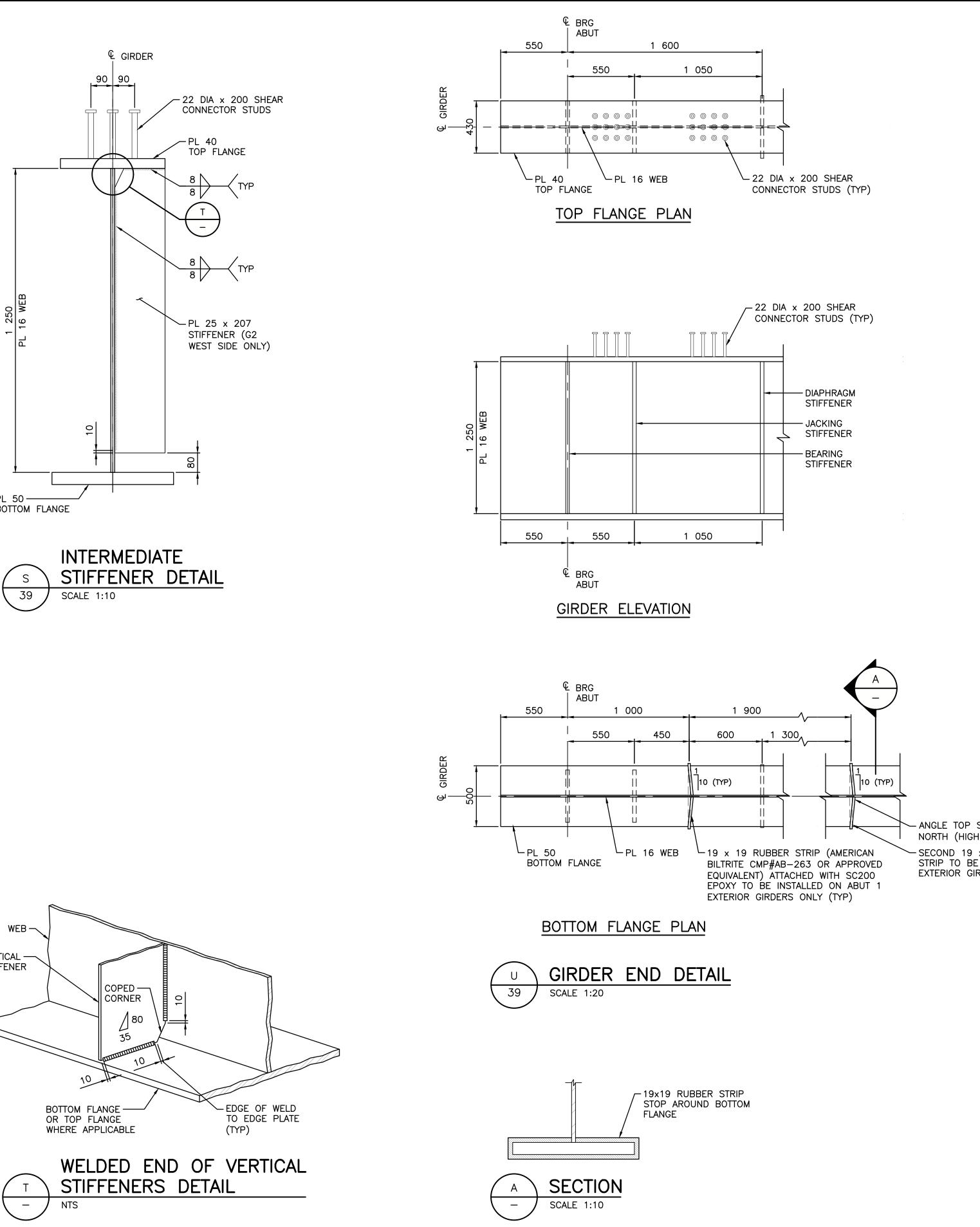
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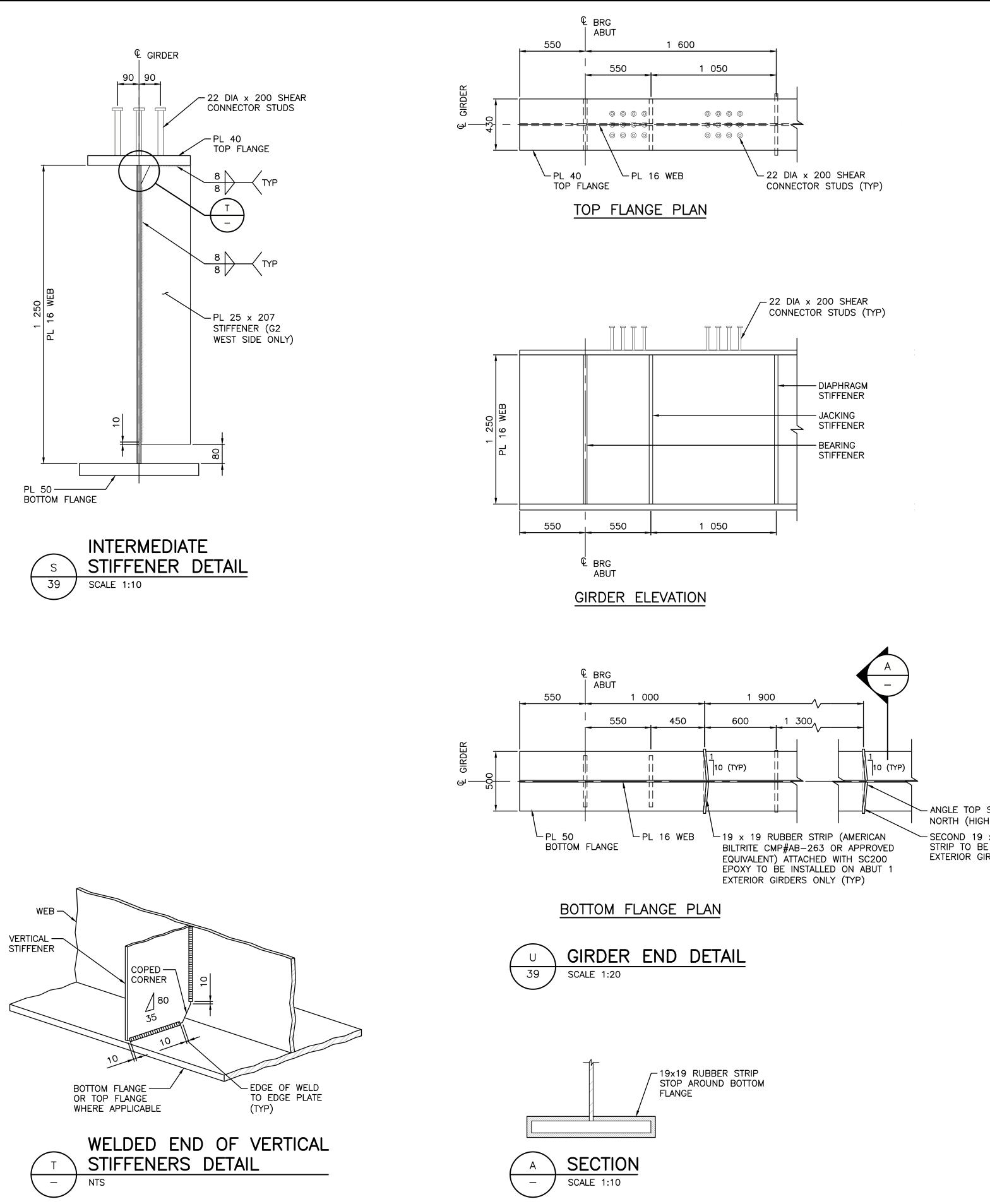


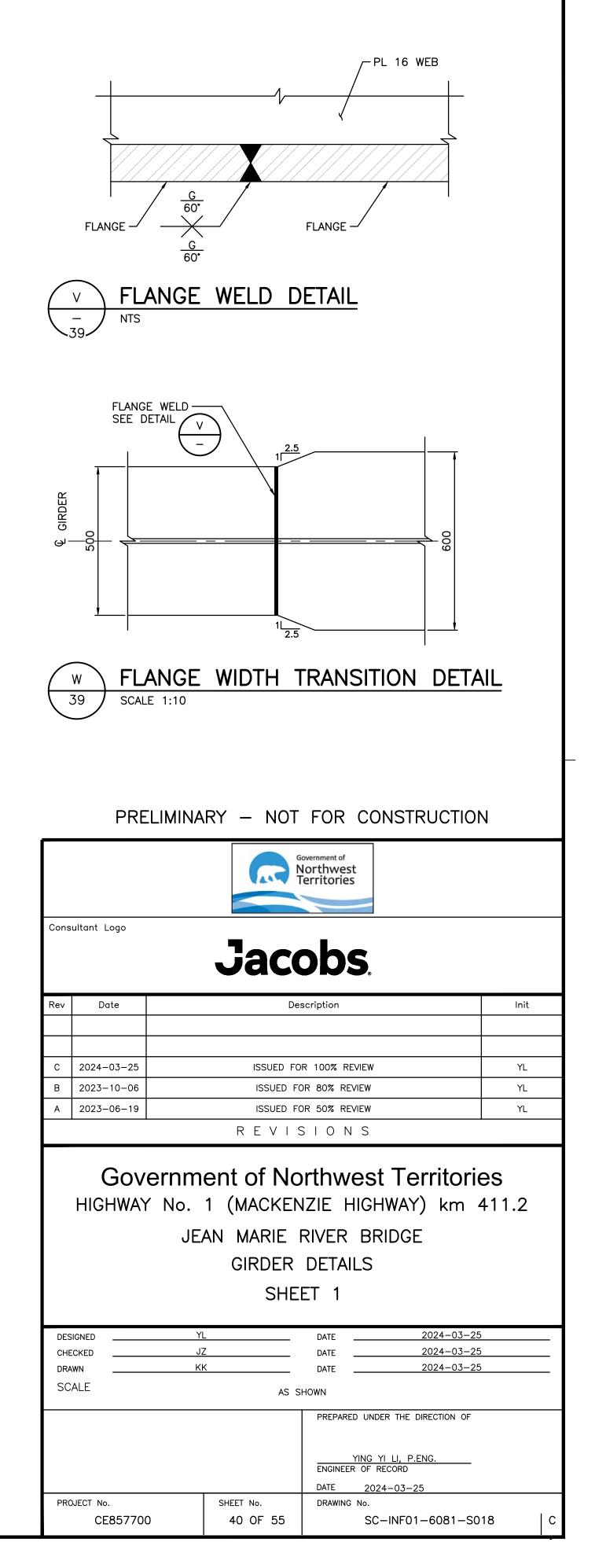
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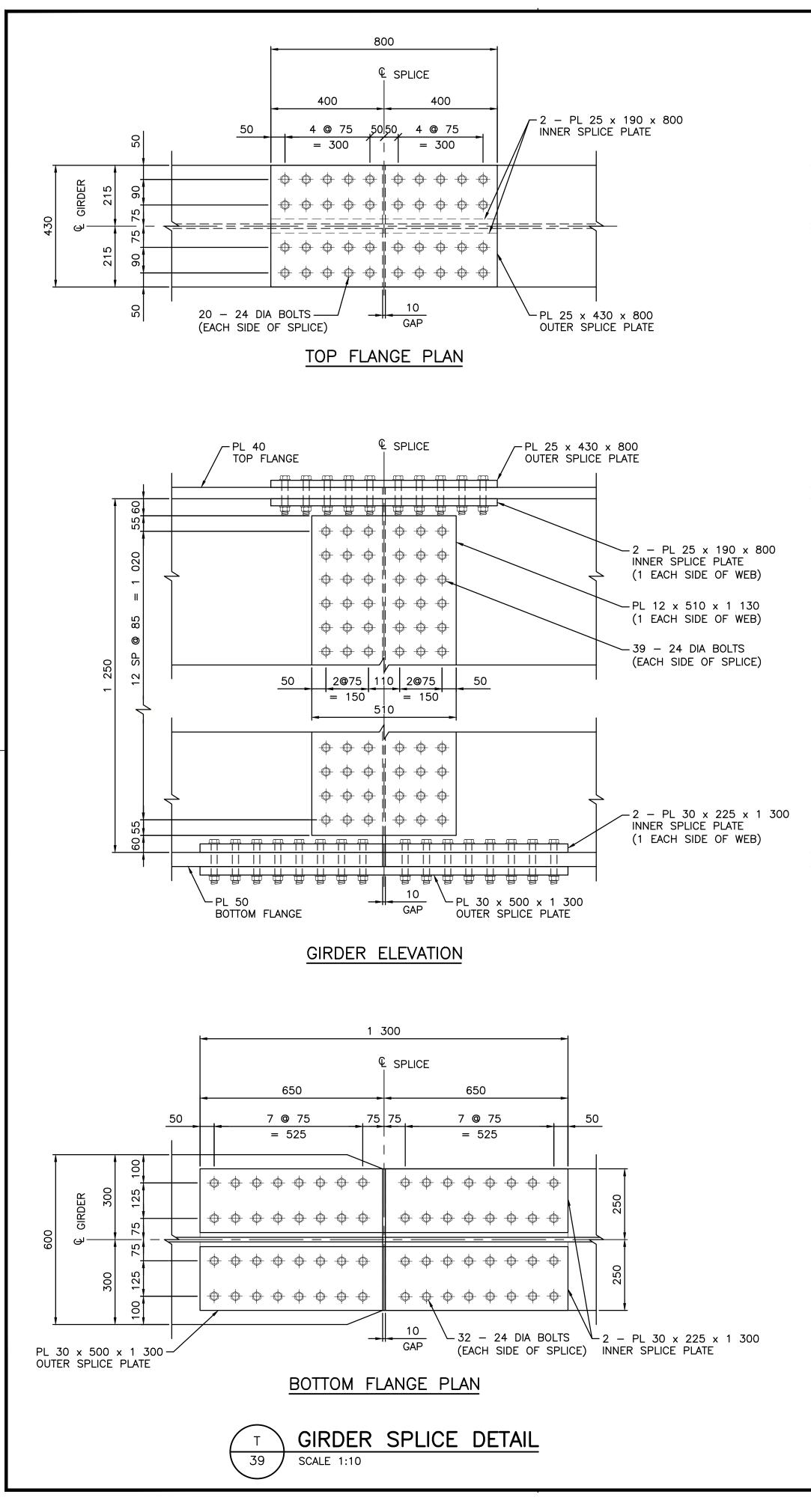


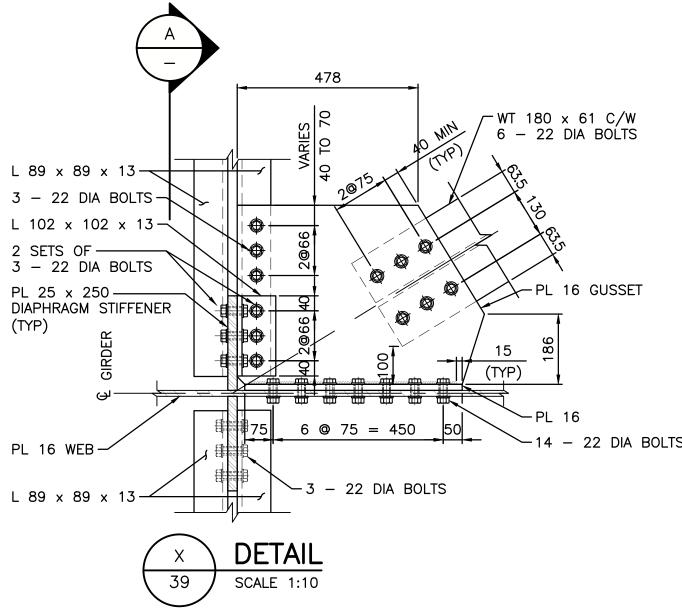




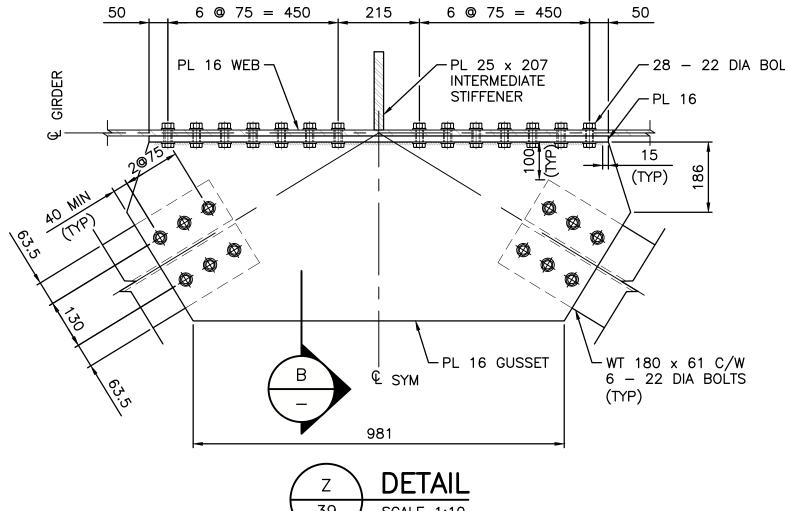


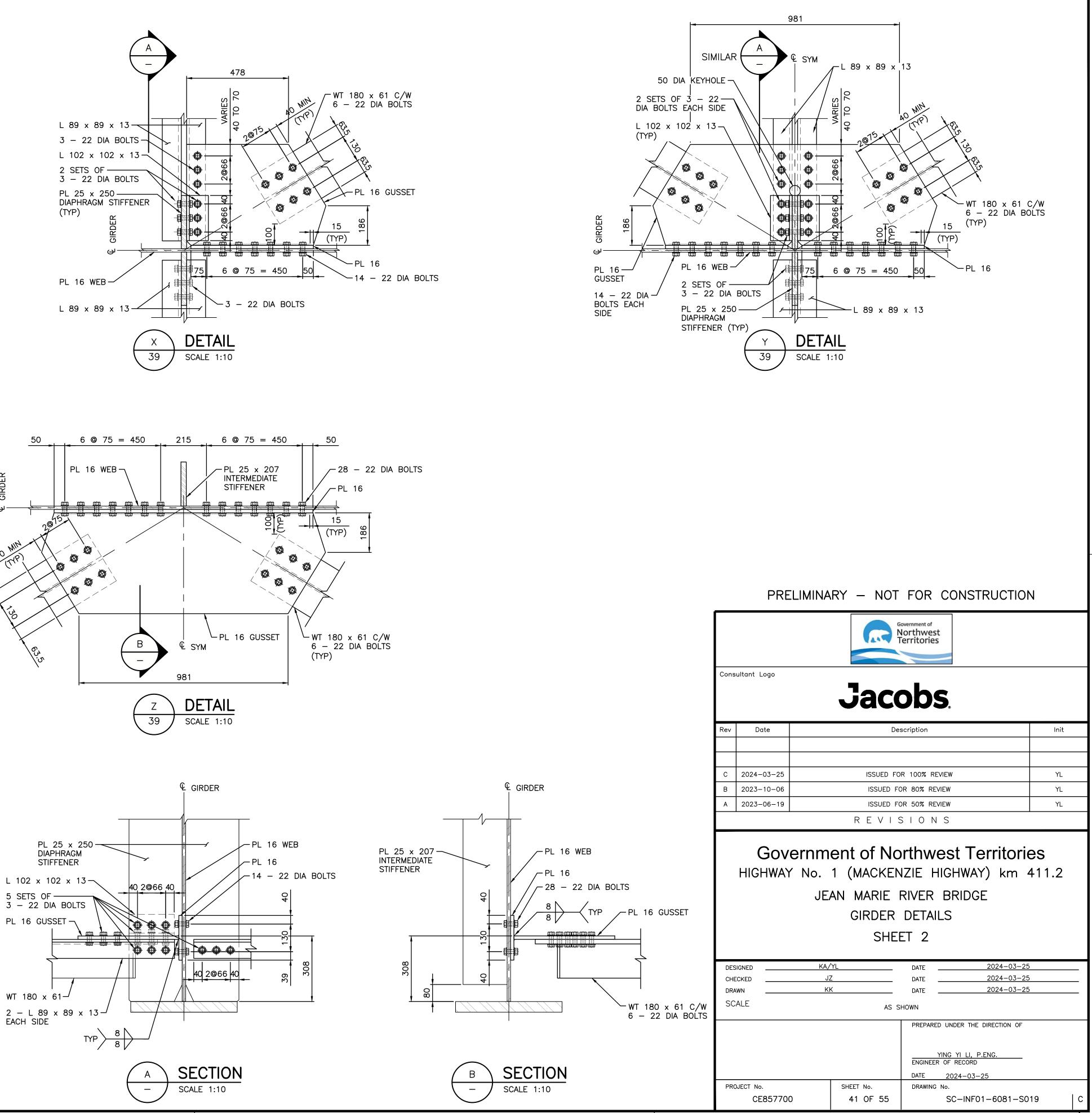


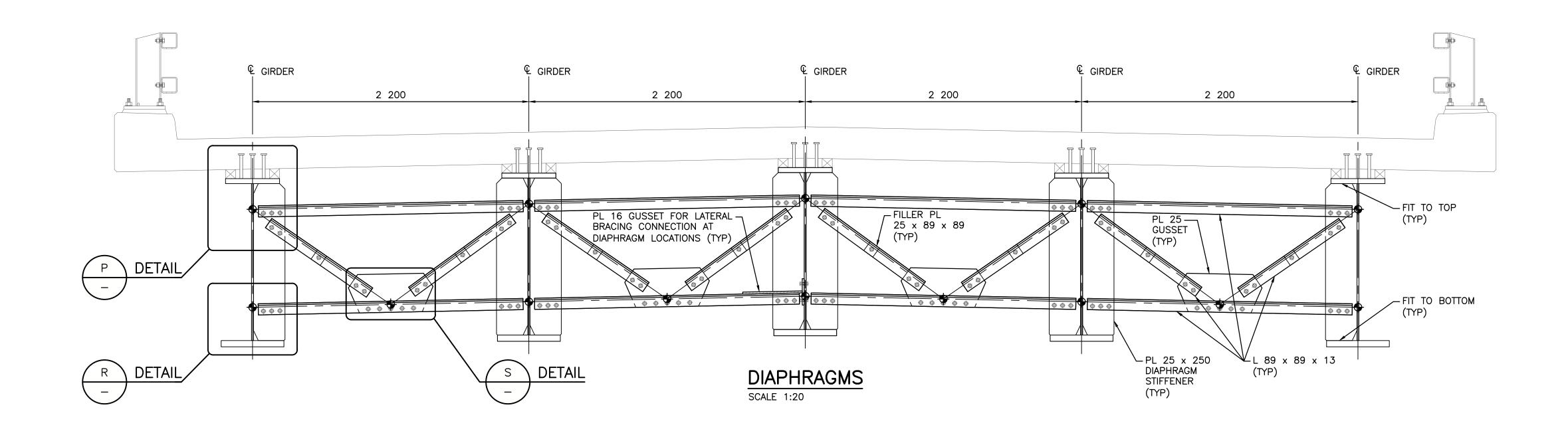


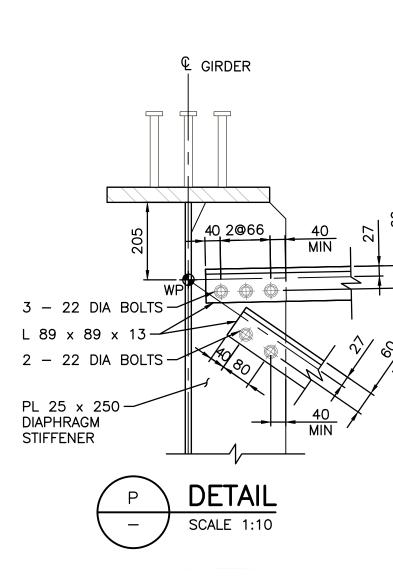


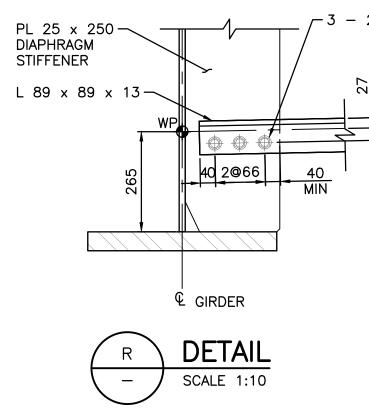
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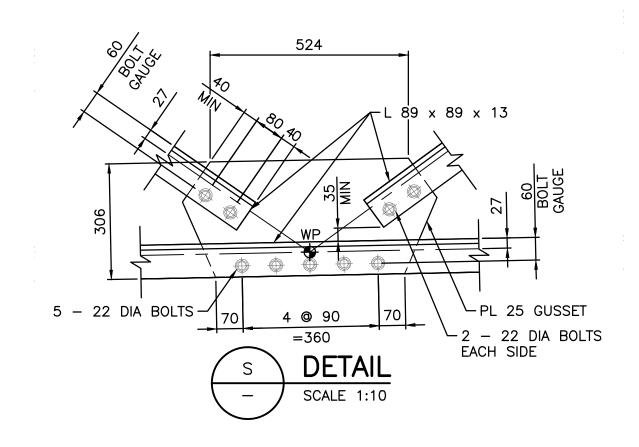




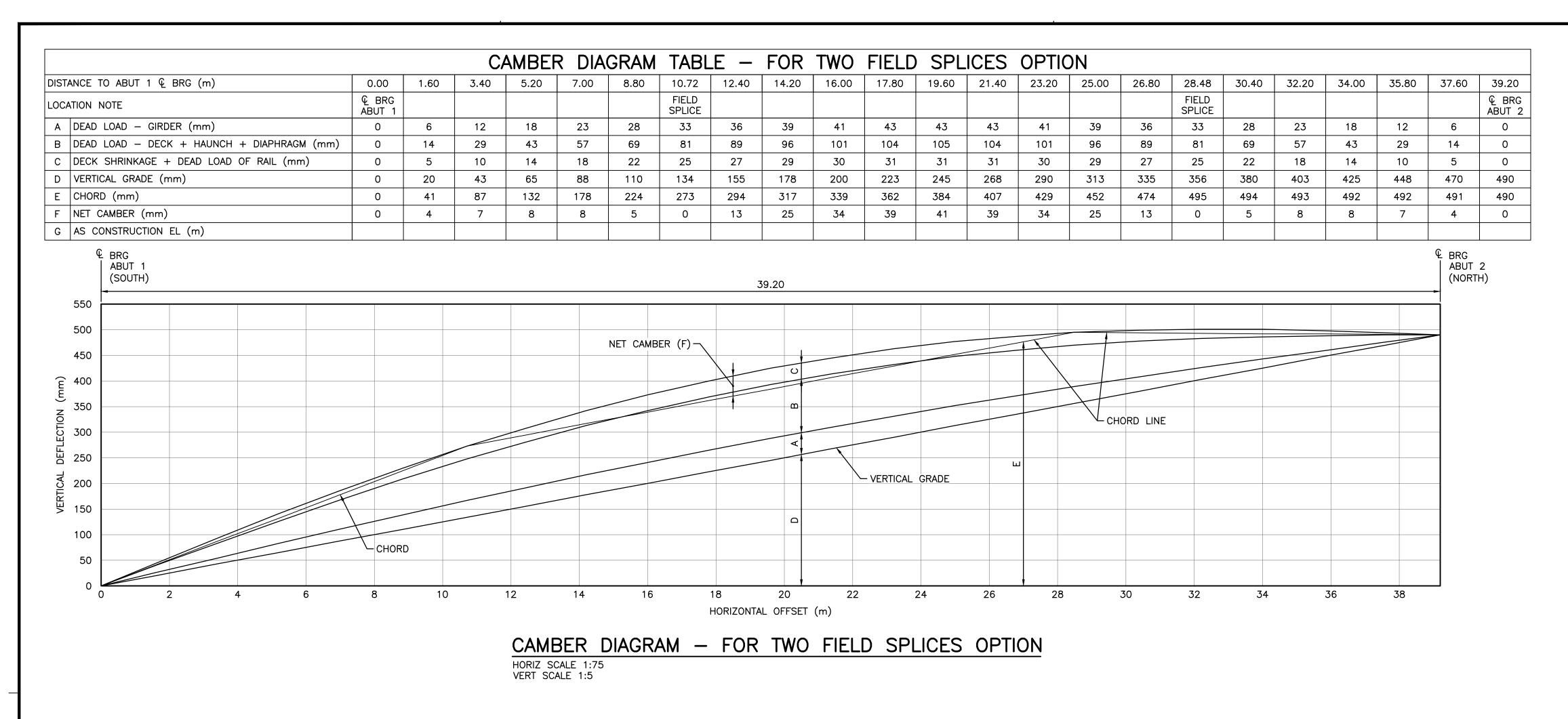


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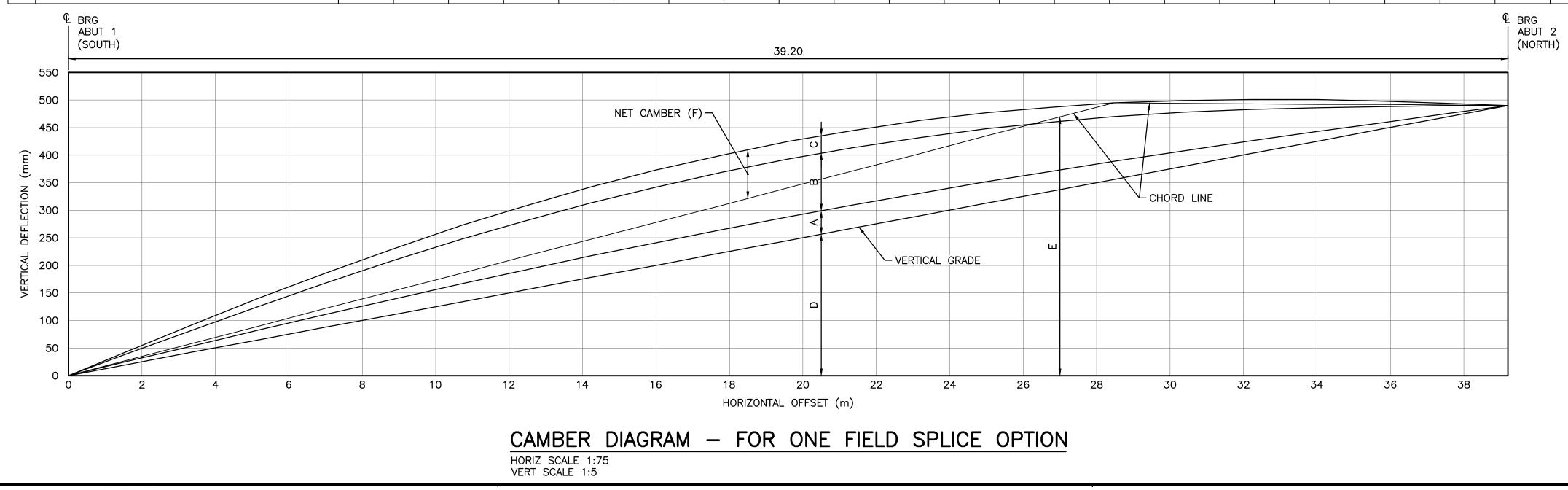
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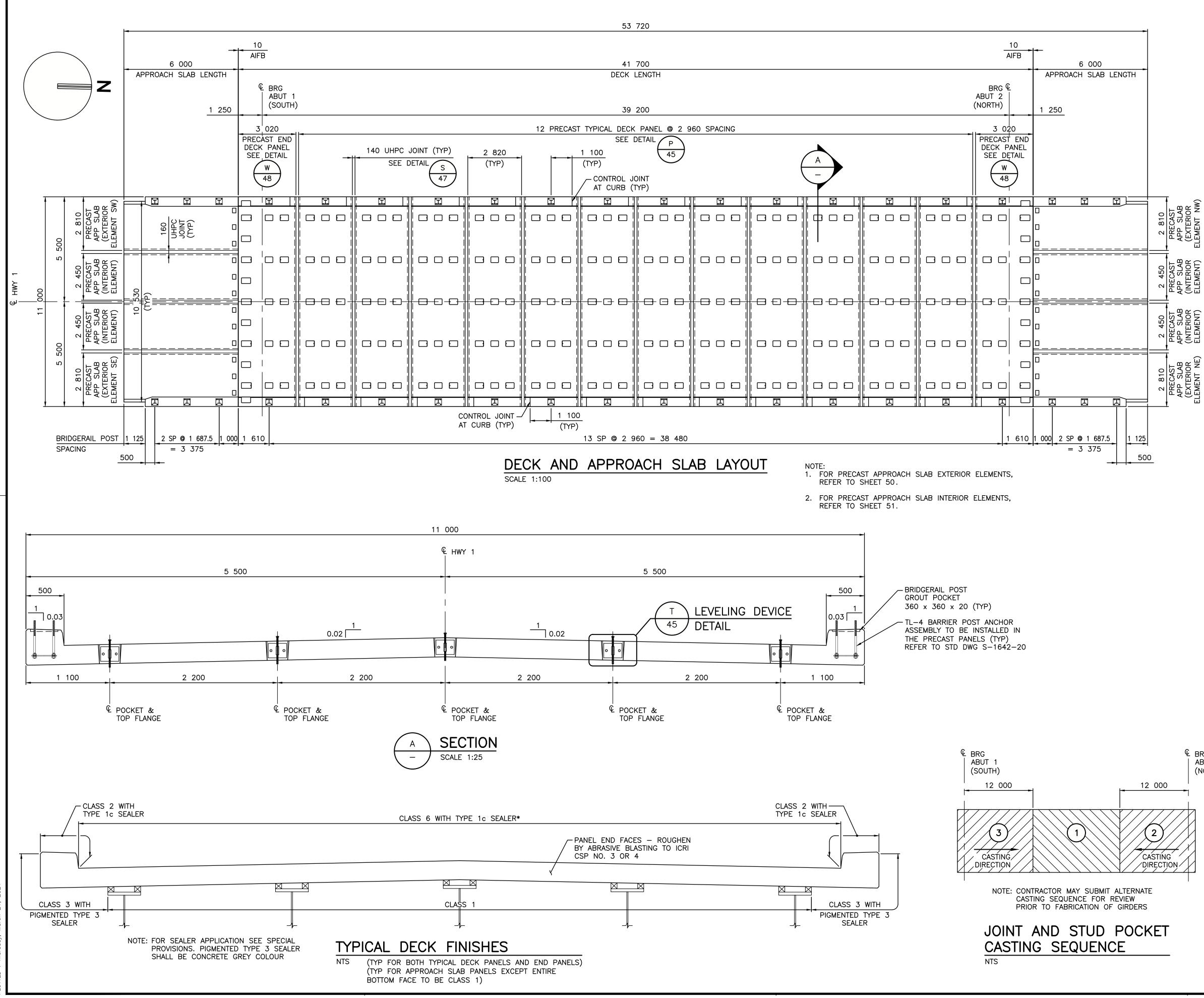


			С	AMBE	R DIA	AGRAN	/ TAB	LE –	FOR	ONE	FIEL	D SP	LICE	OPTIO	Ν								
DISTANCE TO ABUT 1 & BRG (m)	0.00	1.60	3.40	5.20	7.00	8.80	10.72	12.40	14.20	16.00	17.80	19.60	21.40	23.20	25.00	26.80	28.48	30.40	32.20	34.00	35.80	37.60	39.20
LOCATION NOTE	€ BRG ABUT 1						SHOP SPLICE										FIELD SPLICE						€ BRG ABUT 2
A DEAD LOAD – GIRDER (mm)	0	6	12	18	23	28	33	36	39	41	43	43	43	41	39	36	33	28	23	18	12	6	0
B DEAD LOAD – DECK + HAUNCH + DIAPHRAGM (mm)	0	14	29	43	57	69	81	89	96	101	104	105	104	101	96	89	81	69	57	43	29	14	0
C DECK SHRINKAGE + DEAD LOAD OF RAIL (mm)	0	5	10	14	18	22	25	27	29	30	31	31	31	30	29	27	25	22	18	14	10	5	0
D VERTICAL GRADE (mm)	0	20	43	65	88	110	134	155	178	200	223	245	268	290	313	335	356	380	403	425	448	470	490
E CHORD (mm)	0	28	59	90	122	153	186	216	247	278	309	341	372	403	435	466	495	494	493	492	492	491	490
F NET CAMBER (mm)	0	17	34	50	65	76	87	92	95	95	91	84	73	59	42	21	0	5	8	8	7	4	0
G AS CONSTRUCTION EL (m)																							



- 1. NET CAMBER (F) = A + B + C + D E
- 2. "NET CAMBER" FIGURES ARE FOR GIRDER SECTIONS IN ZERO LOAD CONDITIONS.
- FABRICATOR IS RESPONSIBLE FOR MAKING ALLOWANCES SUCH THAT CAMBER ON COMPLETED GIRDER SECTIONS WILL BE WITHIN ALLOWABLE TOLERANCES. ALL PROCEDURES FOR CAMBER ADJUSTMENT MUST BE APPROVED BY THE ENGINEER PRIOR TO USE.
- 4. LONGITUDINAL GIRDER DIMENSIONING (SHOWN HORIZONTAL) IS CORRECT ALONG THE BOTTOM FLANGE AT 20 °C.

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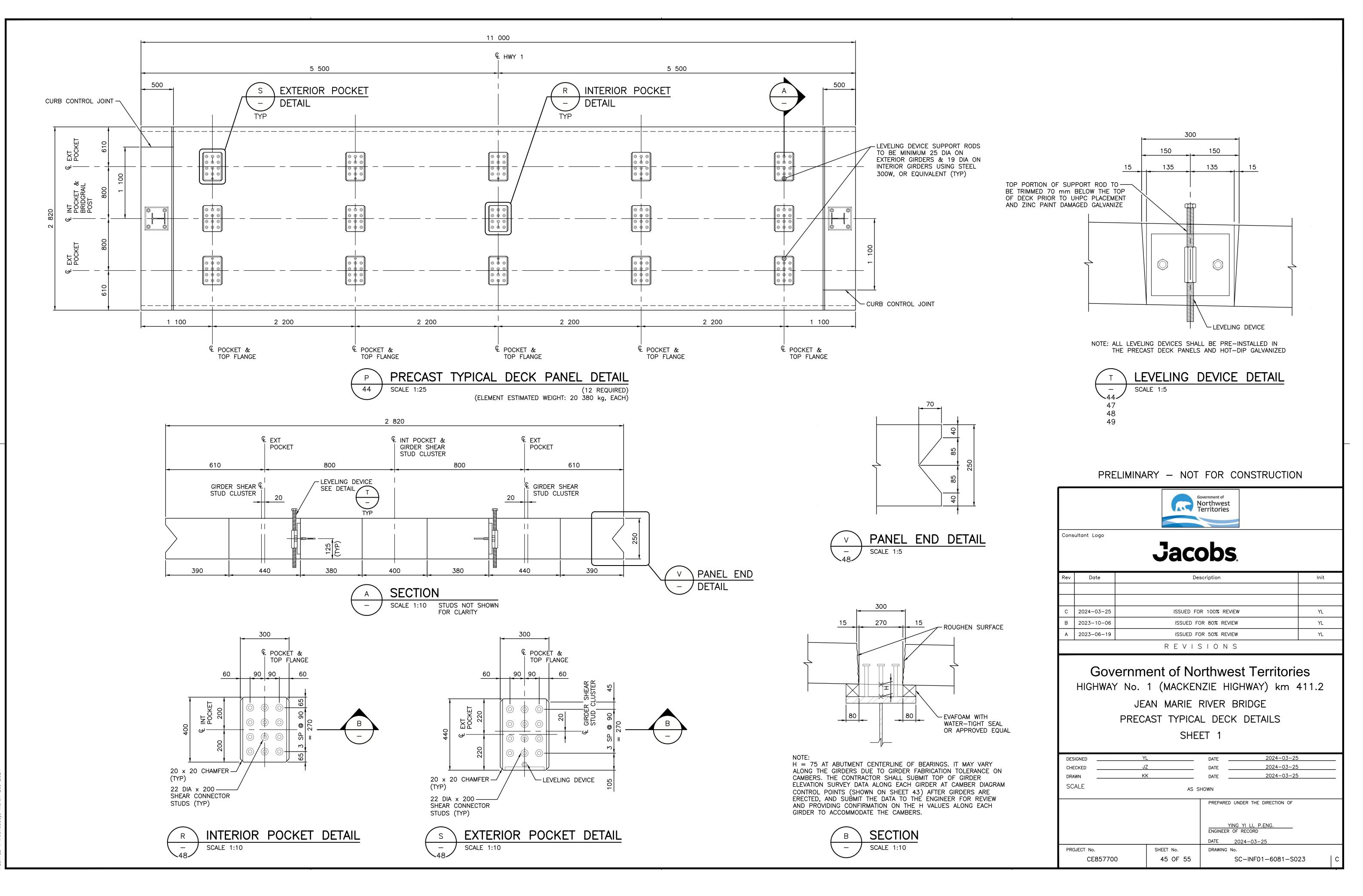


FABRICATION AND ERECTION

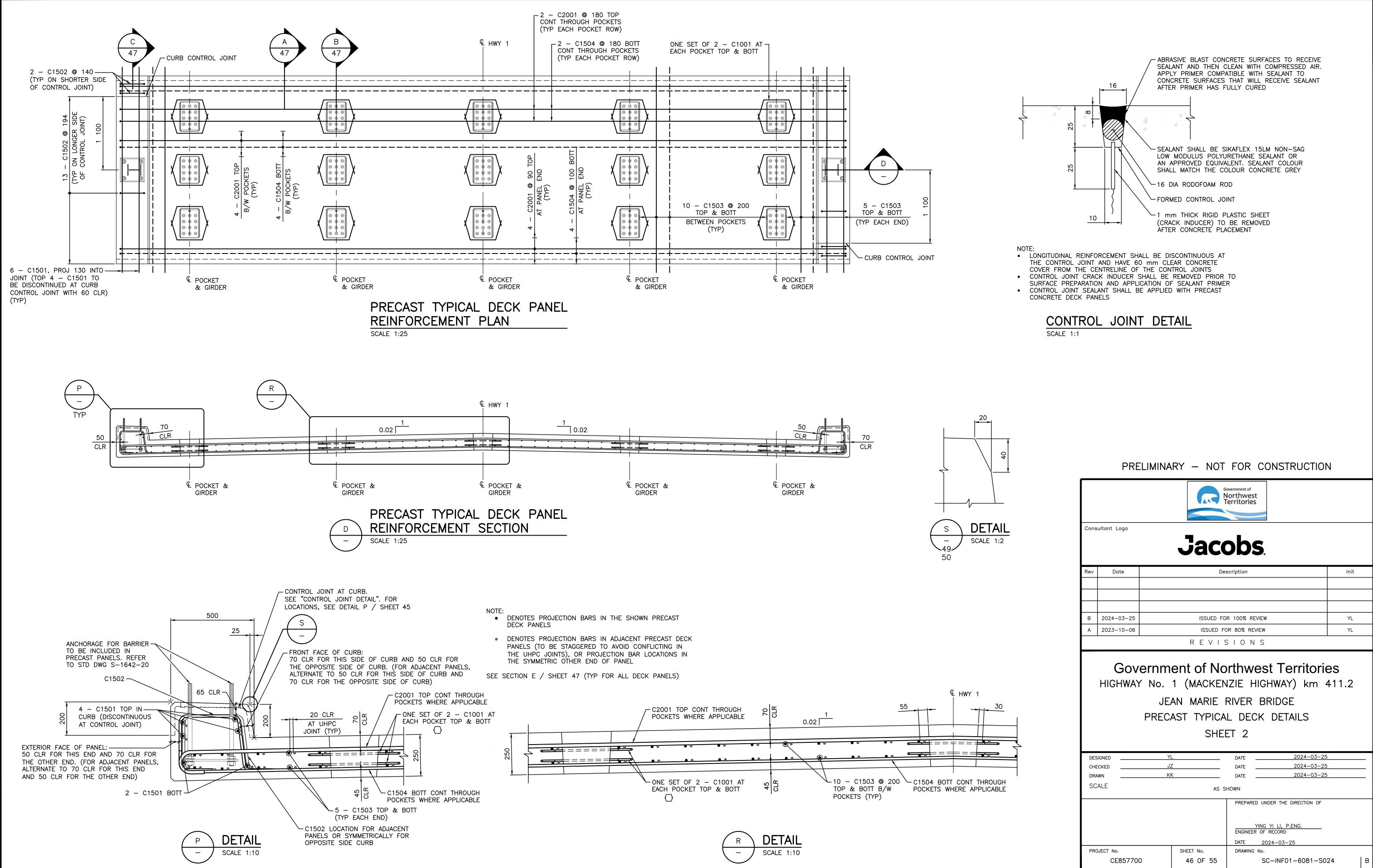
- 1. DECK PANELS SHALL BE INSTALLED AFTER ALL GIRDER DIAPHRAGMS, BRACING AND ABUTMENT DIAPHRAGMS WITH WINGWALLS ARE INSTALLED, AND BACKFILL BEHIND ABUTMENT DIAPHRAGMS ARE COMPLETED.
- 2. DECK PANEL FABRICATION SEQUENCE SHALL BE THE SAME AS INSTALLATION SEQUENCE.
- 3. DECK PANELS SHALL HAVE A MINIMUM AGE OF 60 DAYS AT TIME OF SITE-CASTING THE COMPOSITE JOINTS AND POCKETS.
- 4. PROVIDE TEMPORARY SUPPORTS AS REQUIRED TO ELIMINATE DIFFERENTIAL DEFLECTION BETWEEN ADJACENT DECK PANELS. PROVIDE TEMPORARY SUPPORTS AS REQUIRED AT DECK CANTILEVERS TO MAINTAIN DECK STABILITY DURING THE CONSTRUCTION.
- 5. EVAFOAM TO BE BONDED TO TOP OF STEEL GIRDER, TOP OF ABUTMENT DIAPHRAGMS, AND UNDERSIDE OF PRECAST PANEL WITH COMPATIBLE CONSTRUCTION ADHESIVE. ADHESIVE SHALL PROVIDE A SEAL PREVENTING UHPC LEAKING OUT DURING CASTING.
- 6. EACH DECK PANEL SHALL BE INITIALLY INSTALLED ON GIRDERS WITH 4 LEVELING DEVICE SUPPORT RODS AT 2 EXTERIOR GIRDERS FULLY ENGAGED TO PROVIDE SUPPORTS, THEN EXTEND THE OTHER SUPPORT RODS TO BE ENGAGED AND TO BE ADJUSTED TO FIT REQUIRED HAUNCH DEPTH. ONLY ONE LEVELING DEVICE ADJUSTMENT PERSONAL IS ALLOWED ON THE DECK PANEL UNTIL ALL THE SUPPORT RODS IN THE PANEL ARE FULLY ENGAGED ON THE GIRDERS.
- 7. BEFORE DECK PANELS ARE CONNECTED WITH GIRDERS, ONLY LIGHT CONSTRUCTION EQUIPMENT AND NON-MECHANICAL CONSTRUCTION CART WITH CONSTRUCTION PERSONALS ARE ALLOWED ON DECK PANELS TO TRANSPORT AND PLACE UHPC MATERIAL, AND WITH BALANCED CONSTRUCTION LOADS WHEN OPERTATING OVER DECK OVERHANG REGIONS UNLESS TEMPORARY SUPPORTS AT DECK CANTILEVERS ARE PROVIDED. THE CONTRACTOR TO PROVIDE LOADING TO BE USED ON THE DECK PANELS DURING CONSTRUCTION TO THE ENGINEER FOR REVIEW PRIOR TO STARTING THE WORK.
- 8. THE CONTRACTOR MAY PROPOSE CHANGES TO THE FABRICATION DETAILS SUBJECT TO THE ENGINEER'S APPROVAL. THE CONTRACTOR SHALL MAKE ALL NECESSARY ADJUSTMENTS AND SUBMIT DRAWINGS AND CALCULATIONS. SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE NORTHWEST TERRITORIES, TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION.
- 9. ALL UHPC CASTING FORMWORK SHALL BE WATER-TIGHT TESTED PRIOR TO THE CONCRETE PLACEMENT.

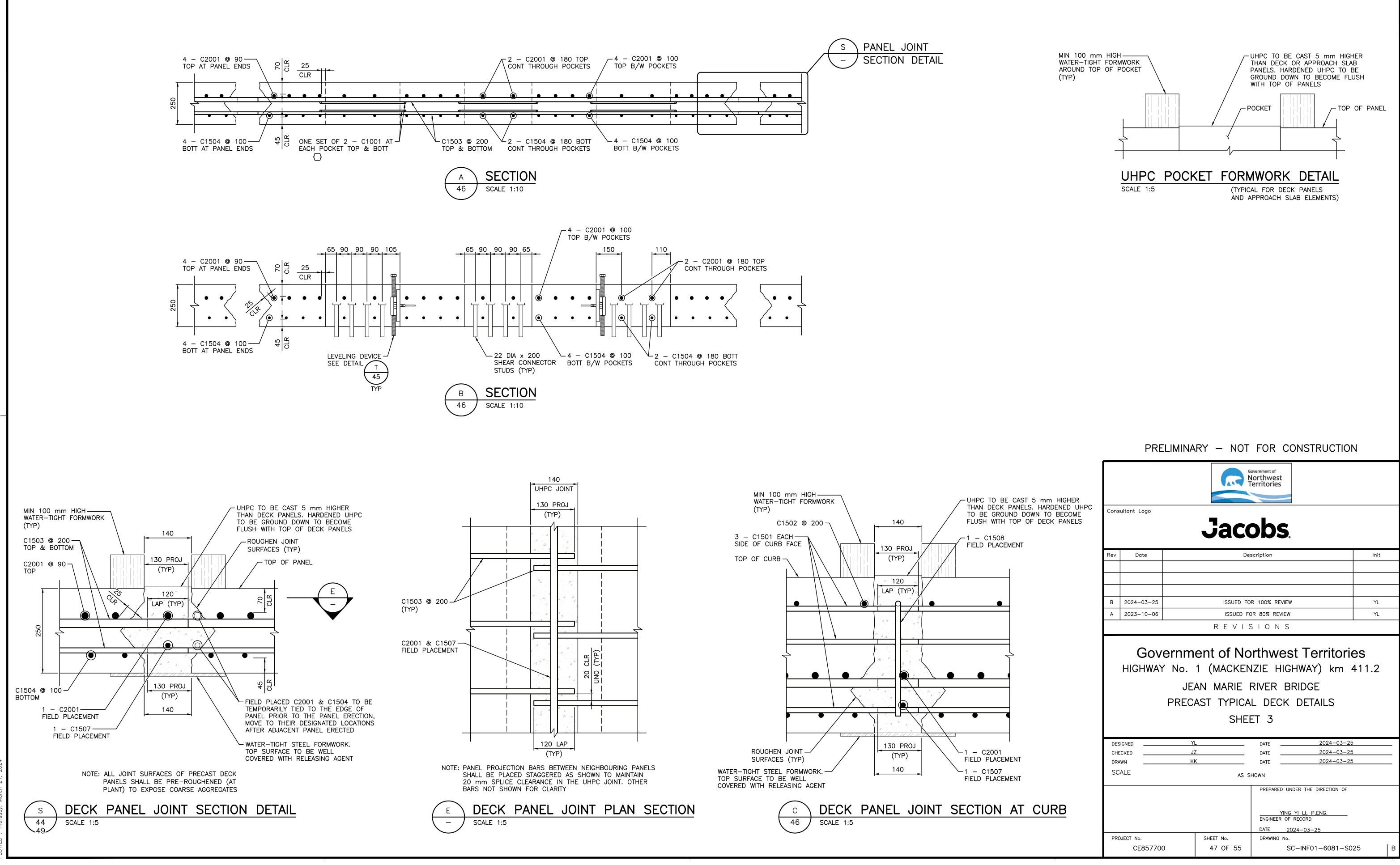
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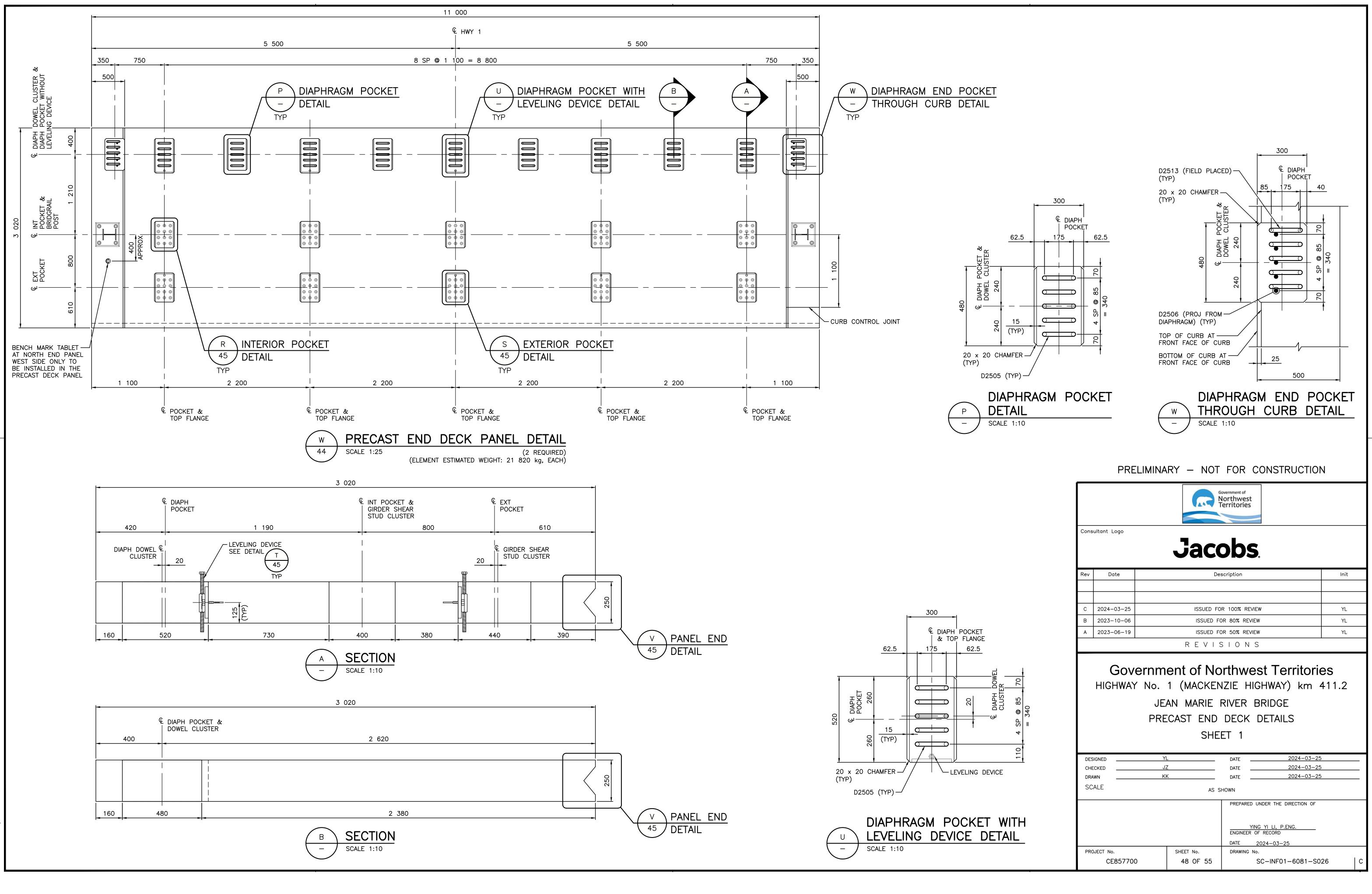
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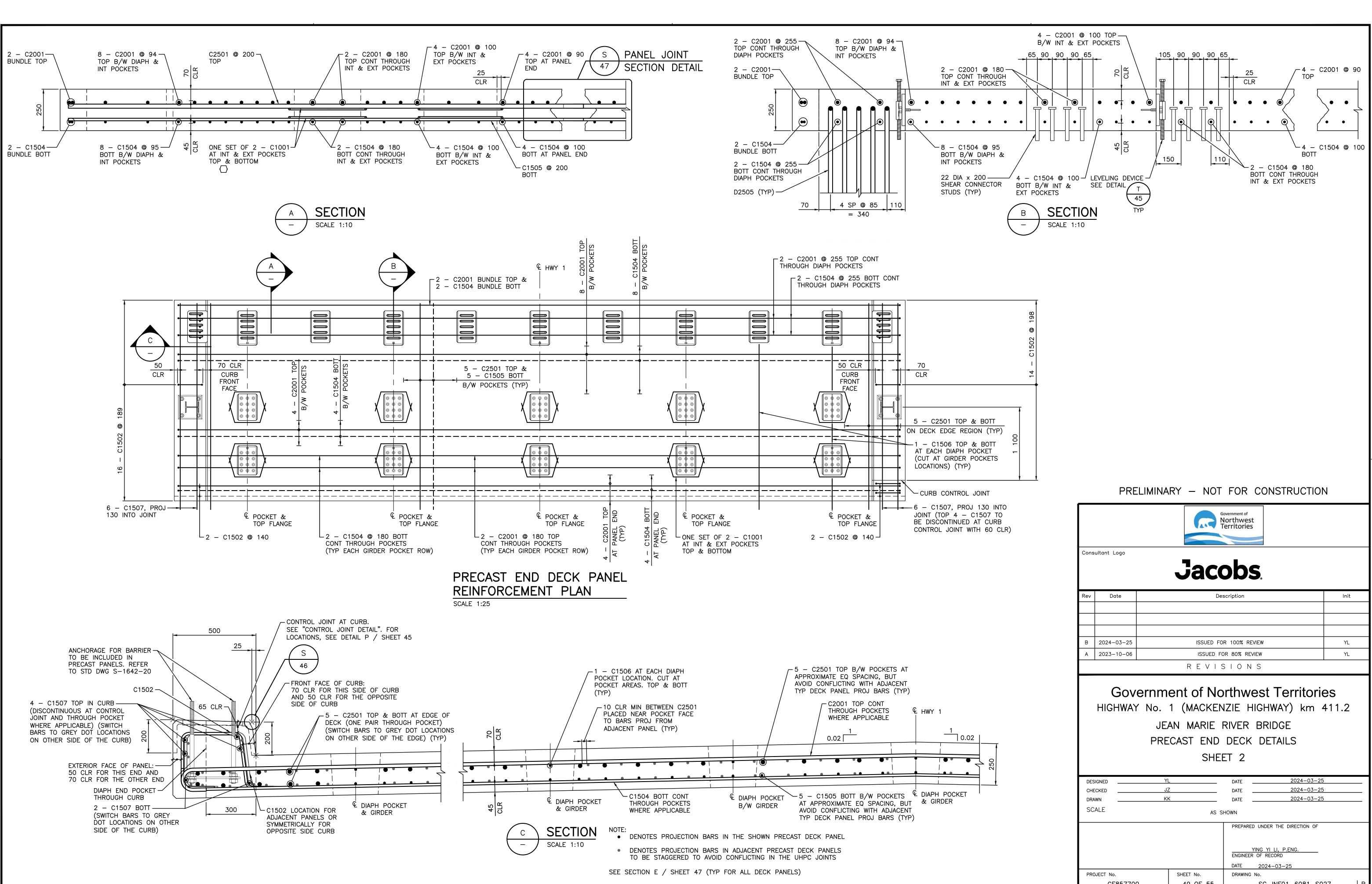
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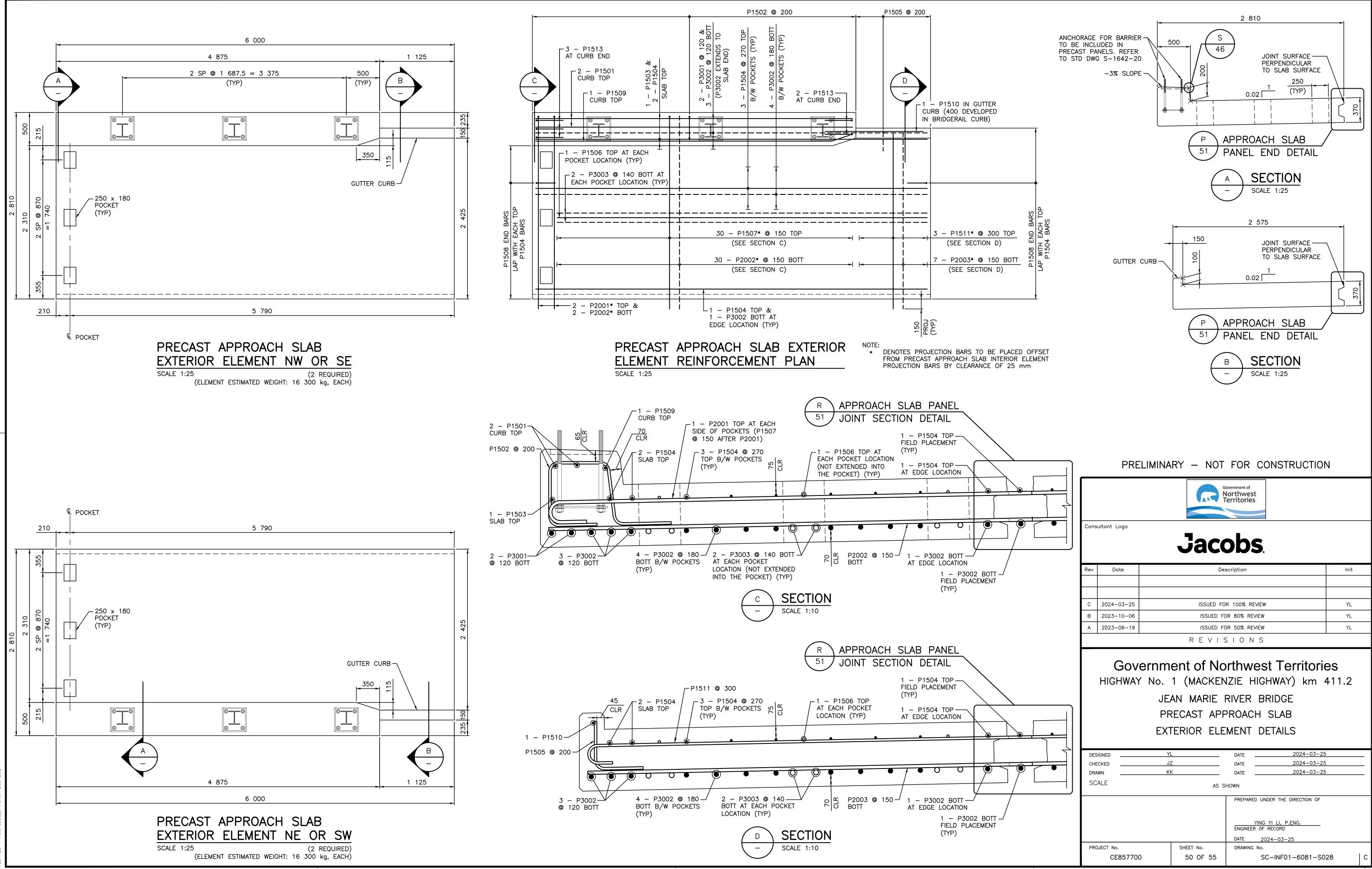




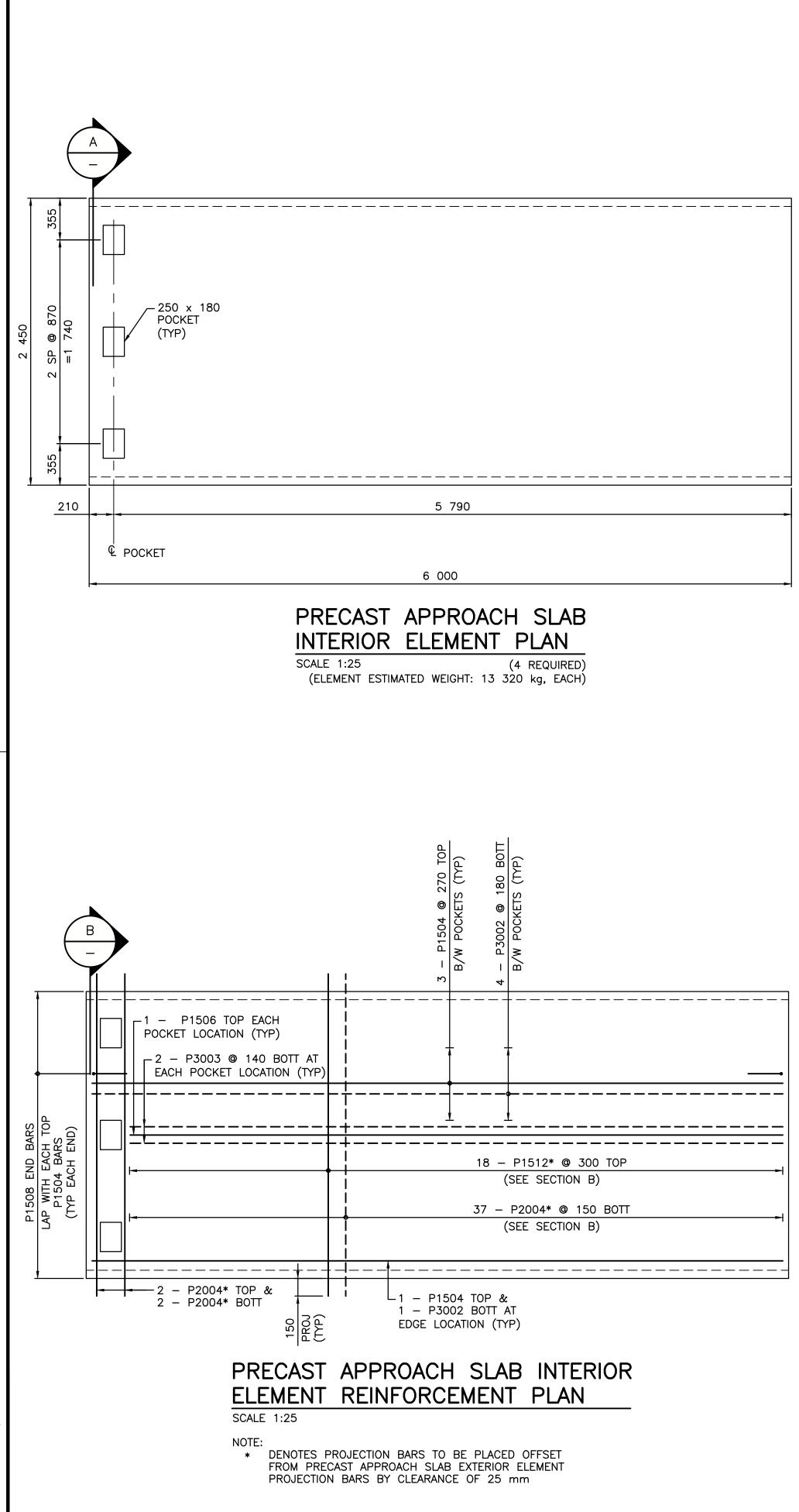
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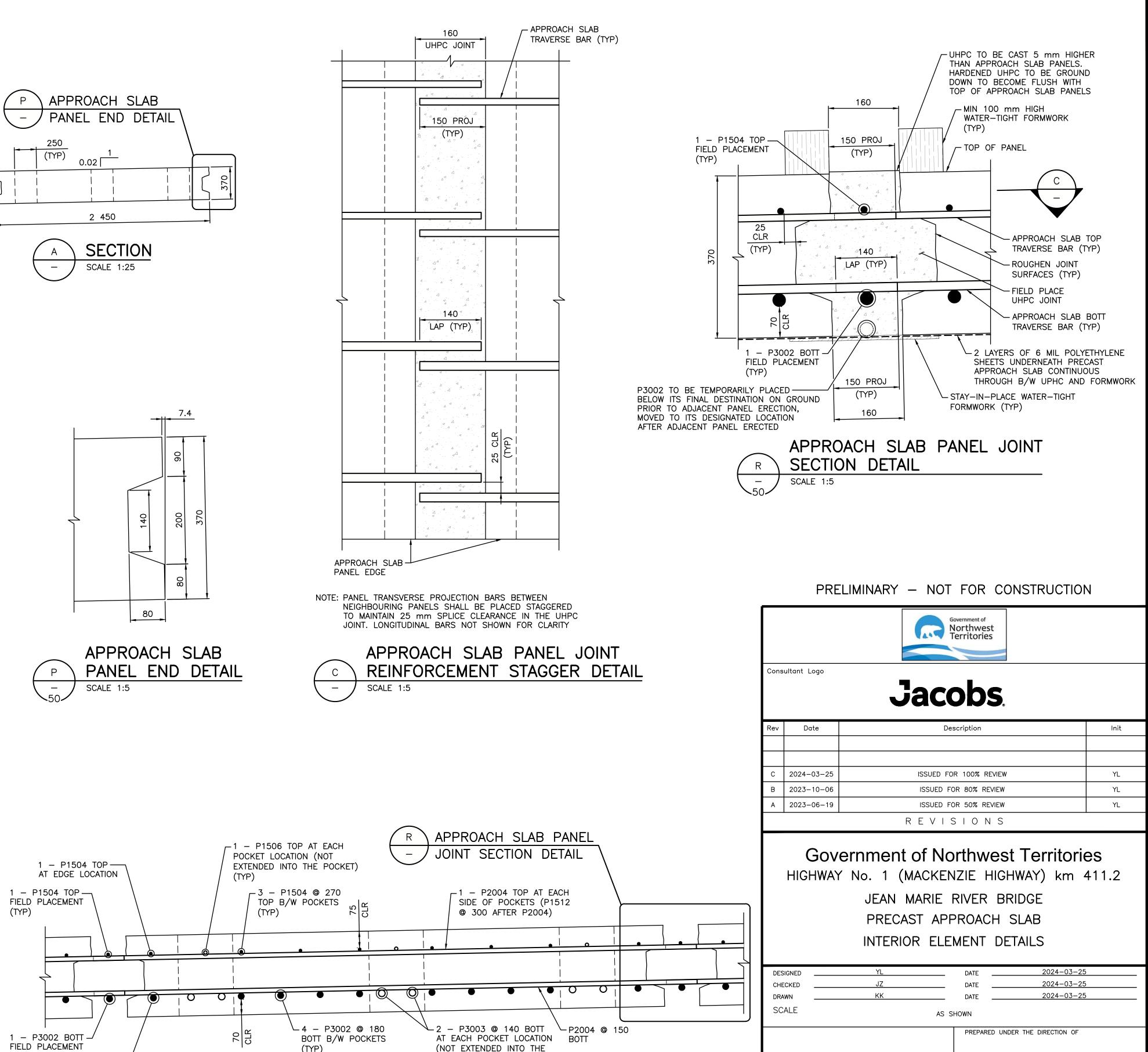


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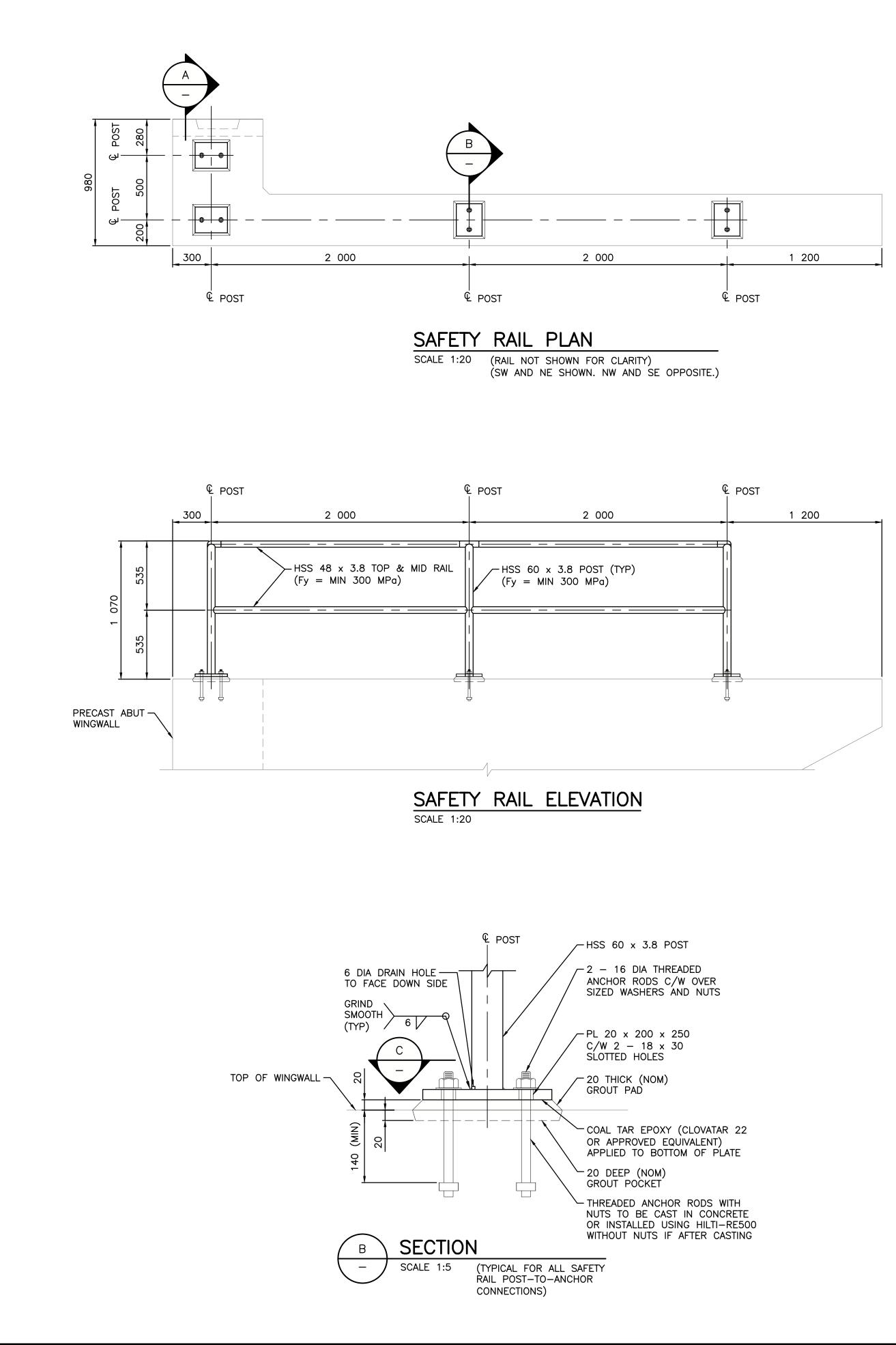
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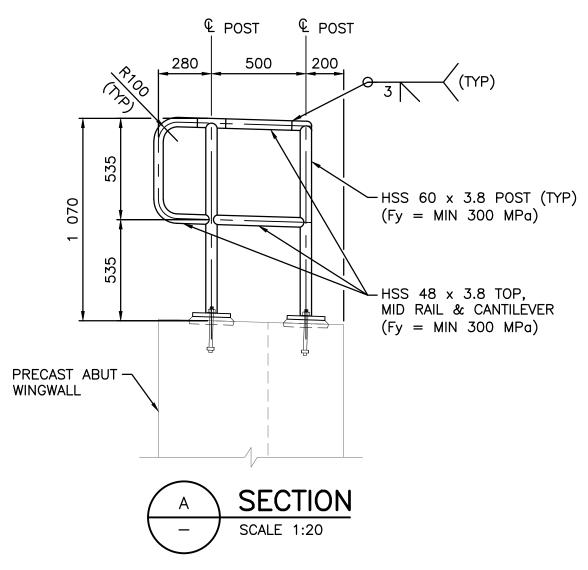
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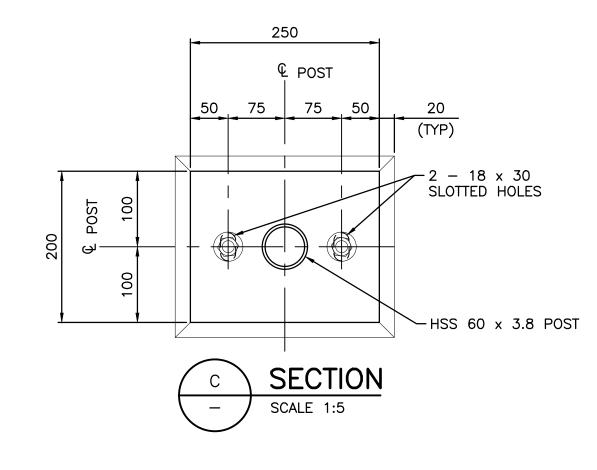
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A 2023-06-19 ISSUED FOR 50% REVIEW YL R E V I S I O N S Government of Northwest Territories HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2 JEAN MARIE RIVER BRIDGE PRECAST APPROACH SLAB INTERIOR ELEMENT DETAILS DESIGNED YL QU24-03-25 DRAWN KK DATE 2024-03-25 DRAWN KK PREPARED UNDER THE DIRECTION OF YING YI LI, P.ENG. ENGINEER OF RECORD DATE 2024-03-25	С	2024-03-25		ISSUED FC	R 100% REVIEW		YL
REVISIONS Government of Northwest Territories HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2 JEAN MARIE RIVER BRIDGE PRECAST APPROACH SLAB INTERIOR ELEMENT DETAILS DESIGNED YL DATE 2024-03-25 DATE 2024-03-25 DATE 2024-03-25 SCALE AS SHOWN PREPARED UNDER THE DIRECTION OF YING YI LI, P.ENG, ENGINEER OF RECORD DATE 2024-03-25 SCALE AS SHOWN PREPARED UNDER THE DIRECTION OF YING YI LI, P.ENG, ENGINEER OF RECORD DATE 2024-03-25 PROJECT NO. SHET NO.	В	2023-10-06		ISSUED FO	DR 80% REVIEW		YL
Government of Northwest Territories HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2 JEAN MARIE RIVER BRIDGE PRECAST APPROACH SLAB INTERIOR ELEMENT DETAILS DESIGNED YL DATE 2024–03–25 CHECKED JZ DATE 2024–03–25 DRAWNN KK SCALE AS SHOWN PREPARED UNDER THE DIRECTION OF YING YI LI, P.ENG. ENGINEER OF RECORD DATE 2024–03–25 SCALE AS SHOWN	A	2023-06-19		ISSUED FO	DR 50% REVIEW		YL
HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2 JEAN MARIE RIVER BRIDGE PRECAST APPROACH SLAB INTERIOR ELEMENT DETAILS DESIGNED YL DATE 2024-03-25 CHECKED JZ DATE 2024-03-25 DRAWN KK DATE 2024-03-25 SCALE AS SHOWN PREPARED UNDER THE DIRECTION OF <u>YING YI LI, P.ENG.</u> ENGINEER OF RECORD DATE 2024-03-25 PROJECT NO. SHEET NO. DRAWING NO.			Į.	REVIS	SIONS	·	
CHECKED JZ DATE 2024-03-25 DRAWN KK DATE 2024-03-25 SCALE AS SHOWN PREPARED UNDER THE DIRECTION OF YING YI LI, P.ENG. ENGINEER OF RECORD DATE 2024-03-25 PROJECT No. SHEET No. DRAWING No.			PF	RECAST APP	PROACH SL	AB	
DRAWN KK DATE 2024-03-25 SCALE AS SHOWN PREPARED UNDER THE DIRECTION OF <u>YING YI LI, P.ENG.</u> ENGINEER OF RECORD DATE 2024-03-25 PROJECT No. SHEET No. DRAWING No.	DES	SIGNED	Y	L	DATE	2024-03-25	
SCALE AS SHOWN PREPARED UNDER THE DIRECTION OF <u>YING YI LI, P.ENG.</u> ENGINEER OF RECORD DATE 2024-03-25 PROJECT NO. SHEET NO.							
YING YI LI, P.ENG. ENGINEER OF RECORD DATE 2024-03-25 PROJECT No. SHEET No.						2027 00 20	
PROJECT No. SHEET No. DRAWING No.					PREPARED UNDER	THE DIRECTION OF	
PROJECT No. SHEET No. DRAWING No.					ENGINEER OF RECO	RD	
	PRC	DJECT No. CE85770	0	SHEET No. 51 OF 55		IF01-6081-S02	

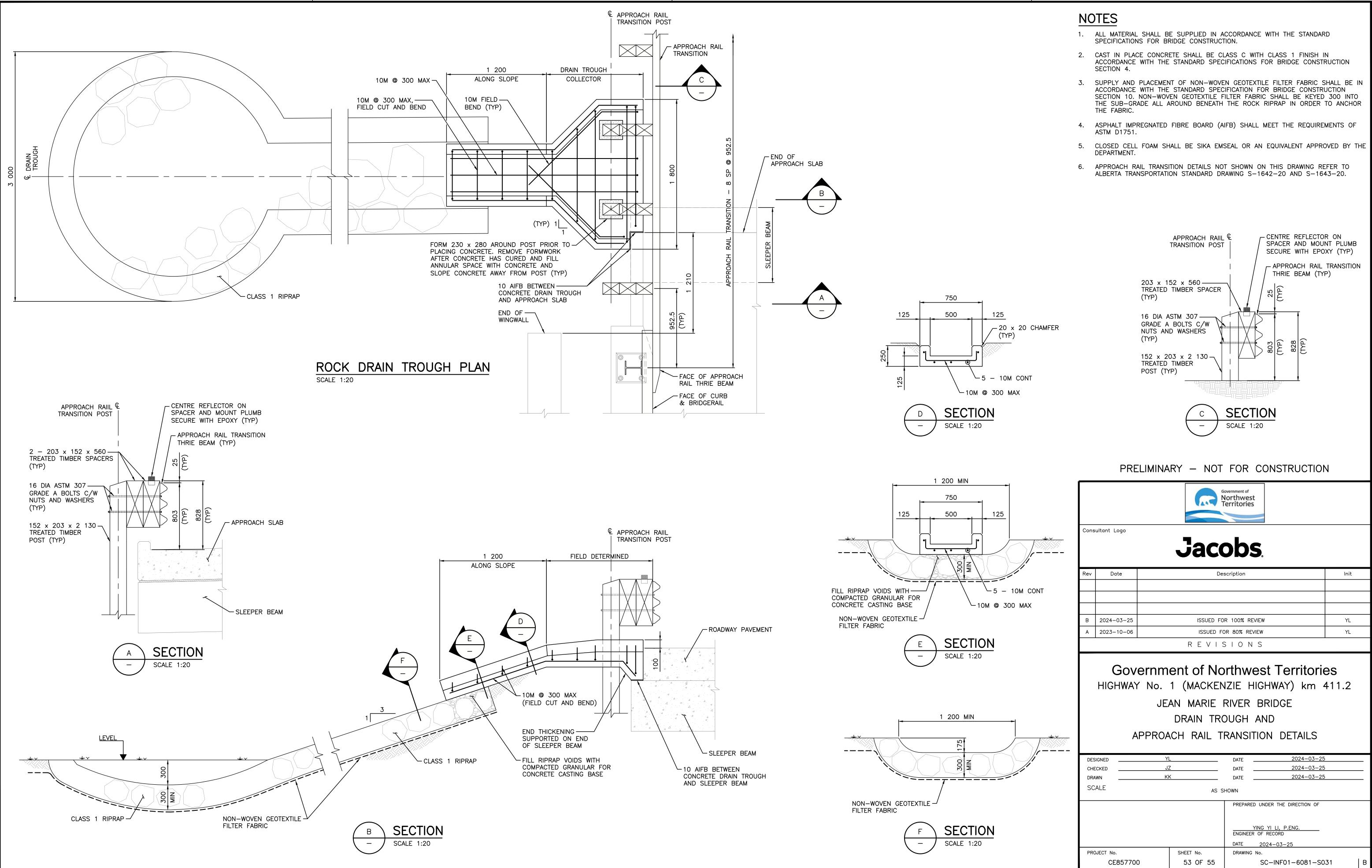






- 1. POST SPACING SHOWN IS CORRECT AT 15°C. LOCATION OF THE POST ANCHOR ROD ASSEMBLIES SHALL BE ADJUSTED TO ACCOUNT FOR INSTALLATION TEMPERATURE.
- 2. ALL REQUIREMENTS OF GNWT SPECIFICATIONS FOR BRIDGE CONSTRUCTION (SSBC) SECTION 12 SHALL BE MET.
- 3. ALL STEEL SHALL CONFORM TO CSA SPECIFICATION G40.21M GRADE 300W. ANCHOR RODS SHALL CONFORM TO ASTM F1554 GRADE 55. ALL NUTS AND WASHERS SHALL CONFORM TO ASTM A563 AND ASTM F436 RESPECTIVELY.
- 4. ALL WELDING SHALL CONFORM TO CURRENT AWS SPECIFICATION D1.1 AND D1.5.
- 5. ALL MATERIALS SHALL BE HOT DIP GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123/A123M AND ASTM F2329 UNLESS NOTED OTHERWISE.
- 6. REPAIR OF GALVANIZED SHALL BE COMPLETED AS PER THE SSBC SECTION 6.2.7.3.3, REPAIR OF GALVANIZED AND METALLIZED MATERIAL.
- 7. ALL EXPOSED CUT TUBE ENDS SHALL BE GROUND SMOOTH.
- 8. THE BOTTOM SURFACE OF THE BASE PLATES SHALL BE COATED WITH CLOVATAR 22 OR AN APPROVED EQUIVALENT SUITABLE FOR APPLICATION ON GALVANIZED STEEL TO PREVENT CONTACT BETWEEN THE ZINC AND THE GROUT. THE COLOUR SHALL BE MEDIUM GREY.
- 9. ALL POSTS SHALL BE VERTICAL.

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		′No.	REVIS ent of No 1 (MACKEN AN MARIE	orthwest	VAY) km 4 DGE	es
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MARK	SIZE	TYPE	×	' SEA ' Y	z	LENGTH	QTY IN 1 PRECAST UNIT ABUT 1 SE OR SW	ABUT 2 NE OR NW	QTY IN 1 PRECAST UNIT MID ELEMENT ABUT 1 OR ABUT 2	TOTAL QTY IN 6 PRECAST UNITS	QTY FIELD PLACEMENT IN PILE POCKETS ABUT 1		QTY FIELD PLACEMENT IN 4 CONNECTIONS	TOTAL QTY IN FIELD PLACEMENT	MASS IN 6 PRECAST UNITS (kg)	MASS IN FIELD PLACEMENT (kg
A1501	15	P	1 695			1 975	ELEMENT 63	ELEMENT 57	48	336			24	24	1 042	74
A1502	15	R	120	770	1 670	2 560	72	72	24	336			21	21	1 350	
A1503	15	G	322	1 220	1 0/0	2 762	36	24	12	144					624	
A1504	15	G	1 695	140		1 975	16	12	12	80					248	
A1505	15	G	1 695	200		2 095	4	4	12	16					53	
A1506	15	G	1 695	250		2 195	3	3		12					41	
A1507	15	E	762	140		902				12		16		16		23
A1507	15	E	720	140		860						16		16		22
A1500	15	G	1 756	140		2 036			16	32		10		10	102	
A1510	15	G	982	140		1 262		6	10	12					24	-
A1510	15	G	1 024	140		1 304		6		12					25	
A1512	15	E	1 695	140		1 835		0		12	56	32		88	25	254
A1312 A2001	20	G	1 034	1 320		3 674	42	44	32	236	50	52	16	16	2 042	138
A2001	20	G	992	1 320		3 632	42	44	32	236			16	16	2 042	137
A2002	20	P	1 695	1 520		2 095	36	48	12	192			10	10	947	
A2003	20	STR	1 095			4 315	14	14	12	56					569	-
A2004	20	M	1 320			1 520	14	12		24					86	-
A2005	20	E	3 700	250		3 950	4	4		16					149	
A2008	20	STR	3 700	250		2 400	+	4	10	20					149	
A2007	20	STR				3 030			16	32					228	
A2008	20	STR				3 030			4	8					57	-
A2501	25	E	4 315	400		4 715	8	8	+	32					592	-
A2502	25	STR	4 515	400		4 315	8	8		32					542	-
A2502	25	E	400	590		990	6	6		24					93	
A2503	25	Ē	510	400		910	0	0		0	8	8		16	95	57
A2504	25	STR	510	+00		3 030			24	48				10	571	
A2505	25	STR				2 400			10	20					188	-
A2001	30	STR				4 315	4	4		16					379	+
A3002	30	STR				3 975	8	8		32					699	
A3002	30	E	4 315	490		4 805	4	4		16					422	+
A3005	30	STR	+ 515	730		3 030		T	8	16					266	-
A3005 A3006	30	STR				2 400			<u>ک</u>	8					106	-
H1501	15	E	650	760		1410	4	4	4	16					35	
H1502	15	G	370	760		1890	2	2	3	10			2	2	30	6
H1502	15	G	400	952		2304	<u>ک</u>	۷	5	10			8	8		29
H1503	15	STR	+00	352		1194							8	8		15

BAR	LIST:	ABUT	MEN [®]	T DIAI	PHRAC	SMS								BAR	LIST:	ABUT	MENT	WIN	GWALI	S				
MARK	SIZE	TYPE	V	w	x	Y	Z	LENGTH IN	QTY 1 PRECAST UNIT	QTY IN 2 PRECAST UNITS	TOTAL QTY IN FIELD PLACEMENT	MASS IN 2 PRECAST UNITS (kg)	MASS IN FIELD PLACEMENT (kg)	MARK	SIZE	TYPE	w	x	Y	Z	LENGTH	QTY IN 1 PRECAST UNIT ABUT 1	QTY IN 1 PRECAST UNIT ABUT 2	IN 4 PRECAST
D1501	15	Р			1 422			1 702	24	48		128										SE OR SW	NE OR NW	UNITS
D2001	20	J		5 155	10 310	103	103	10 310	5	10		243										ELEMENT	ELEMENT	
D2002	20	Т	300	834	670	200	715	2 269	52	104		556		W1501	15	Р		585			865	6	6	24
D2003SS	20	S		300	160	750		2 260	12	24		128		W1502	15	Р		1 205			1 485	3	3	12
D2501	25	G			585	1 211		3 007			16		189	W1502	15	F	180	4 740	2 551	630	6 193	2		4
D2502	25	н			585	1 422		4 574	53	106		1 903		W1503	15	F	180	4 740	2 687	630	6 259		2	4
D2503	25	STR						6 000	10	20		471		W1504	15	E		3 400 (AVE)	750		4 280	14	14	56
D2504	25	STR						3 200	10	20		251		W1505	15	R		710	1 635	250	2 595	3	3	12
D2505	25	G			200	920		2 040	45	90		721			10				1 600	230		5		
D2506	25	С			74	750	400	1 154	10	20		91		W1506	15	G		255	(AVE)		3 455	15	15	60
D2507	25	J		3 900	7 800	78	78	7 800	6	12		367		W1507	15	G		295	620		1 535	2	2	8
D2508	25	STR						4 600	12	24		433		W1508	15	G		255	620		1 495	15	15	60
D2509	25	G			350	3 000		6 350	4	8		199		W1509	15	R		810	820	250	1 880	3	3	12
D2510	25	STR						3 000	8	16		188		W2001	20	Н		535	832		3 134	2	2	8
D2511	25	G			535	3 000		6 535	18	36		923		W2002	20	н		535	750		2 970	6	6	24
D2512	25	STR						3 500	14	28		385		W2003	20	G		2 850	300		3 450	5	5	20
D2513	25	R			200	1 200	450	1 850			20		145	W2501	25	G		585	2 025		4 635	10	10	40
										PLAIN TOTAL	IN 2 PRECAST	ELEMENTS (kg):	6 860	W2502	25	н		535	1 210		4 290	8	8	32
										SS TOTAL	IN 2 PRECAST	ELEMENTS (kg):	128	W2503	25	н		350	1 210		3 920	2	2	8
										PLAIN TO	DTAL IN FIELD F	PLACEMENT (kg):	334	W2504	25	E		1 160	400		1 560	4	4	16
														W2505	25	E		1 210	400		1 610	6	6	24
														W2506	25	н		585	1 742		4 150	1	1	4
														W2507	25	E		3 400 (AVE)	400		3 800	14	14	56
																		/	1				IN 4 PRECAST	 ELEMENTS (kg):

BAR	LIST:	SLEE	IPER	BEAM	S					
MARK	SIZE	TYPE	w	x	Y	z	LENGTH	QTY IN 1 PRECAST UNIT	QTY IN 2 PRECAST UNITS	MASS (kg)
B1001	10	L		80	440 (AVE)	180	1 220	32	64	61
B1501	15	G		790	440 (AVE)		1 670	16	32	84
B1502	15	Н		790	450 (AVE)		2 760	45	90	390
B1503	15	G		760	500		1 760	4	8	22
B2001	20	Y		790			1 1 30	8	16	43
B2002	20	STR					1 700	8	16	64
B2003	20	STR					10 440	4	8	197
B2004	20	STR					10 440	2	4	98
B3001	30	J	5 220	10 440	104	104	10 440	4	8	459
B3002	30	STR					10 440	4	8	459

PLAIN TOTAL IN 6 PRECAST ELEMENTS (Kg): PLAIN TOTAL IN FIELD PLACEMENT (kg):

755

PLAIN TOTAL IN 4 PRECAST ELEMENTS (kg):

PLAIN TOTAL IN 2 PRECAST ELEMENTS (kg): 1 877

NOTES

- 1. BARS DENOTED AS 'SS' SHALL BE SOLID STAINLESS REINFORCING STEEL.
- 2. DIAMETERS OF ALL BENDS AND DETAILS OF ALL HOOKS, UNLESS NOTED OTHERWISE SHALL CONFORM TO THE RECOMMENDED SIZED DETAILED IN THE CURRENT EDITION OF THE REINFORCING STEEL STANDARD PRACTICE MANUAL PUBLISHED BY THE REINFORCING STEEL INSTITUTE OF CANADA.
- 3. WHERE SPLICES ARE SPECIFICALLY DETAILED ON THESE DRAWINGS THE MINIMUM SPLICE LENGTHS SHALL BE THE LONGER LENGTH SHOWN IN THE SPECIFIC DETAIL OR AS SUMMARIZED IN THE FOLLOWING TABLE. FOR ALL OTHER CASES APPROVAL IN WRITING FROM THE ENGINEER IS REQUIRED.

REINFORCING SPLICE LENGTHS (UNO)								
BAR SIZE	SPLICE LENGTH							
10M	450							
15M	650							
20M	850							
25M	1300							
30M	1550							
35M	1800							

PRELIMINARY - NOT FOR CONSTRUCTION Government of Northwest Territories MASS IN 4 PRECAST Consultant Logo Jacobs. UNITS (kg) 33 28 Description Date Init Rev 39 39 376 49 A 2024-03-25 ISSUED FOR 100% REVIEW YL 325 _____ REVISIONS 141 Government of Northwest Territories 35 HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2 59 168 JEAN MARIE RIVER BRIDGE 162 728 BAR LIST 539 SHEET 1 123 98 152 2024-03-25 DATE DESIGNED YL 65 2024-03-25 JZ CHECKED DATE 2024-03-25 DRAWN KK DATE 835 SCALE AS SHOWN 4 014 PREPARED UNDER THE DIRECTION OF YING YI LI, P.ENG. ENGINEER OF RECORD DATE 2024-03-25 PROJECT No. SHEET No. DRAWING No. CE857700 54 OF 55 SC-INF01-6081-S032

BAR LIST: DECK PA					
DAN LIST. DECK FF	ANELS			BAR LIST: APPROACH SLAB PANELS	
MARK SIZE TYPE V	W X Y Z L	LENGTH IN 1 TYP IN 1 END IN 1	OTAL QTYTOTAL QTYMASSMASS4 PRECASTIN FIELDIN 14 PRECASTIN FIELD	MARK SIZE TYPE V W X Y Z LENGTH IN 1 EXTERIOR IN 1 INTERIOR IN	TOTAL QTY TOTAL No. OF MASS MASS 8 PRECAST BARS FOR FIELD IN PRECAST ALL IN FIELD
C1001 10 V	380 585 172	DECK PANEL DECK PANEL 1 600 60 40	UNITSPLACEMENTUNITS (kg)PLACEMENT (kg)8001 005	P1501 15 STR Image: STR <th< td=""><td>ELEMENTSPLACEMENTELEMENTS (kg)PLACEMENT (kg)860</td></th<>	ELEMENTSPLACEMENTELEMENTS (kg)PLACEMENT (kg)860
C1501 15 STR C1502 15 U 44		3 080 12 1 652 30	144 696 424 1 100	P1502 15 U 50 250 362* 400 250 1 662 24 91 P1503 15 STR 4 775 1	96 250 4 30
C1503 15 STR		3 080 100	1 200 5 803	P1504 15 STR 5 900 9 8	68 6 630 56
C1504 15 X C1505 15 STR		11 380 22 24 3 100 40	312 13 5 574 232 80 389 (111)	P1505 15 E 300 300 600 6 P1506 15 STR 5 625 3 3	24 23 24 212
C1506 15 STR		2 495 18	36 141	P1507 15 M 2 910 3 080 30	120 580
C1507 15 STR C1508 15 Z		3 100 12 1 066	24 117 26 44	P1508 15 G 225 300 825 18 16 P1509 15 C 4 465 308 101 4 789 1 16	136 176 4 30
C2001 20 W C2501 25 STR		11 280 22 24 3 100 20	312 13 8 288 345 40 487	P1510 15 STR 1 1 P1511 15 M 2 675 2 845 3 5	4 9 12 54
			PLAIN TOTAL IN 14 PRECAST ELEMENTS (kg): 23 600	P1512 15 STR 2 750 18	72 311
			PLAIN TOTAL IN FIELD PLACEMENT (kg): 621	P1513 15 G 415 250 915 5 P2001 20 M 2 910 3 110 2 410 2 410 2 410 3 110 2 410 410 410 410 410 5 5 410 <t< td=""><td>20 29 8 59</td></t<>	20 29 8 59
				P2002 20 STR 2 910 32 910 </td <td>128 877 28 176</td>	128 877 28 176
				P2004 20 STR 2 750 41	164 1 062
				P3001 30 STR 4 775 2 10 P3002 30 STR 5 900 12 10	8 210 88 6 2.853 195
				P3003 30 STR 5 625 6 6	48 1 484
				NOTE: * ADJUST LAST 2 END BARS TO FIT CURB END CHAMFER	PLAIN TOTAL IN 8 PRECAST ELEMENTS (kg): 9 115 PLAIN TOTAL IN FIELD PLACEMENT (kg): 250
			X	4	
	<u>₹</u>				
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		Lw L		7	
Х 180° 135° НООК НООК		JD_SHAPE_VIEW		Z	HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2 JEAN MARIE RIVER BRIDGE
Х 180° 135° Ноок Ноок				Ζ	HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2 JEAN MARIE RIVER BRIDGE BAR LIST
		3D SHAPE VIEW		Ζ	HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2 JEAN MARIE RIVER BRIDGE BAR LIST SHEET 2
					HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2 JEAN MARIE RIVER BRIDGE BAR LIST SHEET 2 DATE
	Image: Non-State Image: Non-State Image: Non-State Image: Non-State Image: Non-State Image: Non-State				HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2 JEAN MARIE RIVER BRIDGE BAR LIST SHEET 2 DESIGNED YL DATE 2024-03-25 CHECKED JZ DATE 2024-03-25 DRAWN KK DATE 2024-03-25
	R	S			HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2 JEAN MARIE RIVER BRIDGE BAR LIST SHEET 2 DESIGNED YL DATE 2024-03-25 CHECKED JZ DATE 2024-03-25
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P X Z	×	S			HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2 JEAN MARIE RIVER BRIDGE BAR LIST SHEET 2 DESIGNED YL DATE 2024-03-25 CHECKED JZ DATE 2024-03-25 DRAWN KK DATE 2024-03-25 SCALE AS SHOWN PREPARED UNDER THE DIRECTION OF
	X W W W W W W W W	S	$\begin{array}{c} \hline \\ \hline \\ \hline \\ \hline \\ \hline \\ \hline \\ 180^{\circ} \\ HOOK \end{array} \begin{array}{c} \\ 180^{\circ} \\ HOOK \end{array} \end{array}$		HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2 JEAN MARIE RIVER BRIDGE BAR LIST SHEET 2 DESIGNED YL DATE 2024-03-25 CHECKED JZ DATE 2024-03-25 DRAWN KK DATE 2024-03-25 SCALE AS SHOWN PREPARED UNDER THE DIRECTION OF <u>YING YI LI, P.ENG.</u> ENGINEER OF RECORD
P Y 7	X W Y W W W W W W W W	S			HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2 JEAN MARIE RIVER BRIDGE BAR LIST SHEET 2 DESIGNED YL DATE 2024-03-25 CHECKED JZ DATE 2024-03-25 DRAWN KK DATE 2024-03-25 SCALE AS SHOWN PREPARED UNDER THE DIRECTION OF

