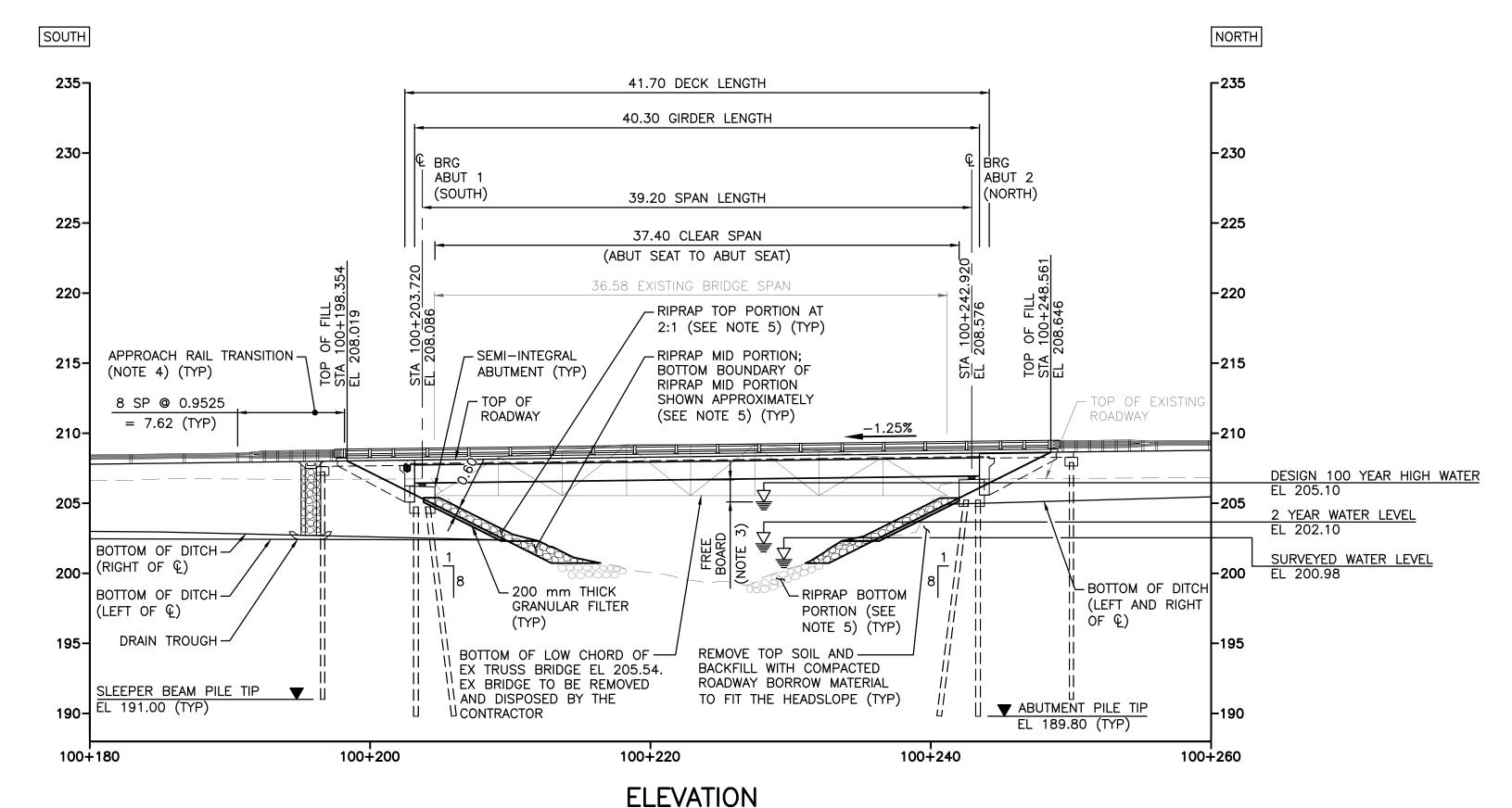


	1 01111	SCHEDU	LL	
NORTHING	EASTING	STATION	OFFSET	ELEVATION
6813599.524	593974.476	100+203.720	0.00	208.086
6813638.714	593973.606	100+242.920	0.00	208.576
	6813599.524	6813599.524 593974.476	NORTHING EASTING STATION 6813599.524 593974.476 100+203.720 6813638.714 593973.606 100+242.920	6813599.524 593974.476 100+203.720 0.00

BENCHMARK POINT TABLE						
DESCRIPTION	NORTHING	EASTING	ELEVATION			
BM1	6813590.94	593981.66	206.55			
WSC 209	6813636.61	593979.31	205.21			
WSC 210	6813600.75	593980.07	205.16			

SCALE 1:250



SCALE 1:250

NOTES

- 1. DIMENSIONS ON THIS DRAWING ARE IN METRES UNLESS NOTED OTHERWISE.
- 2. ELEVATIONS ARE GIVEN TO THE TOP OF ROADWAY ALONG THE HIGHWAY 1 CONTROL
- 3. REQUIRED MINIMUM FREE BOARD = 1.20 m FREE BOARD PROVIDED AT ABUTMENT 1 = 1.20 m FREE BOARD PROVIDED AT ABUTMENT 2 = 1.67 m FREE BOARD PROVIDED AT MID-SPAN = 1.44 m
- 4. APPROACH RAIL TRANSITION: FIRST SIX SPACES AT 952.5 mm TO BE TWO LAYERS OF 2.7 mm THICK THRIE BEAM GUARDRAIL WITH TERMINAL CONNECTOR. LAST TWO SPACES AT 952.5 mm TO BE 2.7 mm THICK W—THRIE BEAM TRANSITION SECTION. REFER TO ALBERTA TRANSPORTATION STANDARD DRAWINGS S—1642—20, S—1643—20 AND SHEET 53 FOR DETAILS.
- 5. RIPRAP SHALL BE PLACED AS BELOW:

TOP PORTION:

(ABOVE THE 1:2-YEAR WATER LEVEL OF 202.10 m TO THE TOP OF RIPRAP ELEVATION OF 205.4 m FOR BOTH ABUTMENTS)

USE A COMBINATION OF SALVAGED EXISTING RIPRAP AND IMPORTED CLASS 1 HEAVY ROCK RIPRAP UNDERLAIN BY 200 mm THICK GRANULAR FILTER BASE. IMPORTED CLASS 1 HEAVY ROCK RIPRAP AND SORTED SALVAGED RIPRAP MEETING THE CLASS 1 GRADATION SHALL BE PLACED ON THE BANK TO A THICKNESS OF 600 mm. THE TRANSITIONS BETWEEN THE SLOPE ARMOURING AND THE ADJACENT NATURAL/EXISTING BANK SHALL BE SMOOTH AND TO THE SATISFACTION OF THE HYDROTECHNICAL ENGINEER. GRANULAR FILTER BASE MATERIAL SHALL BE APPROVED EITHER AS WELL GRADED CRUSHED GRANULAR MATERIAL WITH D100 OF 100 mm OR EQUIVALENT WELL GRADED UNCRUSHED BANK PITRUN GRAVEL. THE CONTRACTOR SHALL SUBMIT THE PROPOSED MATERIAL FOR APPROVAL BY THE HYDROTECHNICAL ENGINEER AT LEAST 2 WEEKS PRIOR TO THE WORKS.

MID PORTION:

(BELOW THE 1:2-YEAR WATER LEVEL OF 202.10 m AND ABOVE THE WATER LEVEL AT THE TIME OF THE RIPRAP PLACEMENT FOR THIS PORTION. RIPRAP FOR THIS PORTION SHALL BE PLACED DURING LOWER WATER LEVEL SEASON(S)) ADD CLASS 1 HEAVY ROCK RIPRAP TO THE VOIDS OF THE EXISTING RIPRAP SURFACE TO AREAS WITH NOMINAL 1.0 m THICKNESS TO THE SATISFACTION OF THE HYDROTECHNICAL ENGINEER.

BOTTOM PORTION:

(EXISTING RIPRAP BELOW THE WATER LEVEL AT THE TIME OF PLACING MID PORTION RIPRAP DURING LOWER WATER LEVEL SEASON)

NO NEW RIPRAP PLACEMENT BELOW THE WATER LEVEL. EXISTING RIPRAP BELOW THE

WATER LEVEL SHALL REMAIN AND UNDISTURBED.

(THIS PORTION OF RIPRAP SHOWN ON THE DRAWING IS SCHEMATIC)



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lev.	Date	Description	Init
1	2024-12-06	ISSUED FOR CONSTRUCTION	YL
0	2024-07-19	ISSUED FOR TENDER	YL
	<u> </u>	REVISIONS	

Government of Northwest Territories HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2

JEAN MARIE RIVER BRIDGE GENERAL LAYOUT

 DESIGNED
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 DATE
 2024–12–06

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 JZ
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 2024–12–06

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 DATE
 2024–12–06

 SCALE
 AS SHOWN

PREPARED UNDER THE DIRECTION OF

G449506 CANADA INC. O/A
JACOBS CONSULTANCY CANADA INC.
Signature Assurded To L4371
Date 6 Dec. 2024

PERMIT NUMBER: P 1453
NT/NU Association of Professional
Engineers and Geoscientists

PROJECT No.

SHEET No.

DRAWING No.

CE857700

23 OF 55

SC-INF01-6081-S001

IILE: C:\pw_workdir\Jacobs—us—va—pw—02\dms92401a\s001.d[.] LOTTED : Friday, December 6, 2024 OBTAIN CLARIFICATION FROM THE ENGINEER IN THE EVENT THAT INFORMATION CONTAINED ELSEWHERE IN THE CONTRACT DOCUMENTS APPEARS TO

DO NOT SCALE DRAWINGS.

CONFLICT WITH THESE GENERAL NOTES.

ALL DIMENSIONS ARE IN MILLIMETERS (mm OR MM) UNO.

ALL STATIONS, ELEVATIONS, EASTINGS, AND NORTHINGS ARE IN METERS (m OR M) UNO.

STATIONS, NORTHINGS, EASTINGS AND DIMENSIONS ARE GIVEN IN GRID COORDINATES AND VALUES UNO.

ANY UTILITY INFORMATION SHOWN ON THESE DRAWINGS CONCERNING TYPE AND LOCATION OF UNDERGROUND UTILITIES IS NOT GUARANTEED TO BE ACCURATE OR ALL-INCLUSIVE. THE CONTRACTOR SHALL DETERMINE THE TYPE AND LOCATION OF UNDERGROUND UTILITIES AND PROTECT AS REQUIRED. FOR RELOCATING EXISTING UNDERGROUND UTILITIES, REFER TO SPECIAL PROVISIONS.

ALL CONSTRUCTION SHALL CONFORM TO SPECIAL PROVISIONS, GOVERNMENT OF NORTHWEST TERRITORIES STANDARD SPECIFICATIONS FOR BRIDGE CONSTRUCTION (SSBC) (EDITION 1 2021), AND THESE DRAWINGS.

ALL CONSTRUCTION MATERIALS NOT SPECIFIED HEREIN AND IN THE DRAWINGS SHALL CONFORM TO THE REQUIREMENTS IN SSBC.

DESIGN CRITERIA & LOADING

DESIGN IS BASED ON:

- 1. CAN/CSA-S6-19 'CANADIAN HIGHWAY BRIDGE DESIGN CODE' (REFERRED TO HEREAFTER AS 'CSA S6').
- 2. DESIGN CRITERIA / STANDARDS BY GOVERNMENT OF NORTHWEST
- TERRITORIES (GNWT) WITH APPROVED DESIGN EXCEPTIONS (DE) 3. ALBERTA TRANSPORTATION BRIDGE STRUCTURES DESIGN CRITERIA, VERSION 9.0, JANUARY 2022

DESIGN LIFE OF THE BRIDGE STRUCTURE: 75 YEARS

DESIGN LIVE LOAD: CL-800

BARRIERS DESIGNED FOR PERFORMANCE LEVEL: TL-4

CONSTRUCTION LIVE LOAD ON DECK = 1.5 kPa (NOT ON THE DECK CANTILEVER UNLESS BALANCED)

50-YEAR RETURN PERIOD MINIMUM AND MAXIMUM DAILY MEAN TEMPERATURES = -48 AND 28 °C MINIMUM AND MAXIMUM EFFECTIVE TEMPERATURES = -46 AND 43 °C

LONGITUDINAL BRAKING FORCE = 228 kN

MEAN WIND PRESSURE FOR 50 YEAR RETURN PERIOD = 0.41 kPa MEAN WIND PRESSURE FOR 10 YEAR RETURN PERIOD (DURING CONSTRUCTION) = 0.30 kPa

DESIGN HORIZONTAL WIND PRESSURE = 1.64 kPa

DESIGN VERTICAL WIND PRESSURE = 0.82 kPa

SAFETY RAIL DESIGN LOAD: A CONCENTRATED LOAD OF 1.0 KN APPLIED AT ANY POINT SO AS TO PRODUCE THE MOST CRITICAL EFFECT, AS PER NATIONAL BUILDING CODE, CL 7.4.5.14 (B), APPLICABLE TO WHERE THE GATHERING OF MANY PEOPLE IS IMPROBABLE.

SEISMIC:

LIFELINE BRIDGE

SITE CLASS: C

SEISMIC PERFORMANCE CATEGORY: 1

S(0.2) = 0.169

SEISMIC CONNECTION HORIZONTAL LOAD AT ABUTMENTS: 455 kN. EACH

(TRANSVERSE DIRECTION RESTRAINED BY SHEAR BLOCKS: LONGITUDINAL DIRECTION RESTRAINED BY BACKFILL PASSIVE EARTH PRESSURE BEHIND ABUTMENT DIAPHRAGMS)

CAST-IN-PLACE CONCRETE

APPROVED 120 MPa UHPC WITH 2% FIBER

DRAIN TROUGHS AND SHEAR BLOCKS ON ABUTMENT SEATS: CLASS C. 28-DAY STRENGTH 35 MPa

PRECAST CONCRETE ELEMENT CONNECTIONS, POCKETS AND GIRDER HAUNCHES:

ALL UHPC CASTING FORMWORK SHALL BE WATER-TIGHT TESTED PRIOR TO THE CONCRETE PLACEMENT.

PRECAST CONCRETE

DECK PANELS, DIAPHRAGMS AND APPROACH SLABS: CLASS HPC, 28-DAY STRENGTH 45 MPa

ALL OTHER PRECAST CONCRETE ELEMENTS: CLASS C, 28-DAY STRENGTH 35 MPa

REINFORCING STEEL SHALL HAVE 50 mm CLEAR CONCRETE COVER, EXCEPT THAT POCKET AND UHPC CONNECTION JOINT SURFACES TO HAVE MINIMUM 25 mm CLEAR CONCRETE COVER, UNLESS NOTED OTHERWISE.

ALL CORNERS (NOT INCLUDING CORNERS AT POCKET AND UHPC CONNECTION SURFACES) SHALL BE CONSTRUCTED WITH A MINIMUM 20 mm CHAMFER OR FILLET UNO.

THE CONTRACTOR IS RESPONSIBLE FOR ALL ASPECTS OF LIFTING (INCLUDING DESIGN OF LIFTING AND LEVELING DEVICES), HANDLING, ERECTION AND INSTALLATION OF PRECAST ELEMENTS AND TEMPORARY WORKS. THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER THE PLANS AND DRAWINGS, SIGNED AND SEALED BY A PROFESSIONAL ENGINEER IN THE NORTHWEST TERRITORIES INDICATING ALL LOADS, AND ANY TEMPORARY SUPPORT SYSTEM REQUIRED AT LEAST 2 WEEKS PRIOR TO THE FABRICATION. THE SUBMISSION SHALL VERIFY THAT THE PRECAST ELEMENTS ARE CAPABLE OF RESISTING ALL CONSTRUCTION LOADS SAFELY AND WITHOUT DAMAGE.

ALL JOINT AND POCKET SURFACES OF PRECAST ELEMENTS SHALL BE PRE-ROUGHENED (AT PLANT) BY ABRASIVE BLASTING TO ICRI CSP NO. 6, AS PER SSBC SECTION 7.2.5.15.

THE ESTIMATED MASS OF PRECAST CONCRETE ELEMENTS SHOWN IN THE DRAWINGS ARE FOR THE GOVERNMENT OF NORTHWEST TERRITORIES USE ONLY AND THE DEPARTMENT ASSUMES NO RESPONSIBILITY FOR ITS ACCURACY OR USE BY OTHERS.

REINFORCING STEEL

PLAIN REINFORCING STEEL SHALL CONFORM TO CAN/CSA-G30.18-M. GRADE

STAINLESS STEEL SHALL CONFORM TO THE REQUIREMENTS OF ASTM A276 AND A955/A955M (INCLUDING ANNEXES) WITH MINIMUM YIELD STRENGTH OF 420 MPa.

BAR MARK SUFFIX SS DENOTES SOLID STAINLESS STEEL BARS.

WELDING OF REINFORCEMENT IS NOT PERMITTED WITHOUT THE WRITTEN APPROVAL FROM THE ENGINEER.

STRUCTURAL STEEL & MISC METAL

PILES: STEEL SHALL CONFORM TO CAN/CSA-G40.21 GRADE 350W.

ALL STEEL FOR GIRDERS, STIFFENERS, SPLICE PLATES, AND ALL MATERIALS WELDED TO THE GIRDERS, INCLUDING PLATES EMBEDDED IN PRECAST CONCRETE ABUTMENT DIAPHRAGM FOR GIRDER CONNECTIONS, SHALL CONFORM TO CSA G40.21M-350AT, CATEGORY (4,) WITH CHARPY V-NOTCH STRENGTH OF 27 JOULES AT -45°C.

ALL OTHER STEEL INCLUDING DIAPHRAGM ELEMENTS AND LATERAL BRACINGS SHALL CONFORM TO CSA G40.21M-350A.

SHEAR STUDS (EXCEPT STAINLESS STEEL STUDS IN ABUTMENT SEAT SHEAR BLOCKS):

ASTM STANDARD A108, GRADES 1015, 1018, OR 1020, AND CONFORMING TO AWS D1.5 TABLE 7.1 FOR TYPE B STUDS

STRUCTURAL BOLTS TO WEATHERING STEEL APPLICATIONS SHALL BE ASTM STRUCTURAL BOLTS TO GALVANIZED STEEL APPLICATIONS SHALL BE A325M - TYPE 1 GALVANIZED IN ACCORDANCE WITH ASTM F2329

NUTS AND WASHERS FOR GIRDERS: MARKED WITH A "3" TO DENOTE WEATHERING GRADE

USE TWO WASHERS FOR ALL CONNECTIONS.

BRIDGERAIL AND APPROACH RAIL TRANSITION MATERIALS SHALL BE IN ACCORDANCE WITH ALBERTA TRANSPORTATION STANDARD DRAWING S-1642-20.

ALL MATERIALS AND CONNECTIONS ARE DESIGNED BASED ON METRIC UNITS. CHANGING TO IMPERIAL UNITS SHALL NOT BE DONE WITHOUT APPROVAL FROM THE ENGINEER. IMPERIAL SIZE COMPONENTS LARGER THAN METRIC SPECIFIED SHALL NOT BE AN EXTRA COST TO THE CONTRACT.

GALVANIZED COMPONENTS:

ALL COMPONENTS, EXCEPT WEATHERING STEEL, SHALL BE HOT DIP GALVANIZED, IN ACCORDANCE WITH ASTM A123M WITH A MINIMUM NET RETENTION OF 600 g/m², UNO.

GRIND ALL WELDS SMOOTH BEFORE GALVANIZING.

FIELD WELDS AND DAMAGED AREAS OF GALVANIZING SHALL BE METALLIZED IN ACCORDANCE WITH SSBC SECTION 6.2.7.3.3

EMBEDDED GALVANIZED STEEL ELEMENTS IN CONTACT WITH PLAIN OR STAINLESS REINFORCING STEEL SHALL BE SEPARATED BY PLASTIC ISOLATOR CLIPS OR APPROVED EQUIVALENT. WEATHERING STEEL COMPONENTS IN CONTACT WITH GALVANIZED ELEMENTS SHALL BE ISOLATED USING AN APPROVED COATING.

SEE SPECIFICATIONS AND THE DRAWINGS FOR FABRICATION AND INSTALLATION DETAILS, AND MATERIALS NOT SPECIFIED ABOVE.

STEEL GIRDERS

ALL BOLTED CONNECTIONS SHALL BE MADE WITH 22 mm DIAMETER UNO. BOLTED CONNECTIONS SHALL BE DETAILED WITH THREAD EXCLUDED FROM THE SHEAR PLANE.

THE ESTIMATED MASS OF THE STEEL GIRDER (GIRDERS AND DIAPHRAGMS BUT NOT BEARINGS, STUDS, BOLTS, ETC.) IS 23 TONNES. THIS ESTIMATE IS FOR THE GOVERNMENT OF NORTHWEST TERRITORIES USE ONLY AND THE DEPARTMENT ASSUMES NO RESPONSIBILITY FOR ITS ACCURACY OR USE BY OTHERS.

DESIGN

GIRDER

DISTRIBUTION FACTORS

= 0.647SHEAR (ULS, SLS) = 0.578BENDING (ULS, SLS) = 0.611 SHEAR (FLS) BENDING (FLS) = 0.398DEAD LOAD:

ASSUMED CONSTRUCTION LOADS = 1.5 kPg

= 6.66 kN/m (MID-SPAN)6.26 kN/m (END SPAN) PER GIRDER DECK AND HAUNCH = 18.3 kN/m PER GIRDER

FATIGUE: ACCORDING TO CAN/CSA-S6-19, HIGHWAY CLASS A

THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW CONSTRUCTION PLANS, SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE NORTHWEST TERRITORIES INDICATING ALL LOADS, PROPOSED METHODS AND SEQUENCES OF CONSTRUCTION, AND ANY TEMPORARY SUPPORT SYSTEMS REQUIRED. THE SUBMISSION SHALL VERIFY THAT THE GIRDERS ARE CAPABLE OF RESISTING THE ACTUAL LOADS SAFELY AND WITHOUT DAMAGE.

FABRICATION

FABRICATION AND ERECTION SHALL BE IN ACCORDANCE WITH SSBC.

ALL GIRDER DIMENSIONS ARE CORRECT AT 20°C AND GIRDER LENGTHS ARE MEASURED ALONG THE BOTTOM FLANGE.

ALL WELDING, CUTTING AND PREPARATION SHALL BE IN ACCORDANCE WITH THE AWS BRIDGE WELDING CODE, D1.5.

BEARING STIFFENERS UNDER FULL BRIDGE DEAD LOAD SHALL BE VERTICAL. INTERMEDIATE WEB STIFFENERS AND DIAPHRAGM CONNECTION STIFFENERS SHALL BE INSTALLED PERPENDICULAR TO THE GIRDER FLANGES.

THE CONTRACTOR SHALL SUBMIT WELD SIZES AND WELDING PROCEDURE TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION.

ALL WELD METAL DEPOSITS SHALL HAVE CHARPY V NOTCH IMPACT STRENGTH OF AT LEAST 27 JOULES AT -45°C. ALL WELD METAL DEPOSITS SHALL BE SUBJECT TO THE APPROVAL OF THE ENGINEER AND SHALL PRODUCE BOTH THE CORROSION RESISTANCE AND THE COLOUR PROPERTIES OF THE BASE

ALL BOLT HOLES SHALL BE DRILLED 2 mm LARGER THAN THE SPECIFIED BOLT DIAMETER UNLESS NOTED OTHERWISE.

TIGHTENING OF ALL HIGH STRENGTH BOLTS SHALL BE DONE BY THE TURN OF NUT METHOD ONLY IN ACCORDANCE WITH SSBC. BEFORE FINAL TIGHTENING THERE SHALL BE A SUFFICIENT NUMBER OF BOLTS BROUGHT TO A SNUG TIGHT CONDITION TO ENSURE THAT PARTS OF THE JOINT ARE BROUGHT INTO FULL CONTACT WITH EACH OTHER.

GIRDERS SHALL MEET THE CAMBER REQUIREMENTS AS SHOWN ON GIRDER

ALL STEEL SHALL BE BLAST CLEANED AFTER FABRICATION IN ACCORDANCE WITH SSBC SECTION 6.2.7.1

TEMPORARY SUPPORT

THE CONTRACTOR SHALL INSTALL TEMPORARY SUPPORTING STRUCTURES TO MAINTAIN THE GIRDER'S STABILITY SOON AFTER THE GIRDERS ARE ERECTED.

GIRDERS SHALL BE TEMPORARILY RESTRAINED SOON AFTER BEING PLACED ON THE BEARINGS AND PRIOR TO RELEASING THE LIFTING DEVICE TO AVOID SLIDING. THE RESTRAINING SYSTEM SHALL REMAIN IN PLACE UNTIL BACKFILL WITH COMPACTION BEHIND ABUTMENT DIAPHRAGMS IS MINIMUM 1.2 m ABOVE THE BOTTOM OF THE DIAPHRAGMS TO RETAIN THE SLIDING FORCE.

INSPECTION & NON-DESTRUCTIVE TESTING

WELD INSPECTION AND TESTING SHALL BE CARRIED OUT IN ACCORDANCE WITH SSBC.

BRIDGE ELEMENTS LIFTING & ERECTION

THE CONTRACTOR SHALL SUBMIT GIRDER AND PRECAST CONCRETE ELEMENT LIFTING AND ERECTION PROCEDURES TO THE ENGINEER FOR REVIEW AT LEAST 2 WEEKS PRIOR TO THE WORKS AS PER THE CONTRACT SPECIAL PROVISION AND SSBC.

THE LIFTING AND ERECTION PROCEDURES FOR ALL BRIDGE ELEMENTS SHALL BE DESIGNED, STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER

HYDROTECHNICAL DATA

HYDROTECHNICAL DESIGN REPORT PREPARED BY MATRIX SOLUTIONS INC. DATED JANUARY 16, 2024.

DESIGN FLOOD RETURN PERIOD: 1:100 YEAR DESIGN FLOW: 210 m³/s DESIGN WATER LEVEL: 205.10 m DESIGN VELOCITY: 2.1 m/s

GEOTECHNICAL

GEOTECHNICAL REPORT WAS COMPLETED BY MASKWA ENGINEERING LTD., DATED OCTOBER 30, 2023.

SURVEY

TOPOGRAPHICAL SURVEY WAS COMPLETED BY STANTEC CONSULTING LTD. IN 2019. SUPPLEMENTAL TOPOGRAPHICAL SURVEY WAS COMPLETED BY MASKWA ENGINEERING LTD. IN JUNE 2023.

BRIDGE CONSTRUCTION SEQUENCE

BELOW IS A GENERAL DESCRIPTION OF THE CONSTRUCTION SEQUENCE ASSUMED FOR THE PURPOSE OF THE DESIGN OF THE WORKS. IT IS NOT A DETAILED WORK PLAN AND HIGHLIGHTS ONLY CERTAIN ACTIVITIES REQUIRED TO COMPLETE THE WORK. SPECIFIC WORK PLANNING AND DETAILS OF THE CONSTRUCTION EXECUTION AND SCHEDULING ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND THE CONTRACTOR'S PLAN IS SUBJECT TO THE REVIEW AND ACCEPTANCE OF THE ENGINEER AS WELL AS OTHER AUTHORITIES HAVING JURISDICTION.

AN ALTERNATE SEQUENCE PLAN WILL BE GIVEN CONSIDERATION BY THE ENGINEER SO LONG AS THE CONTRACTOR SUBMITS A COMPLETE PROPOSAL FOR REVIEW. SUCH AN ALTERNATIVE SEQUENCE PLAN MAY OR MAY NOT BE GIVEN ACCEPTANCE BY THE ENGINEER AND THE ENGINEER'S RULING SHALL GOVERN.

- SURVEY EXISTING BRIDGE PILE ROW LOCATIONS AS PER DRAWING S005 AND S006 (SHEET 27 AND 28);
- BUILD DETOUR ROAD AND TEMPORARY BRIDGE;

REMOVE THE EXISTING BRIDGE;

- INSTALL PILES;
- INSTALL ABUTMENT SEATS AND UHPC CONNECTION JOINTS;
- INSTALL SLEEPER BEAM (FLEXIBLE STAGE AS DETERMINED BY THE CONTRACTOR):
- INSTALL HEAD SLOPE AND RIPRAP (FLEXIBLE STAGE AS DETERMINED BY THE CONTRACTOR):
- INSTALL BEARINGS, BUT DO NOT GROUT THE BEARING POCKETS;

 INSTALL GIRDERS AND PROVIDE GIRDER TEMPORARY STABILITY SUPPORTS. ALSO INCLUDING THE SUPPORT SYSTEM TO PREVENT GIRDER LONGITUDINAL SLIDING. THE LONGITUDINAL SUPPORT SYSTEM TO STAY IN PLACE UNTIL BACKFILL BEHIND ABUTMENTS IS COMPLETED;

- INSTALL GIRDER INTERMEDIATE DIAPHRAGMS;
- GROUT BEARING POCKETS:
- INSTALL PRECAST CONCRETE ABUTMENT DIAPHRAGMS;
- INSTALL PRECAST CONCRETE WINGWALLS WITH TEMPORARY SUPPORTS (LATERALLY AND VERTICALLY) AND UHPC CONNECTION JOINTS;
- BACKFILL BEHIND ABUTMENTS UP TO 1.2 m ABOVE BOTTOM OF ABUTMENT DIAPHRAGMS WITH BALANCED METHOD SPECIFIED IN THE
- INSTALL PRECAST CONCRETE DECK PANELS AND UHPC CONNECTION
- COMPLETE BACKFILL AND INSTALL APPROACH SLAB ELEMENTS AND UHPC CONNECTION JOINTS;
- INSTALL BRIDGERAILS AND APPROACH RAIL POSTS;
- INSTALL CIP CONCRETE DRAIN TROUGHS

IN ADDITION TO SUBMITTING EXISTING BRIDGE PILE ROW SURVEY DATA, THE CONTRACTOR SHALL HAVE THE BEARINGS DESIGNED AND SUBMIT THE SHOP DRAWINGS AT AN EARLIEST POSSIBLE TIME FOR THE ENGINEER'S REVIEW PRIOR TO THE FABRICATION OF BRIDGE ELEMENTS THAT ARE RELATED TO ACTUAL BEARING ASSEMBLY SIZES.

LEGEND & ABBREVIATIONS

BH	BOREHOLE	GALV	GALVANIZED
Ę	CENTRELINE	HPC	HIGH PERFORMANCE CONCRETE
\otimes	STUD WELDING	HORIZ	HORIZONTAL
A	SURVEY CONTROL POINT	HWY	HIGHWAY
<u></u>	WATER LEVEL	INT	INTERIOR
→WP	WORK POINT	LG	LONG
ABUT	ABUTMENT	MIN	MINIMUM
AIFB	ASPHALT IMPREGNATED FIBREBOARD	MISC	MISCELLANEOUS
APP	APPROACH	NOM	NOMINAL
B/W	BETWEEN	NTS	NOT TO SCALE
BRG	BEARING	OH	OVERHEAD FIBRE OPTIC LINE
BOTT	ВОТТОМ	PL	PLATE
C/W	COMPLETE WITH	PROJ	PROJECTION
CIP	CAST-IN-PLACE	QTY	QUANTITY
CJ	CONSTRUCTION JOINT	SIM	SIMILAR
CLR	CLEAR	SS	STAINLESS STEEL BAR
CONT	CONTINUOUS	STA	STATION
CSP	CORRUGATED STEEL PIPE	SYM	SYMMETRICAL
DIA	DIAMETER	TYP	TYPICAL
DIAPH	DIAPHRAGM	UHPC	ULTRA HIGH PERFORMANCE CONCRETE
EL	ELEVATION	UNO	UNLESS NOTED OTHERWISE
EX	EXISTING	VERT	VERTICAL
EXT	EXTERIOR	WSC	WATER SURVEY OF CANADA
FO	FIBRE OPTIC CABLE (BURIED)	WW	WINGWALL

Northwest Territories

Consultant Logo

Rev	Date	Description	Init			
1	2024-12-06	ISSUED FOR CONSTRUCTION	YL			
0	2024-07-19	ISSUED FOR TENDER	YL			
	REVISIONS					

Government of Northwest Territories

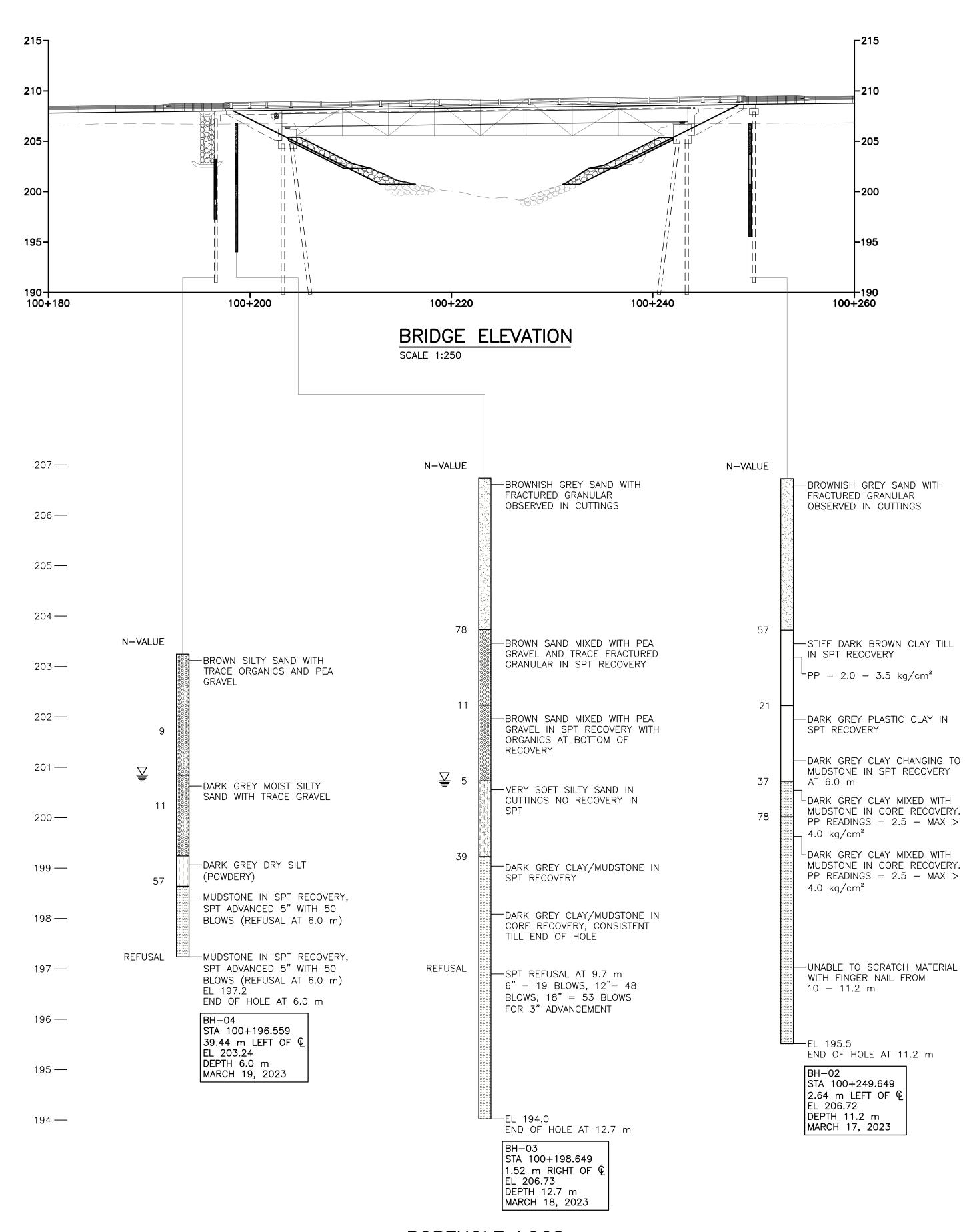
JEAN MARIE RIVER BRIDGE GENERAL NOTES

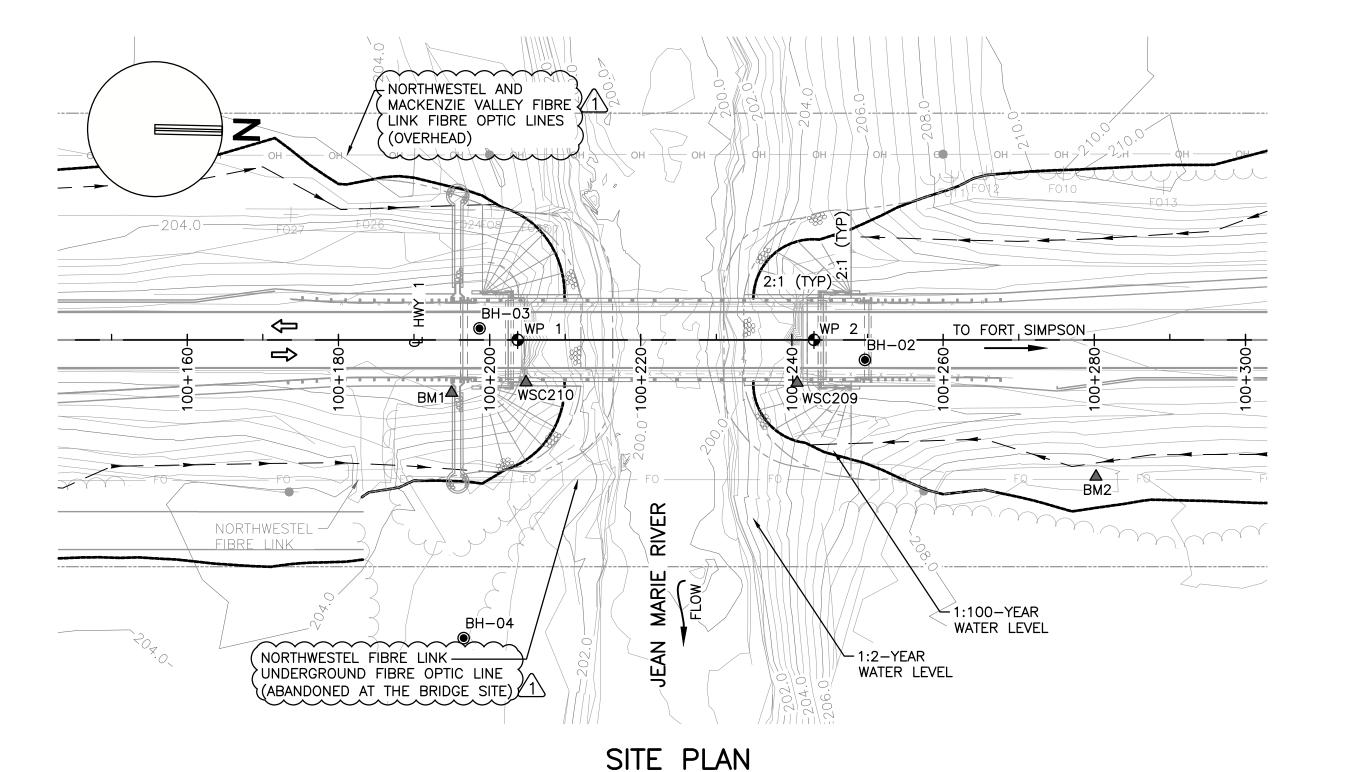
HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2

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PERMIT TO PRACTICE ENGINEER OF RECORD PERMIT NUMBER: P 1453 NT/NU Association of Profession Engineers and Geoscientists DATE 2024-12-06 PROJECT No. SHEET No. DRAWING No. CE857700 24 OF 55 SC-INF01-6081-S002

REGISTERED IN THE NORTHWEST TERRITORIES.





SCALE 1:500

NOTES

- 1. ALL GEOTECHNICAL INFORMATION PROVIDED FOR THIS PROJECT HAS BEEN COMPILED FOR THE GOVERNMENT OF NORTHWEST TERRITORIES FOR DESIGN PURPOSES ONLY. WHILE IT IS BELIEVED TO CORRECTLY REPRODUCE OR SUMMARIZE OBSERVATIONS MADE DURING TESTING, IT IS ONLY VALID FOR THE PRECISE LOCATION(S) SHOWN, AND IS NOT TO BE CONSTRUED AS GUARANTEEING THE ACTUAL MATERIALS AND CONDITIONS EXISTING THROUGHOUT THE SITE. THE TESTING METHODS USED MAY NOT HAVE DETERMINED THE PRESENCE, ABSENCE OR EXTENT OF BOULDERS, HARD OR SOFT FORMATIONS, WATER TABLES, ARTESIAN CONDITIONS AND OTHER VARIABLES. IT IS THE RESPONSIBILITY OF OTHERS USING THIS INFORMATION TO ENSURE THAT IT IS ADEQUATE FOR THEIR PURPOSES, OR TO SUPPLEMENT IT WITH ADDITIONAL INFORMATION.
- 2. THE INFORMATION IS COMPILED FOR CONVENIENCE FROM HIGHWAY 1 JEAN MARIE RIVER BRIDGE REPLACEMENT GEOTECHNICAL INVESTIGATION REPORT BY MASKWA ENGINEERING LTD. ALL DISCLAIMERS IN THIS REPORT ARE APPLICABLE. REFER TO REPORT FOR FULL GEOTECHNICAL INFORMATION AND IN CASE OF DISCREPANCY THE GEOTECHNICAL REPORT GOVERNS.
- 3. FOR PILE DRIVING REFUSAL CRITERIA REFER TO THE GEOTECHNICAL REPORT BY MASKWA ENGINEERING LTD.



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Jacobs

ev	Date	Description	Init
1	2024-12-06	ISSUED FOR CONSTRUCTION	YL
0	2024-07-19	ISSUED FOR TENDER	YL
		REVISIONS	

Government of Northwest Territories
HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2

JEAN MARIE RIVER BRIDGE
GEOTECHNICAL INFORMATION SHEET

DESIGNED	YL	DATE	2024-12-06				
CHECKED	JZ	DATE	2024-12-06				
DRAWN	KK	DATE	2024-12-06				
SCALE	AS	AS SHOWN					

PERMIT TO PRACTICE
6449506 CANADA INC. O/A
JACOBS CONSULTANCY CANADA INC.
Signature Assertian To L 4371
Date 600c 2024
PERMIT NUMBER: P 1453
NT/NU Association of Professional
Engineers and Geoscientists

PROJECT No.

PREPARED

PREPARED

PREPARED

ENGINEER

DATE

PROJECT No.

DRAWING

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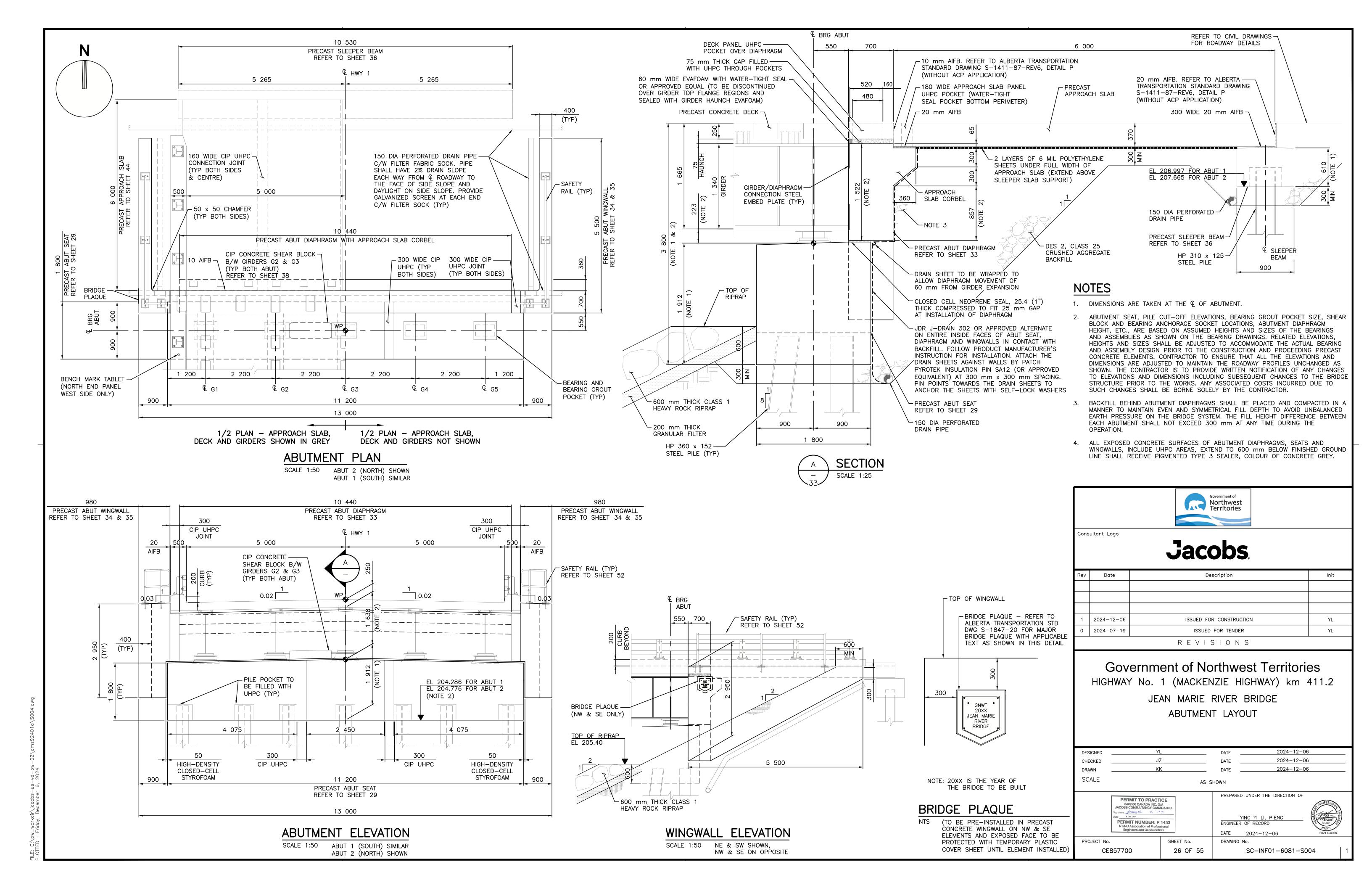
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ENGINEER OF RECORD
DATE 2024–12–06
DRAWING No.

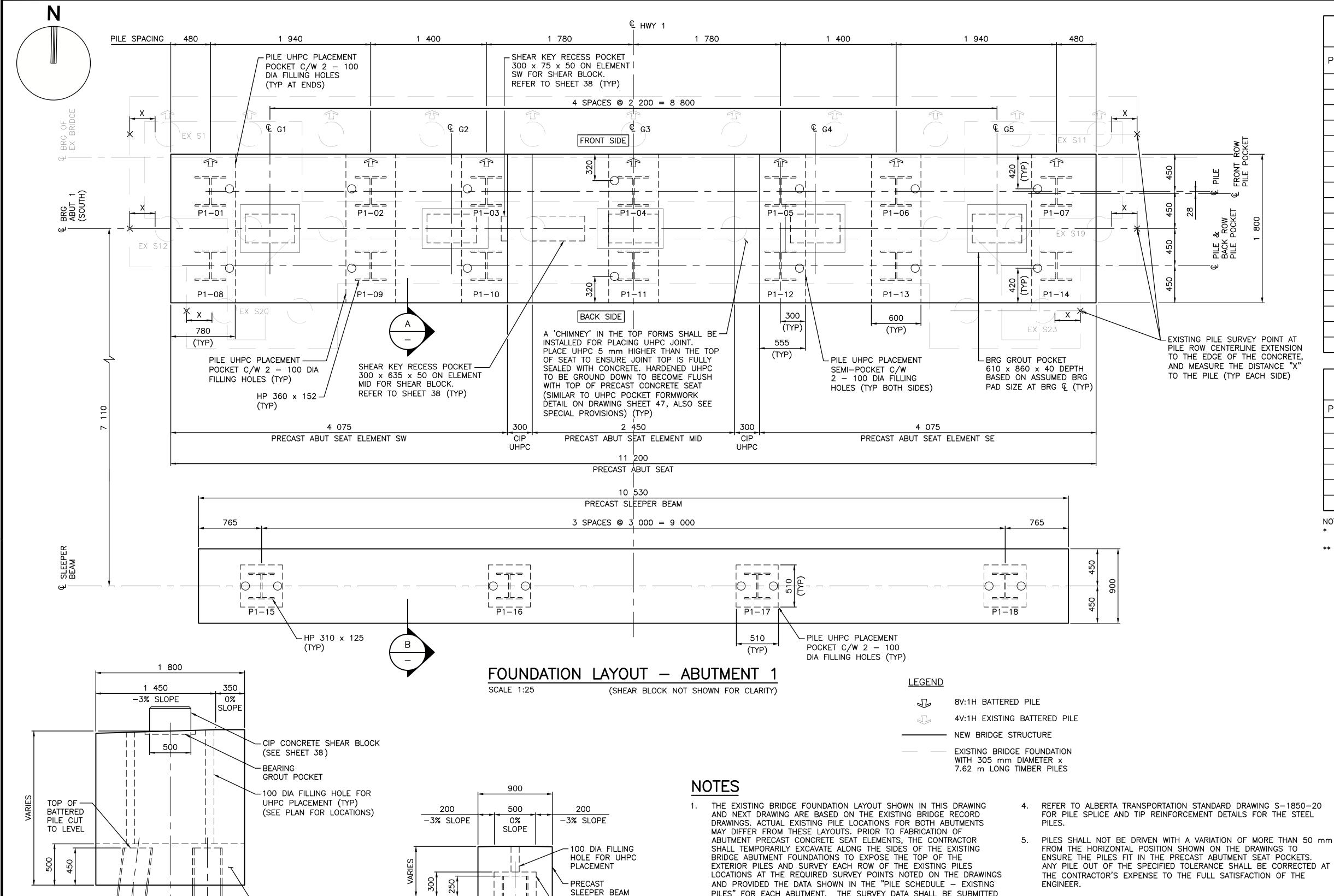
25 OF 55

SC-INF01–6081–S003

BOREHOLE LOGS

SCALE 1:50





- ANY PILE OUT OF THE SPECIFIED TOLERANCE SHALL BE CORRECTED AT
- ALL EXISTING BRIDGE TIMBER PILES SHALL BE CUT TO MINIMUM 0.3 m BELOW BOTTOM OF NEW ABUTMENT SEAT, EXCEPT FRONT ROWS OF EXISTING PILES, WHICH SHALL BE CUT TO MINIMUM 1.0 m BELOW THE NEW FINISHED SOIL GROUND (BOTTOM OF RIPRAP).
- PILE DESIGN LOADS AS BELOW: ABUTMENT SEAT PILES SLS PERMANENT LOADS ONLY: 500 kN SLS EXTREME LOADS (COMBINATION # 1): 700 kN ULS PERMANENT LOADS ONLY: ULS EXTREME LOADS (COMBINATION # 1): 905 kN ABUTMENT SLEEPER BEAM PILES SLS PERMANENT LOADS ONLY: 150 kN SLS EXTREME LOADS (COMBINATION # 1): 450 kN ULS PERMANENT LOADS ONLY: 165 kN ULS EXTREME LOADS (COMBINATION # 1): 510 kN
- 8. PDA TESTING SHALL BE COMPLETED ON PILES P-01, P1-04, P1-07, P2-01, P2-04 AND P2-07

	PILE SO	CHEDUL	_E — A	BUTMENT	1
PILE MARK	NORTHING	EASTING	CUT OFF ELEVATION	APPROXIMATE TIP ELEVATION	PILE LENGTH
P1-01	6813599.86	593969.35	204.736	189.800	14.936
P1-02	6813599.90	593971.29	204.736	189.800	14.936
P1-03	6813599.93	593972.69	204.736	189.800	14.936
P1-04	6813599.97	593974.47	204.736	189.800	14.936
P1-05	6813600.01	593976.25	204.736	189.800	14.936
P1-06	6813600.04	593977.65	204.736	189.800	14.936
P1-07	6813600.09	593979.58	204.736	189.800	14.936
P1-08	6813598.96	593969.37	204.736	189.800	15.052
P1-09	6813599.00	593971.31	204.736	189.800	15.052
P1-10	6813599.03	593972.71	204.736	189.800	15.052
P1-11	6813599.07	593974.49	204.736	189.800	15.052
P1-12	6813599.11	593976.27	204.736	189.800	15.052
P1-13	6813599.14	593977.67	204.736	189.800	15.052
P1-14	6813599.19	593979.60	204.736	189.800	15.052
P1-15	6813592.32	593970.13	207.247	191.000	16.247
P1-16	6813592.38	593973.13	207.247	191.000	16.247
P1-17	6813592.45	593976.13	207.247	191.000	16.247
P1-18	6813592.52	593979.13	207.247	191.000	16.247

PIL	E SCHE	DULE	- EXIST	ING PI	LES
PILE MARK	NORTHING*	EASTING*	NORTHING**	EASTING**	OFFSET "X"
EX S1	6813600.54	593968.81			
EX S11	6813600.79	593980.09			
EX S12	6813599.39	593968.84			
EX S19	6813599.64	593980.11			
EX S20	6813598.42	593969.55			
EX S23	6813598.64	593979.45			

- * THEORETICAL NORTHING/EASTING AT CENTRE OF PILES BASED ON GEOMETRIC INFORMATION FROM RECORD DRAWINGS
- ** SURVEY POINT NORTHING/EASTING AND OFFSET DISTANCE "X" TO BE PROVIDED BY THE CONTRACTOR

Northwest Territories

Consultant Logo

Jacobs

Rev	Date	Description	Init			
1	2024-12-06	ISSUED FOR CONSTRUCTION	YL			
0	2024-07-19	ISSUED FOR TENDER	YL			
	REVISIONS					

Government of Northwest Territories

HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2

JEAN MARIE RIVER BRIDGE FOUNDATION LAYOUT

SHEET 1

· · · ·	AS	SHOWN) UNDER THE DIRECTION OF
SCALE	Δς.		
DRAWN	KK	DATE	2024-12-06
CHECKED	JZ	DATE _	2024-12-06
DESIGNED	YL	DATE _	2024-12-06

ature Azamyad 10 L4371 PERMIT NUMBER: P 1453 NT/NU Association of Professional Engineers and Geoscientists PROJECT No. SHEET No.

27 OF 55

CE857700

YING YI LI, P.ENG ENGINEER OF RECORD DRAWING No.

SC-INF01-6081-S005

- PILE UHPC

(TYP)

450

€ PILE

450

SECTION

SCALE 1:25

€ BRG

© PILE

←HP 360 x 152

PLACEMENT POCKET

510

450

& PILE

SECTION

SCALE 1:25

♠ SLEEPER BEAM

450

►PILE UHPC

PLACEMENT POCKET

∽HP 310 x 125

SURVEY DATA. 2. PILES SHALL BE DRIVEN TO, BUT NOT PAST PRACTICAL REFUSAL TO AVOID DAMAGE. THE PRACTICAL REFUSAL IS DETERMINED AS 10 BLOWS PER 25 mm FOR THE LAST 250 mm, OR AS SPECIFIED BY THE ENGINEER'S REPRESENTATIVE — A QUALIFIED GEOTECHNICAL ENGINEER.

HOLD POINT FOR THE SURVEY AND THE ENGINEER'S REVIEW OF THE

PILES" FOR EACH ABUTMENT. THE SURVEY DATA SHALL BE SUBMITTED TO THE ENGINEER IMMEDIATELY AFTER THE SURVEY. THE TEMPORARY

EXCAVATION FOR THE EXISTING PILE SURVEY SHALL BE BACKFILLED TO

ENGINEER WILL COMPARE THE ROWS OF EXISTING PILES AGAINST THE

EXPECTED TO CONFLICT WITH NEW PILE INSTALLATION, PILE LOCATION

ADJUSTMENT SHALL BE MADE AS NECESSARY BY THE ENGINEER. THERE

ADJUSTMENT IS MADE. FABRICATION OF PRECAST ABUTMENT SEATS IS A

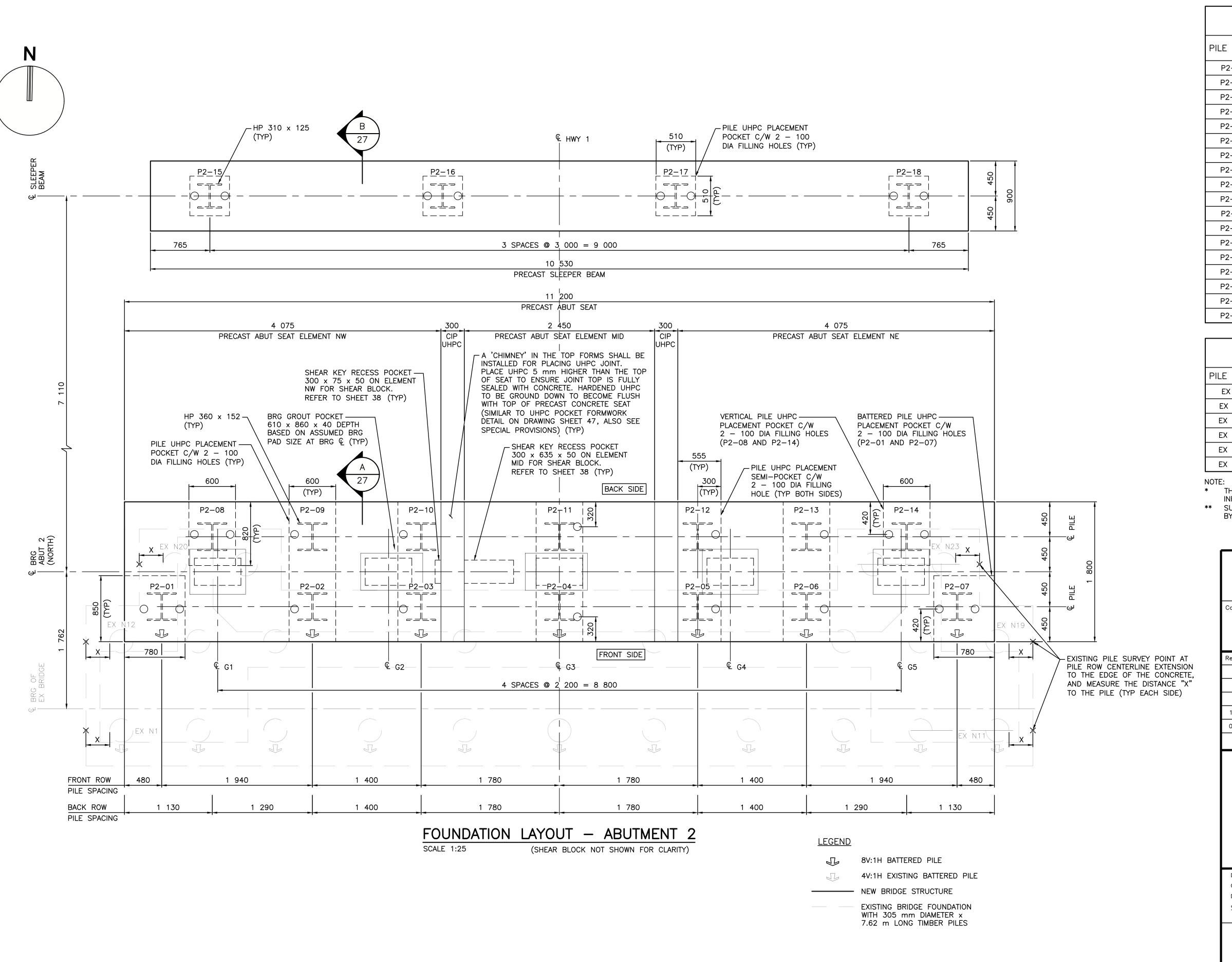
ITS ORIGINAL CONDITION IMMEDIATELY AFTER THE SURVEY. THE

SHALL BE NO ADDITIONAL COST TO THE CONTRACT IF SUCH

ROWS OF THE NEW PILES ON THE FOUNDATION LAYOUTS. IF THE

EXISTING ROWS OF PILES DO NOT MATCH THE DRAWINGS AND ARE

3. PILE TIP REINFORCEMENT SHALL BE PROVIDED FOR ALL PILES.



PILE SCHEDULE - ABUTMENT 2 CUT OFF | APPROXIMATE PILE LENGTH PILE MARK | NORTHING | EASTING ELEVATION | TIP ELEVATION 15.426 6813638.15 | 593968.50 | 205.226 189.800 6813638.19 | 593970.44 | 205.226 189.800 15.426 6813638.22 | 593971.84 | 189.800 15.426 205.226 6813638.26 | 593973.62 | 205.226 189.800 15.426 P2-04 P2-05 6813638.30 | 593975.40 | 205.226 189.800 15.426 P2-06 6813638.33 | 593976.80 | 205.226 189.800 15.426 6813638.38 | 593978.73 | 205.226 189.800 15.426 P2-07 6813639.06 | 593969.13 | 15.546 P2-08 205.226 189.800 P2-09 6813639.09 | 593970.42 | 205.226 189.800 15.546 6813639.12 | 593971.82 | 205.226 189.800 15.546 P2-10 6813639.16 | 593973.60 | 205.226 189.800 15.546 6813639.20 | 593975.38 | 15.546 P2-12 205.226 189.800 6813639.23 | 593976.78 | 205.226 189.800 15.546 6813639.26 | 593978.06 | 205.226 189.800 15.546 6813645.72 | 593968.95 | 207.915 191.000 16.915 P2-16 6813645.79 | 593971.95 | 207.915 191.000 16.915 6813645.86 | 593974.95 | 207.915 191.000 16.915 P2-17 6813645.92 | 593977.94 | 207.915 191.000 16.915

PILE SCHEDULE - EXISTING PILES						
PILE MARK	NORTHING*	EASTING*	NORTHING**	EASTING**	OFFSET "X"	
EX N1	6813636.55	593968.01				
EX N11	6813636.80	593979.29				
EX N12	6813637.70	593967.99				
EX N19	6813637.95	593979.26				
EX N20	6813638.70	593968.65				
EX N23	6813638.92	593978.55				

- * THEORETICAL NORTHING/EASTING AT CENTRE OF PILES BASED ON GEOMETRIC INFORMATION FROM RECORD DRAWINGS
- ** SURVEY POINT NORTHING/EASTING AND OFFSET DISTANCE "X" TO BE PROVIDED BY THE CONTRACTOR

Government of Northwest Territories

Consultant Logo

Jacobs

Rev	Date	Description	Init
1	2024-12-06	ISSUED FOR CONSTRUCTION	YL
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		REVISIONS	

Government of Northwest Territories

HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2

JEAN MARIE RIVER BRIDGE FOUNDATION LAYOUT

SHEET 2

DESIGNED	YL	DATE	2024-12-06
CHECKED	JZ	DATE	2024-12-06
DRAWN	KK	DATE	2024-12-06
SCALE	۸۵	SHOWN	
	73	SHOWIN	

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28 OF 55

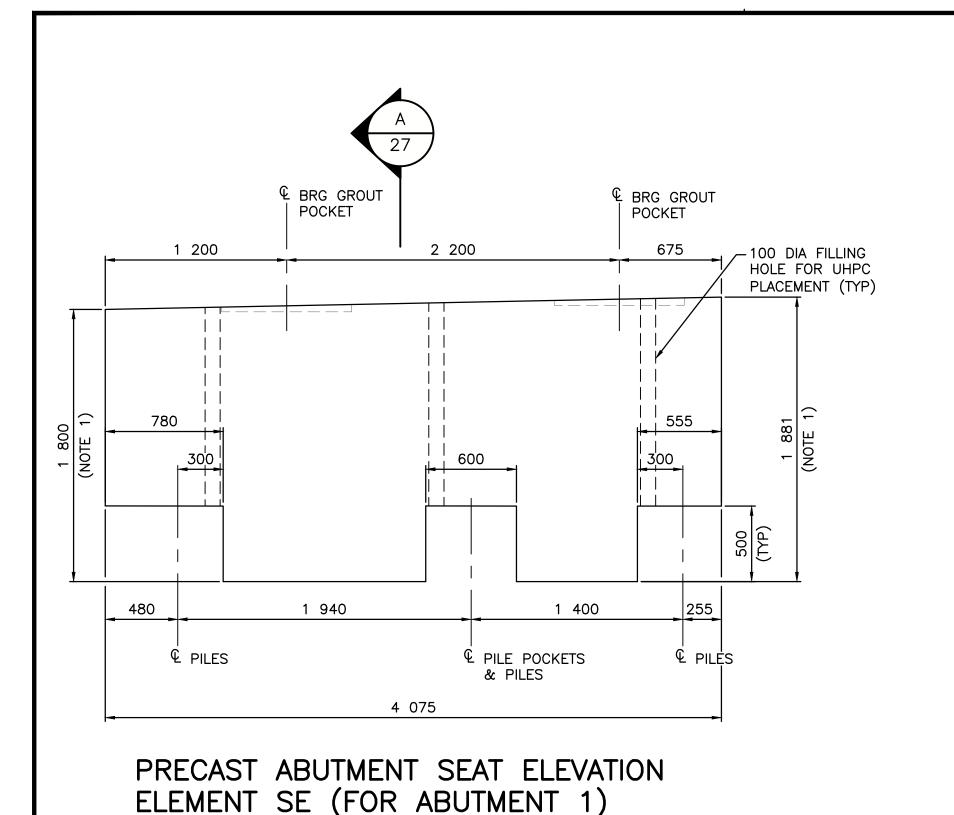
PREPARED UNDER THE DIRECTION OF

YING YI LI, P.ENG.

ENGINEER OF RECORD

DATE 2024–12–06

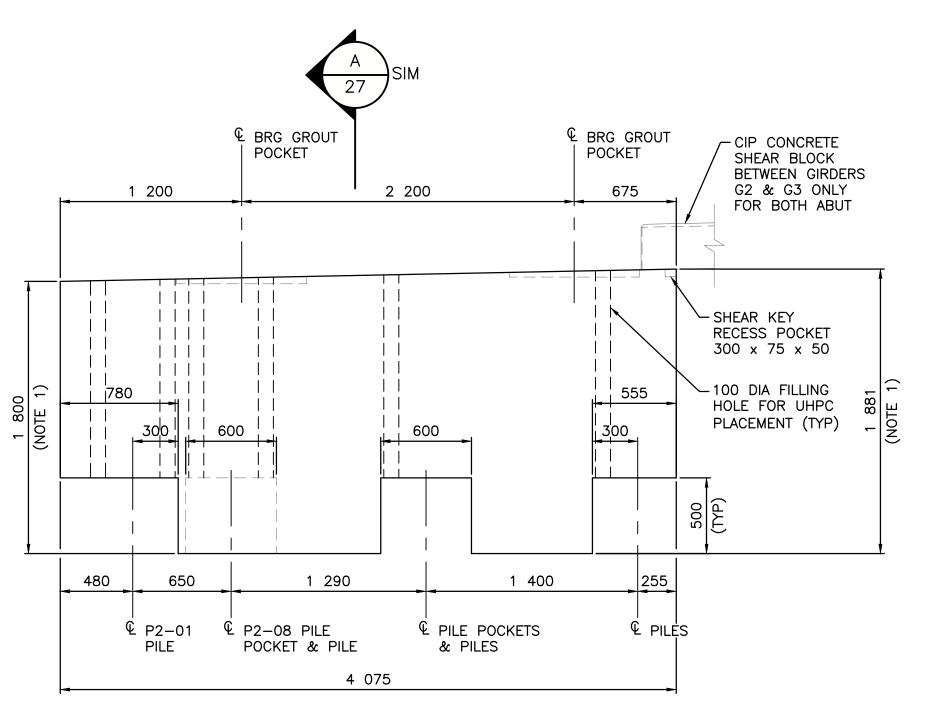
SC-INF01-6081-S006



1. DIMENSIONS ARE TAKEN AT THE € OF BEARING. 2. FOR PRECAST ABUTMENT SEAT ELEMENT PLAN, REFER TO SHEET 27 AND 28. 3. FOR BEARING AND SHEAR BLOCK DETAILS, REFER TO SHEET 38. CIP CONCRETE ---BRG GROUT € BRG GROUT SHEAR BLOCK POCKET POCKET BETWEEN GIRDERS G2 & G3 ONLY 2 200 1 200 FOR BOTH ABUT _⊸675 SHEAR KEY ----RECESS POCKET 300 x 75 x 50 100 DIA FILLING -780 HOLE FOR UHPC PLACEMENT (TYP) 600 300 480 1 400 1 940 € PILES © PILE POCKETS € PILES & PILES 4 075

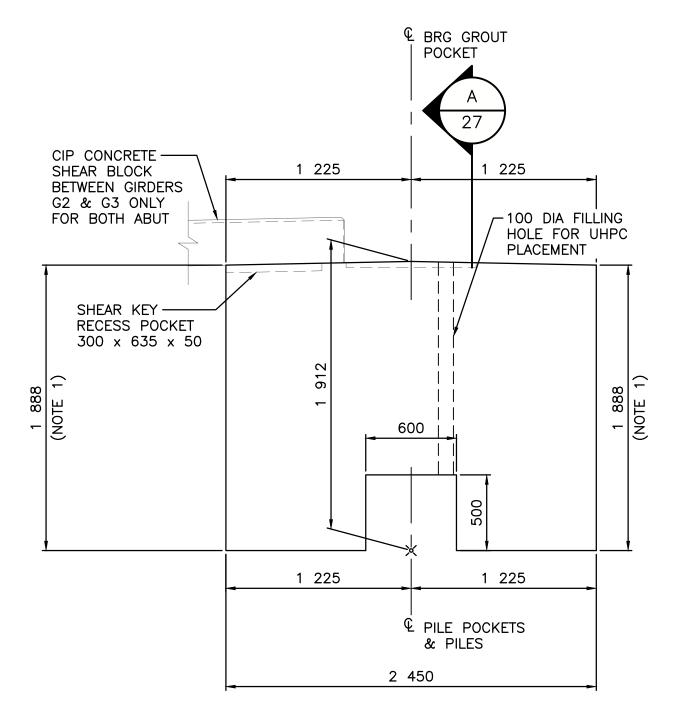
PRECAST ABUTMENT SEAT ELEVATION ELEMENT SW (FOR ABUTMENT 1)

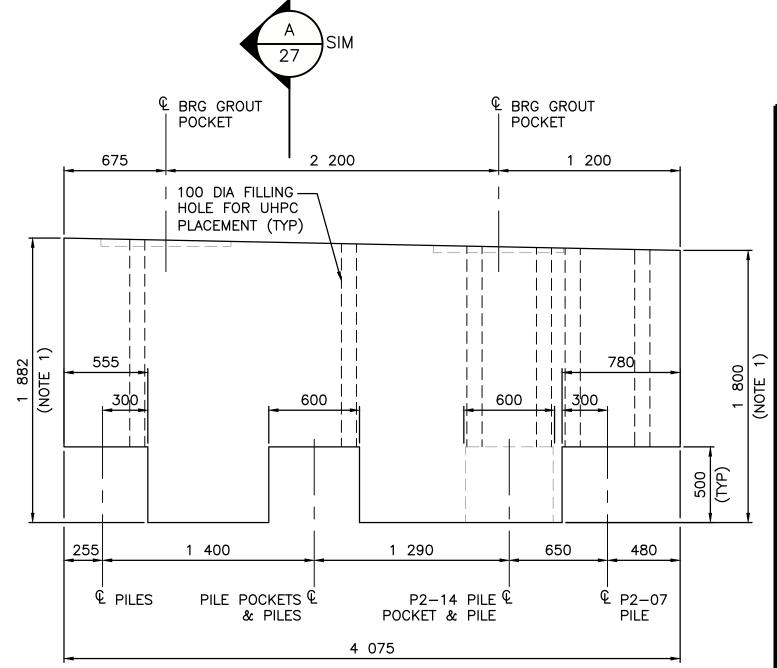
SCALE 1:25 LOOKING SOUTH (1 REQUIRED) (ELEMENT ESTIMATED WEIGHT: 28 800 kg, EACH)



LOOKING SOUTH (1 REQUIRED)

(ELEMENT ESTIMATED WEIGHT: 28 800 kg, EACH)





PRECAST ABUTMENT SEAT ELEVATION ELEMENT NW (FOR ABUTMENT 2)

SCALE 1:25 LOOKING NORTH (1 REQUIRED) (ELEMENT ESTIMATED WEIGHT: 29 100 kg, EACH)

PRECAST ABUTMENT SEAT ELEVATION ELEMENT MID (FOR BOTH ABUTMENTS) SCALE 1:25

(1 REQUIRED FOR EACH ABUTMENT) (ELEMENT ESTIMATED WEIGHT: 19 200 kg, EACH)

PRECAST ABUTMENT SEAT ELEVATION ELEMENT NE (FOR ABUTMENT 2)

SCALE 1:25

LOOKING NORTH (1 REQUIRED) (ELEMENT ESTIMATED WEIGHT: 29 100 kg, EACH)



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NOTES

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1	2024-12-06	ISSUED FOR CONSTRUCTION	YL
0	2024-07-19	ISSUED FOR TENDER	YL

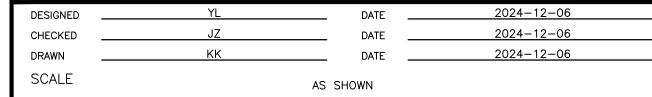
REVISIONS

Government of Northwest Territories

HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2 JEAN MARIE RIVER BRIDGE

PRECAST ABUTMENT SEAT ELEMENT DETAILS

SHEET 1



PERMIT TO PRACTICE
6449506 CANADA INC. O/A
JACOBS CONSULTANCY CANADA INC. PERMIT NUMBER: P 1453 NT/NU Association of Professional Engineers and Geoscientists PROJECT No. SHEET No.

CE857700

29 OF 55

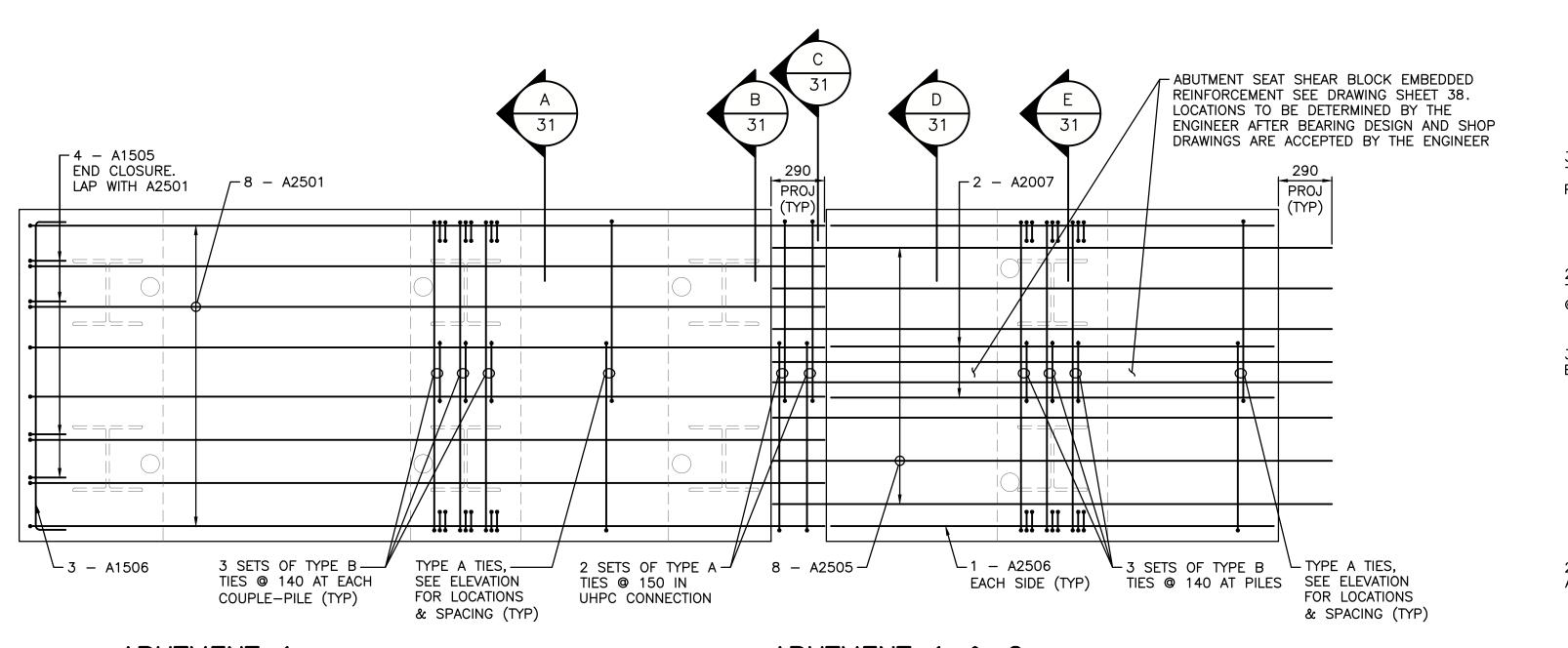
PREPARED UNDER THE DIRECTION OF YING YI LI, P.ENG ENGINEER OF RECORD DATE 2024-12-06 DRAWING No.

SC-INF01-6081-S007

SCALE 1:25

NOTES

- 1. A SET OF TYPE A TIES (BEYOND PILE LOCATIONS) CONSISTS OF A PAIR OF A2001 A PAIR OF A2002 AND 3-A1501 HOOK TIES AS SHOWN ON SECTION A, C AND D
- 2. A SET OF TYPE B TIES (AT A PAIR OF PILES) CONSISTS OF ONE BUNDLE OF TWO PAIRS OF A1503, ONE BUNDLE OF TWO PAIRS OF A1502 ON EACH SIDE AND 4-A2003 HOOKS AS SHOWN ON SECTION B AND E ON SHEET 31.
- 3. A SET OF TYPE C TIES (AT PILE P2-01) CONSISTS OF A PAIR OF A2002 AND ONE BUNDLE OF 2 PAIRS OF A1502 CLOSED TIES, A PAIR OF A2005 AND 4-A2003 AS SHOWN ON SECTION H ON SHEET 32.
- 4. A SET OF TYPE D TIES (AT PILE P2-08) CONSISTS OF A PAIR OF A2001 AND ONE BUNDLE OF 2 PAIRS OF A1502 CLOSED TIES, A PAIR OF A2005 AND 4-A2003 AS SHOWN ON SECTION J ON SHEET 32.
- 5. SHEAR BLOCK EMBEDDED BARS NOT SHOWN FOR CLARITY. REFER TO SHEET 38



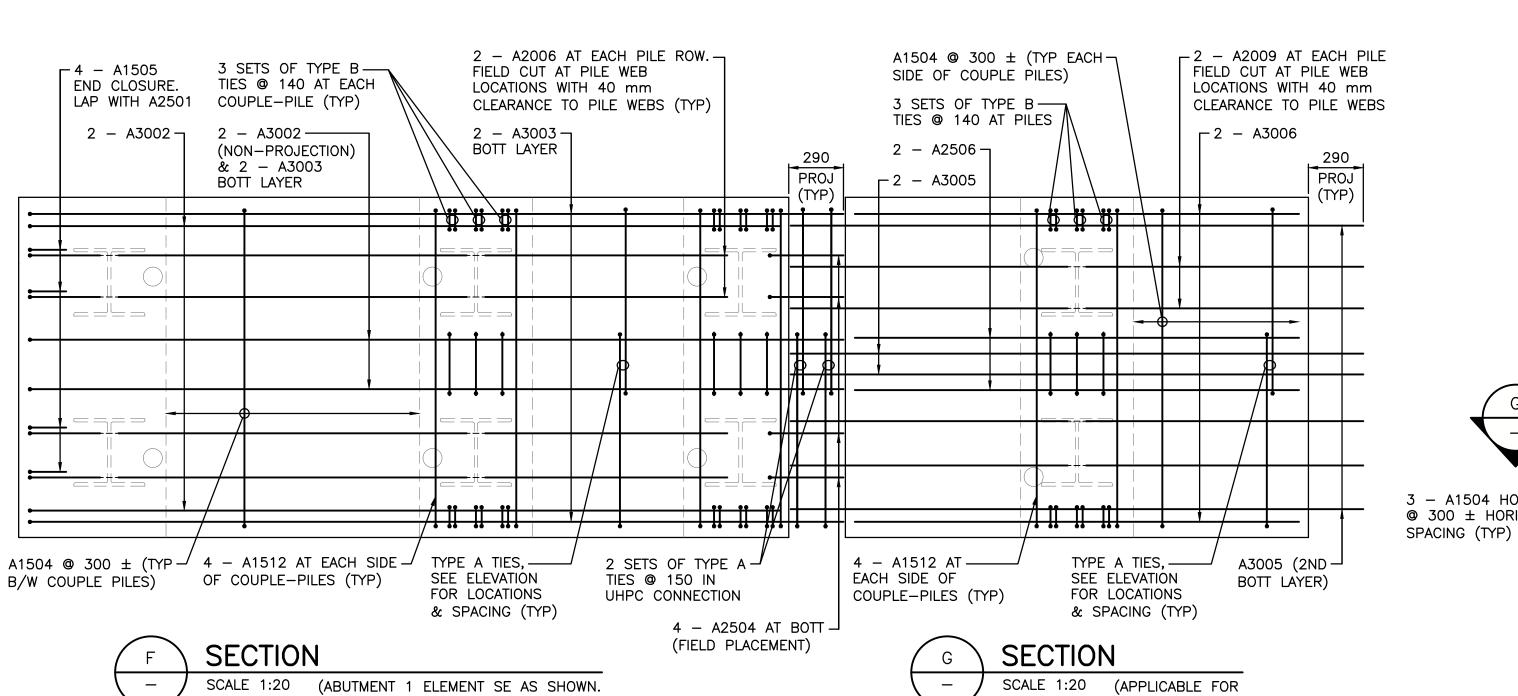
ABUTMENT 1 PRECAST ABUTMENT SEAT ELEMENT SE TOP LAYER OF REINFORCEMENT - PLAN

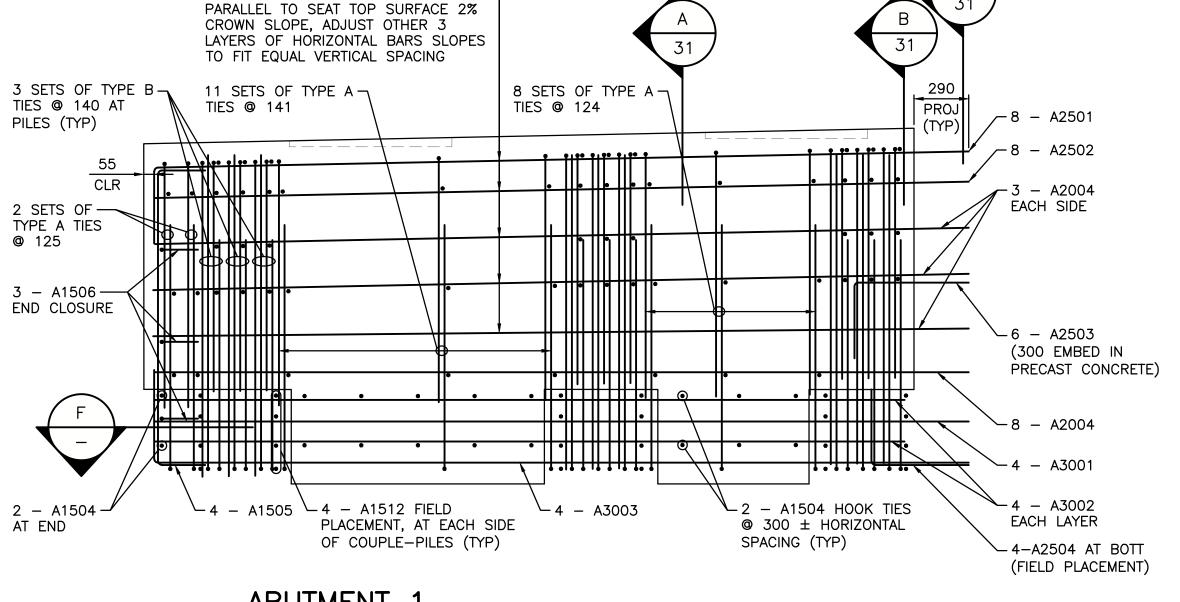
ABUTMENT 1 ELEMENT SW SYMMETRIC)

(ELEMENT SW SYMMETRIC)

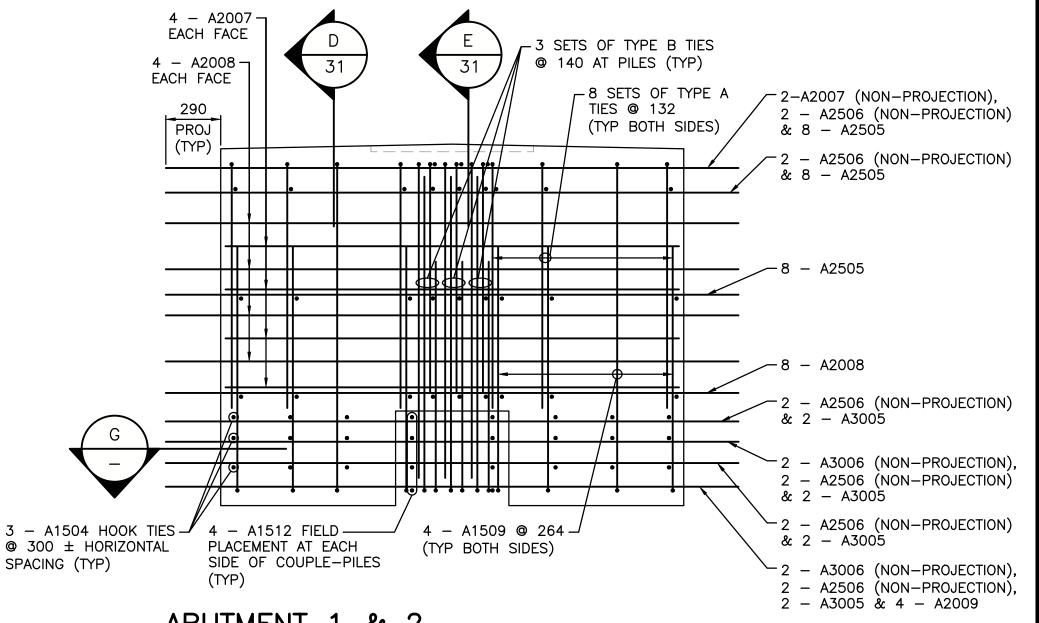
ABUTMENT 1 & 2 PRECAST ABUTMENT SEAT ELEMENT MID TOP LAYER OF REINFORCEMENT - PLAN

BOTH ABUTMENTS)





ABUTMENT 1 PRECAST ABUTMENT SEAT ELEMENT SE REINFORCEMENT DETAIL - ELEVATION (ELEMENT SW SYMMETRIC)



TOP 2 LAYERS OF HORIZONTAL BARS -

ABUTMENT 1 & 2 PRECAST ABUTMENT SEAT ELEMENT MID REINFORCEMENT DETAIL - ELEVATION SCALE 1:20



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Jacobs.

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1	2024-12-06	ISSUED FOR CONSTRUCTION	YL
0	2024-07-19	ISSUED FOR TENDER	YL
		REVISIONS	_

Government of Northwest Territories HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2

JEAN MARIE RIVER BRIDGE PRECAST ABUTMENT SEAT ELEMENT DETAILS

SHEET 2

DESIGNED	YL	DATE	2024-12-06		
CHECKED	JZ	DATE	2024-12-06		
DRAWN	KK	DATE	2024-12-06		
SCALE	AS SHOWN				
	PERMIT TO PRACTICE 6449506 CANADA INC. O/A JACOBS CONSULTANCY CANADA INC. Signature	YING ENGINEER OF	NDER THE DIRECTION OF SYI LI, P.ENG. F RECORD 2024–12–06 NDER THE DIRECTION OF ROFESSION ROF		

DRAWING No.

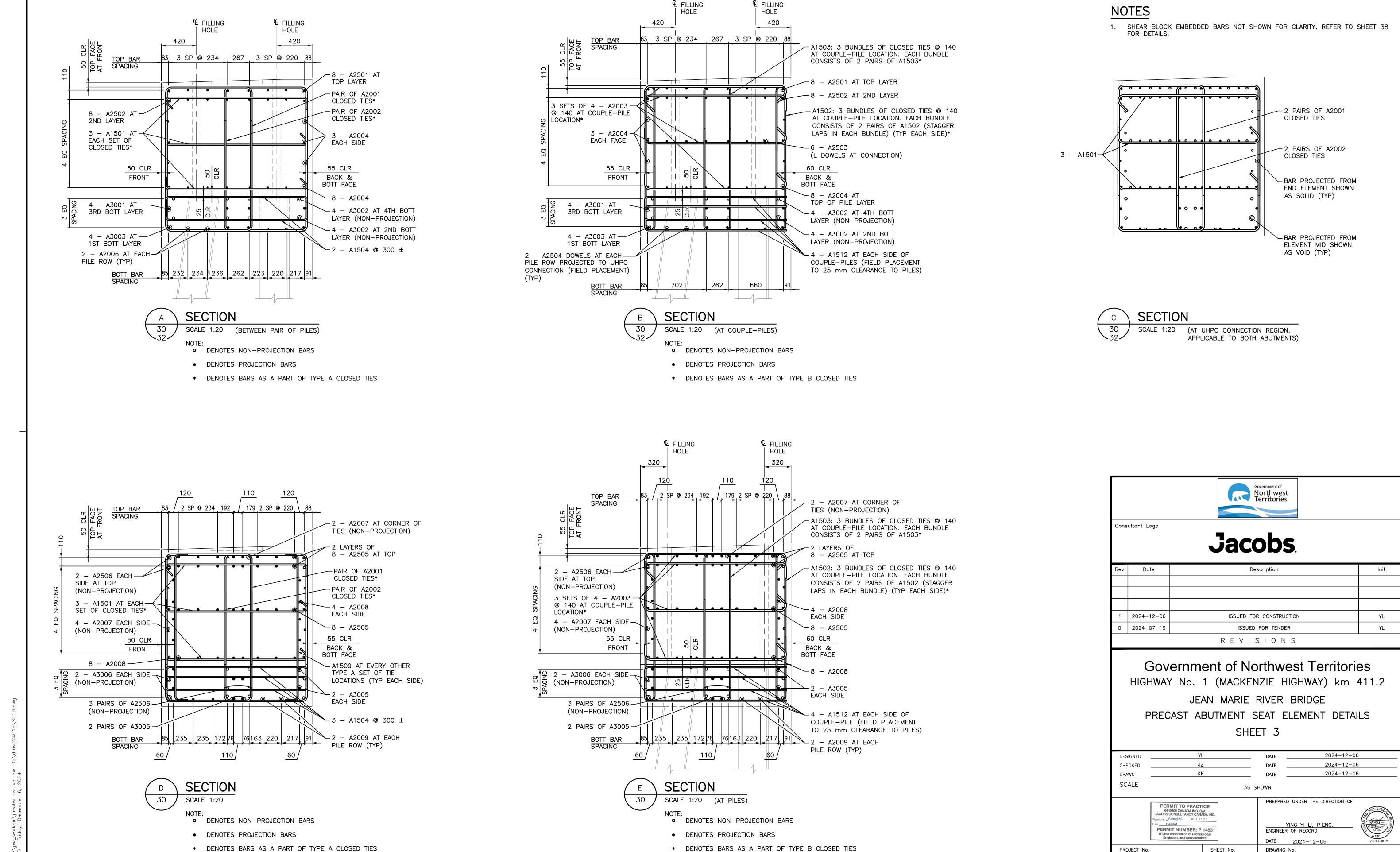
SC-INF01-6081-S008

SHEET No.

30 OF 55

PROJECT No.

CE857700



PROJECT No.

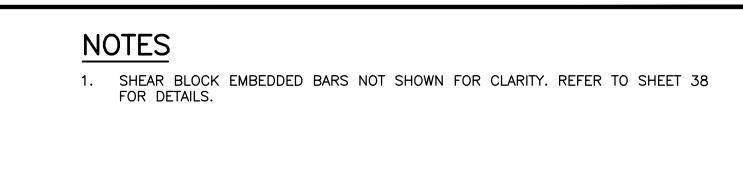
CE857700

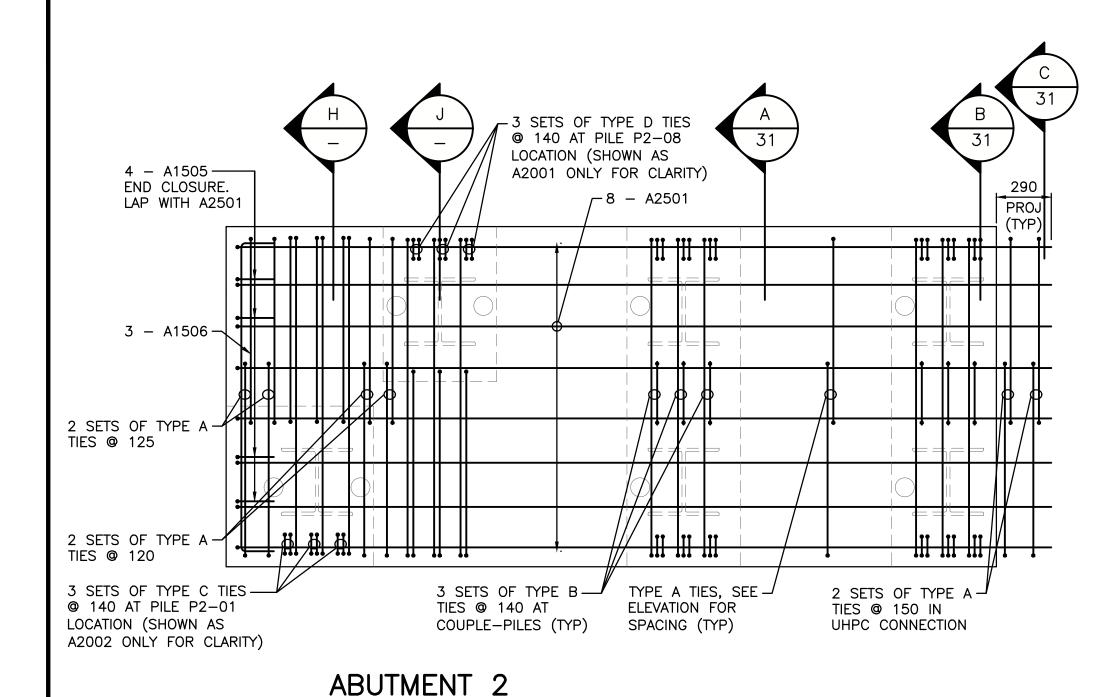
SHEET No.

31 OF 55

DRAWING No.

SC-INF01-6081-S009





€ FILLING € FILLING HOLE HOLE 420 3 SP @ 234 267 3 SP @ 220 88 ~8 - A2501 AT TOP LAYER -3 PAIRS OF A2005 @ 140 AT PILE P2-01* 8 - A2502 AT 2ND LAYER --3 PAIRS OF A2002 CLOSED TIES @ 140 AT PILE P2-01* 4 - A2003 @ 140 AT PILE P2-01* 3 - A2004 EACH SIDE A1502: 3 BUNDLES OF CLOSED -TIES @ 140 AT PILE P2-01. EACH BUNDLE CONSISTS OF 2 PAIRS OF A1502* 55 CLR 55 CLR FRONT BACK & BOTT FACE 4 - A3001 AT 3RD BOTT LAYER --4 - A3002 AT 4TH BOTT LAYER (NON-PROJECTION) 4 - A3003 AT 1ST BOTT LAYER -4 - A3002 AT 2ND BOTT 4 - A1507 AT EACH SIDE OF -LAYER (NON-PROJECTION) PILE P2-01 (FIELD PLACEMENT TO 25 mm CLEARANCE TO PILES) ~2 - A1510 @ 300 -2 - A2006 AT EACH PILE ROW. FIELD 85 232 234 236 262 223 220 217 91 CUT TO ALLOW PILE THROUGH DURING INSTALLATION. CUTTING LOCATION TO BE DETERMINED AFTER INSTALLED PILE COORDINATES SURVEYED. LEAVE BAR PROJECTION AS LONG AS POSSIBLE IN THE PILE POCKETS **SECTION** SCALE 1:20 (AT PILE P2-01)

€ FILLING € FILLING HOLE HOLE 420 3 SP @ 234 267 3 SP @ 220 88 \sim 8 - A2501 AT TOP LAYER -8 - A2502 AT 2ND LAYER 3 PAIRS OF A2005 @ 140 -AT PILE P2-08* -4 — A2003 @ 140 AT PILE P2-08* 3 PAIRS OF A2001 CLOSED -TIES @ 140 AT PILE P2-08* - A1502: 3 BUNDLES OF CLOSED TIES @ 140 AT PILE P2-08. 3 - A2004 EACH SIDE -EACH BUNDLE CONSISTS OF 2 PAIRS OF A1502* 60 CLR **FRONT** 2 - A1511 @ 300 --8 - A2004 AT TOP OF PILE LAYER 4 - A3001 AT 3RD BOTT LAYER -4 - A3002 AT 4TH BOTT LAYER (NON-PROJECTION) 4 - A3003 AT 1ST BOTT LAYER -4 – A3002 AT 2ND BOTT 2 - A2006 AT EACH PILE ROW. FIELD LAYER (NON-PROJECTION) CUT TO ALLOW PILE THROUGH DURING INSTALLATION. CUTTING LOCATION TO -4 - A1508 AT EACH SIDE OF BE DETERMINED AFTER INSTALLED PILE PILE P2-08 (FIELD PLACEMENT COORDINATES SURVEYED. LEAVE BAR TO 25 mm CLEARANCE TO PILES) PROJECTION AS LONG AS POSSIBLE IN | 85 | 232 | 234 | 236 | 262 | 222 | 220 | 217 | 91 | BOTT BAR SPACING THE PILE POCKETS 820 **SECTION** SCALE 1:20 (AT PILE P2-08) DENOTES NON-PROJECTION BARS

DENOTES PROJECTION BARS

Date

* DENOTES BARS AS A PART OF TYPE C CLOSED TIES

Northwest

Territories

Description

PRECAST ABUTMENT SEAT ELEMENT NW TOP LAYER OF REINFORCEMENT - PLAN SCALE 1:20 (ELEMENT NE SYMMETRIC)

(ABUTMENT 2 ELEMENT NW AS SHOWN.

ABUTMENT 2 ELEMENT NE SYMMETRIC)

3 SETS OF TYPE B

COUPLE-PILES (TYP)

(NON-PROJECTION)

TIES @ 140 AT

2 - A3002 -

BOTT LAYER

A1504 @ —

B/W PILES

4 - A1512 AT EACH SIDE -

OF COUPLE-PILES (TYP)

-4 - A1507 AT EACH

SIDE OF PILE P2-01

SECTION

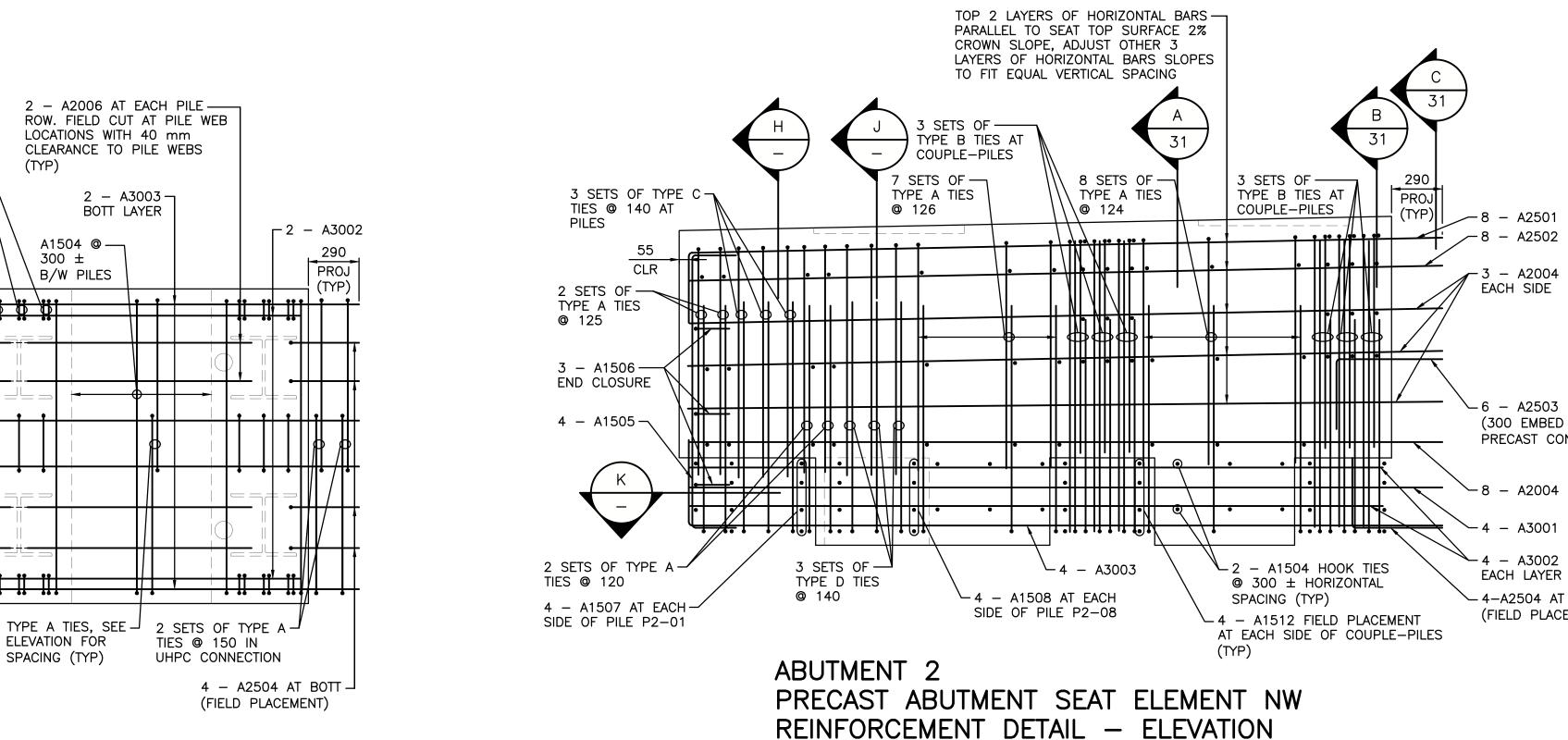
300 ±

& 2 - A3003

DENOTES NON-PROJECTION BARS

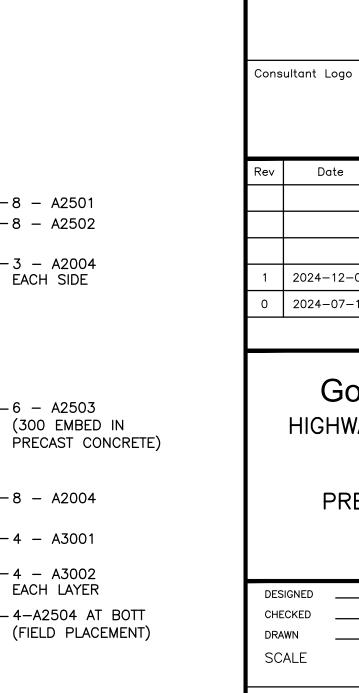
DENOTES PROJECTION BARS

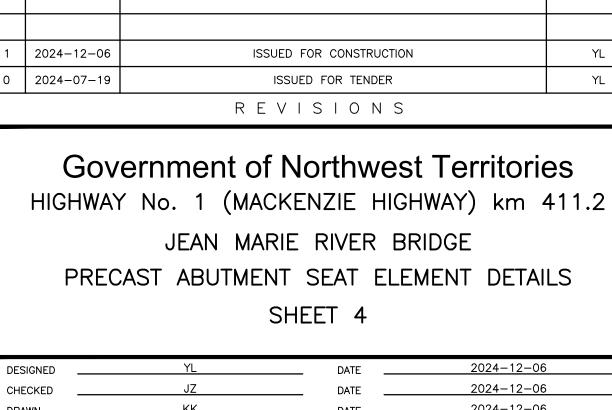
* DENOTES BARS AS A PART OF TYPE C CLOSED TIES



SCALE 1:20

(ELEMENT NE SYMMETRIC)





2024-12-06 SCALE AS SHOWN PREPARED UNDER THE DIRECTION OF PERMIT TO PRACTICE UNIONSEE YING YI LI, P.ENG ENGINEER OF RECORD PERMIT NUMBER: P 1453 NT/NU Association of Professiona Engineers and Geoscientists PROJECT No. DRAWING No. SHEET No. CE857700 32 OF 55 SC-INF01-6081-S010

3 SETS OF TYPE D TIES —

A2001 ONLY FOR CLARITY)

@ 140 AT PILE P2-08

LOCATION (SHOWN AS

4 - A1508 AT EACH-

SIDE OF PILE P2-08

4 - A1505 -

END CLOSURE.

2 SETS OF TYPE TIES @ 125

2 SETS OF TYPE A

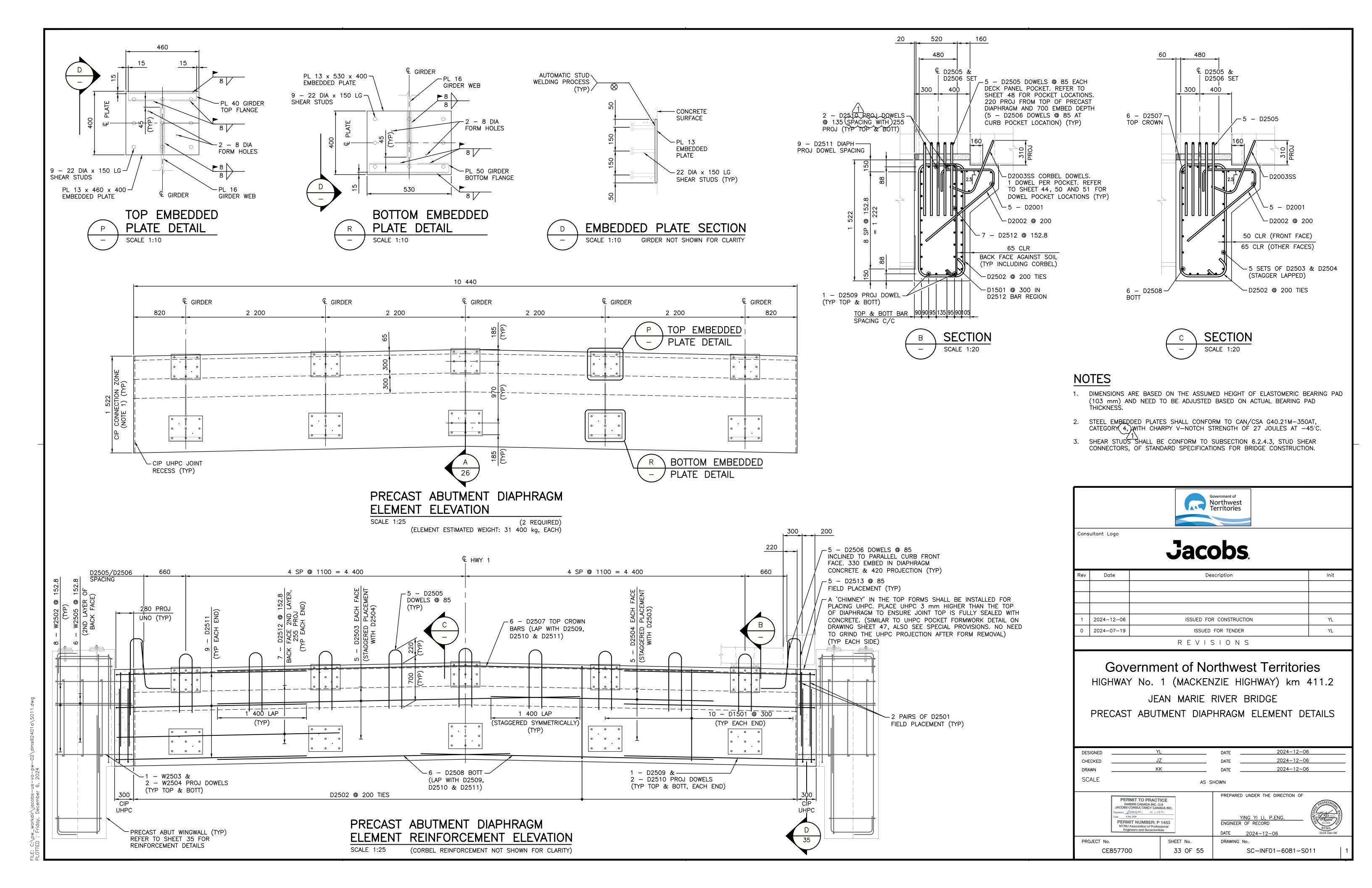
3 SETS OF TYPE C TIES-@ 140 AT PILE P2-01

A2002 ONLY FOR CLARITY)

LOCATION (SHOWN AS

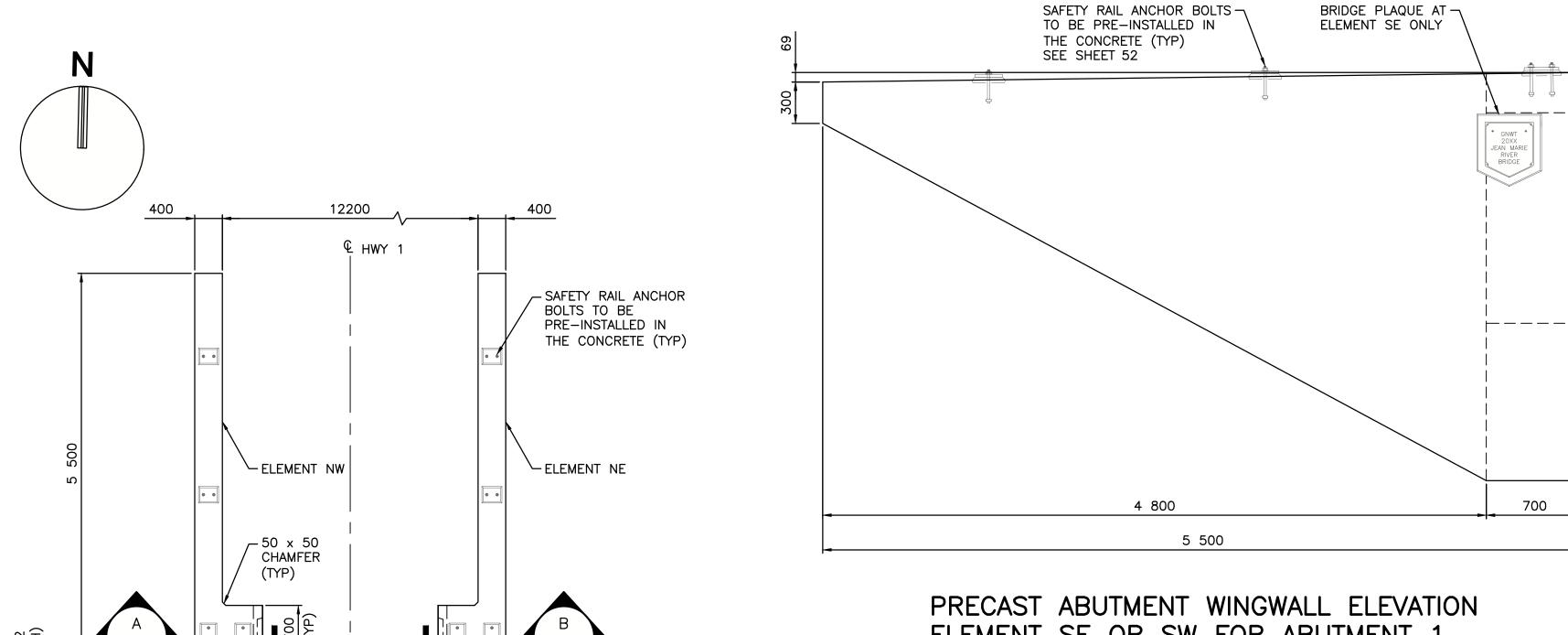
TIES @ 120

LAP WITH A2501



NOTES

1. DIMENSIONS ARE BASED ON THE ASSUMED HEIGHT OF ELASTOMERIC BEARING PAD (103 mm) AND NEED TO BE ADJUSTED BASED ON ACTUAL BEARING PAD

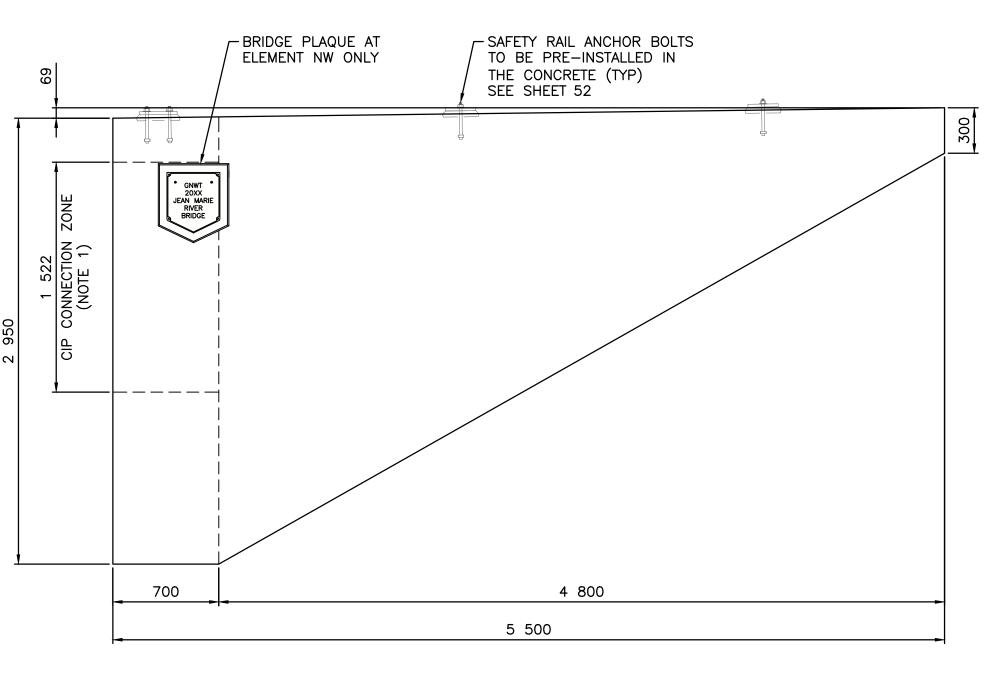


- SAFETY RAIL ANCHOR BOLTS TO BE PRE-INSTALLED IN THE CONCRETE (TYP) 400 850

PRECAST ABUTMENT WINGWALL SECTION ELEMENT NW OR SE SCALE 1:25 (1 REQUIRED EACH CORNER)

ELEMENT SE OR SW FOR ABUTMENT 1 SCALE 1:25

ELEMENT SE SHOWN, SW OPPOSITE ON CORNER (1 REQUIRED EACH CORNER) (ELEMENT ESTIMATED WEIGHT: 12 400 kg, EACH)



SAFETY RAIL ANCHOR BOLTS -TO BE PRE-INSTALLED IN THE CONCRETE (TYP) 400

> PRECAST ABUTMENT WINGWALL SECTION ELEMENT NE OR SW

850

PRECAST ABUTMENT WINGWALL ELEMENT DETAILS

Consultant Logo

Date

2024-12-06

0 2024-07-19

DESIGNED	YL	DATE	2024-12-06
CHECKED	JZ	DATE	2024-12-06
DRAWN	KK	DATE	2024-12-06
COALE			
SCALE	AS	SHOWN	

Northwest Territories

Jacobs.

Description

ISSUED FOR CONSTRUCTION

ISSUED FOR TENDER

REVISIONS

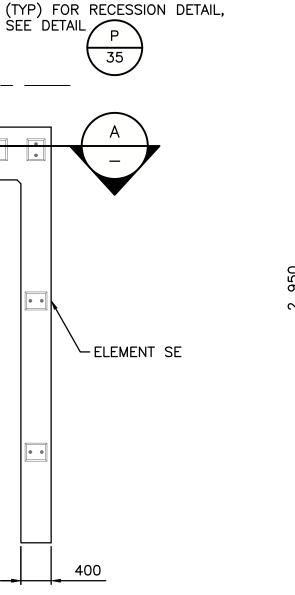
Government of Northwest Territories

HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2

JEAN MARIE RIVER BRIDGE

SHEET 1

PERMIT NUMBER: P 1453 PROJECT No. SHEET No. DRAWING No. CE857700 34 OF 55 SC-INF01-6081-S012



- CONNECTION RECESSION KEY

AT CONNECTION REGION ONLY

PRECAST ABUTMENT WINGWALL

PRECAST ABUTMENT WINGWALL ELEVATION ELEMENT NE OR NW FOR ABUTMENT 2 SCALE 1:25

ELEMENT NE SHOWN, NW OPPOSITE ON CORNER (1 REQUIRED EACH CORNER)
(ELEMENT ESTIMATED WEIGHT: 12 400 kg, EACH)

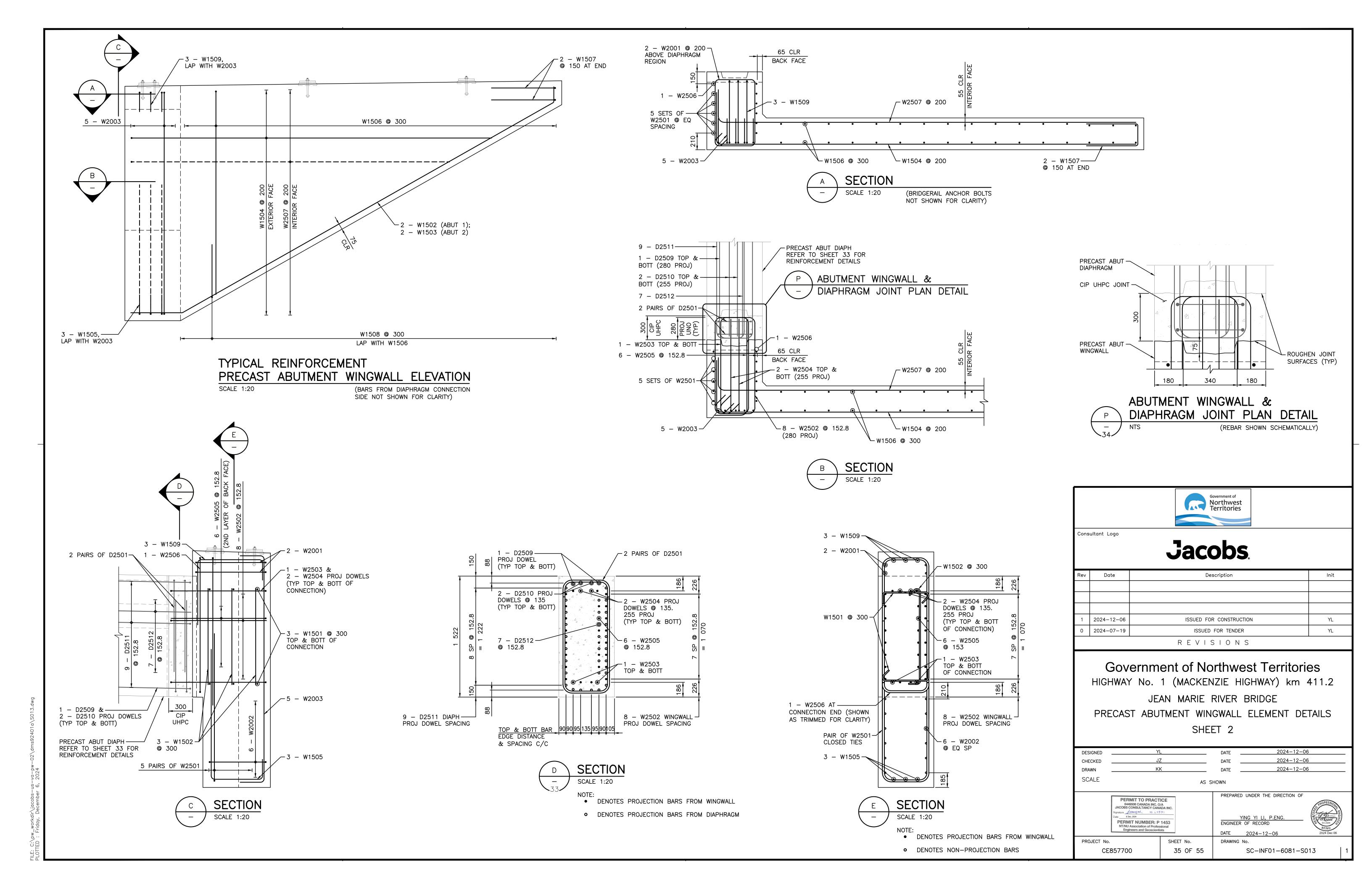
►ELEMENT SW | 12200 ELEMENT PLAN SCALE 1:50

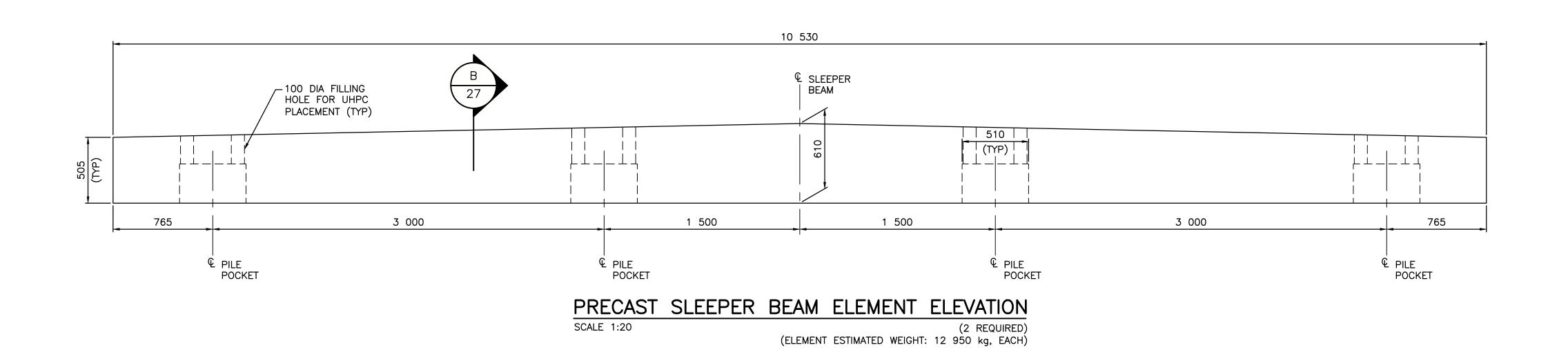
850 TYP)

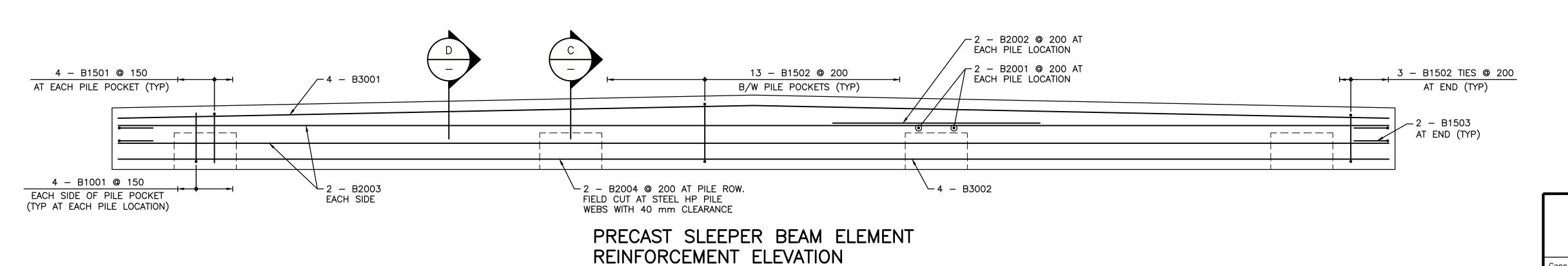
980

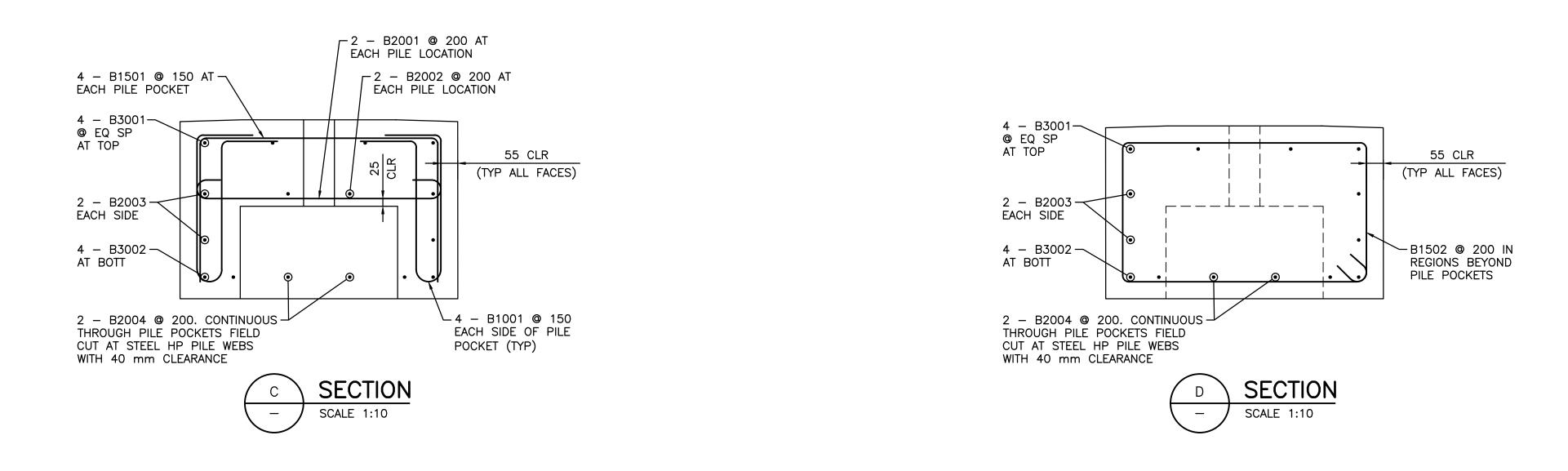
SCALE 1:25 (1 REQUIRED EACH CORNER)

PREPARED UNDER THE DIRECTION OF









SCALE 1:20



Consultant Logo

Jacobs

Rev	Date	Description	Init
1	2024-12-06	ISSUED FOR CONSTRUCTION	YL
0	2024-07-19	ISSUED FOR TENDER	YL
	•	REVISIONS	•

Government of Northwest Territories

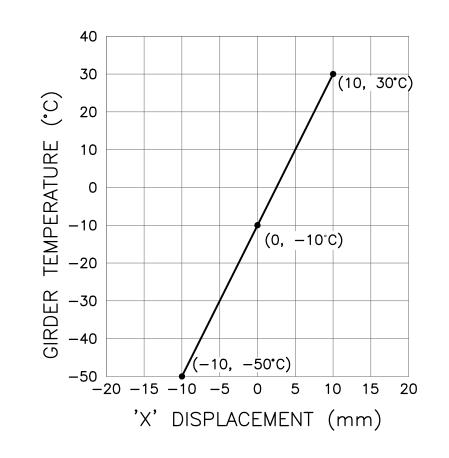
HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2

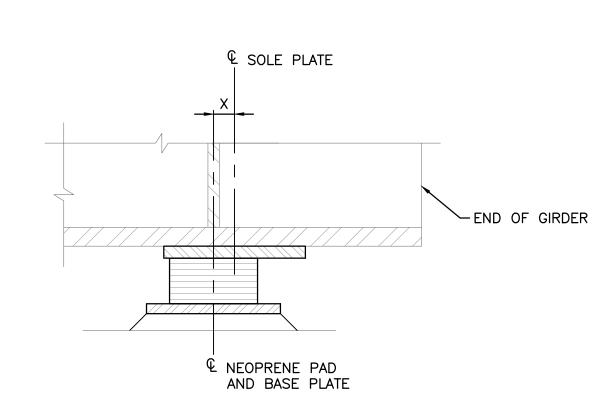
JEAN MARIE RIVER BRIDGE

PRECAST SLEEPER BEAM ELEMENT DETAILS

DESIGNED	YL	DATE	2024-12-06
CHECKED	JZ	DATE	2024-12-06
DRAWN	KK	DATE	2024-12-06
SCALE	AS	SHOWN	

PERMIT TO PRACT 6449506 CANADA INC. O JACOBS CONSULTANCY CAN Signature Assuring and TO L 43 Date 6 Dec. 2024 PERMIT NUMBER: P NT/NU Association of Profes Engineers and Geoscienti	/A ADA INC. 17-1 1453 sional	PREPARED UNDER THE DIRECTION OF YING YI LI, P.ENG. ENGINEER OF RECORD DATE 2024–12–06 PREPARED UNDER THE DIRECTION OF YOUR AND THE DIRECTION OF 2024 Dec 06)
PROJECT No.	SHEET No.	DRAWING No.	
CE857700	36 OF 55	SC-INF01-6081-S014	1





ABUTMENT EXPANSION BEARING SETTING CHART

'X' IS POSITIVE WHEN MEASURED FROM & BASE PLATE TOWARD NEAREST GIRDER END

BEARING NOTES

GENERAL

- 1. ALL BEARINGS SHALL BE DESIGNED BY BEARING MANUFACTURER AND STAMPED BY A PROFESSIONAL ENGINEER REGISTERED IN THE NORTHWEST TERRITORIES.
- 2. LAMINATED ELASTOMERIC BEARING PADS AND ALL OTHER COMPONENTS INCLUDING BUT NOT LIMITED TO TOP PLATES, STAINLESS STEEL SLIDING PLATES, BASE PLATES, CONNECTIONS, ANCHOR RODS SHALL BE DESIGNED IN ACCORDANCE WITH CAN/CSA-S6-19 AND THE ALBERTA TRANSPORTATION STRUCTURES DESIGN CRITERIA V9.0 APPENDIX D.
- 3. ALL REQUIREMENTS OF GNWT STANDARD SPECIFICATIONS FOR BRIDGE CONSTRUCTION (SSBC) SECTION 6 FOR THE SUPPLY OF STRUCTURAL STEEL AND SECTION 8 SHALL BE MET.
- 4. ABUTMENT SEAT AND PILE CUT-OFF ELEVATIONS, AND RELATED BRIDGE ELEMENT SIZES SHALL BE ADJUSTED TO ACCOMMODATE THE PROPOSED BEARING ASSEMBLIES AS PER ACCEPTED SHOP DRAWINGS. THE CONTRACTOR SHALL ENSURE THAT THE ROADWAY PROFILE IS MAINTAINED AS SHOWN. THE CONTRACTOR IS TO PROVIDE WRITTEN NOTIFICATION OF ANY CHANGE TO ELEVATIONS AND PRECAST CONCRETE ELEMENT DIMENSIONS INCLUDING ANY SUBSEQUENT CHANGES TO THE BRIDGE STRUCTURE. ANY ASSOCIATED COSTS INCURRED DUE TO SUCH CHANGES SHALL BE BORNE SOLELY BY THE CONTRACTOR.
- 5. FABRICATION OF BEARINGS SHALL NOT COMMENCE UNTIL SHOP DRAWINGS HAVE BEEN REVIEWED AND ACCEPTED BY THE ENGINEER.
- 6. THE CONTRACTOR SHALL ENGAGE A QUALIFIED INDEPENDENT TESTING COMPANY AT THE CONTRACTOR'S EXPENSE TO PERFORM TESTING OF THE BEARINGS. THE TESTING SHALL BE IN ACCORDANCE AS DESCRIBED IN SSBC.

INSTALLATION

- 1. BEARING GROUT PADS SHALL BE GROUTED AFTER GIRDERS ARE ERECTED AND INTERMEDIATE DIAPHRAGMS ARE INSTALLED. PRECAST CONCRETE DECK AND ABUTMENT DIAPHRAGM ELEMENTS INSTALLATION SHALL NOT BE STARTED UNTIL GROUT PADS HAVE A MINIMUM COMPRESSIVE STRENGTH OF 35 MPa.
- 2. ALL BEARINGS SHALL BE SUPPLIED COMPLETE WITH SOLE PLATES, BASE PLATES AND BOLTS AS DETAILED.
- 3. ALL WELDING SHALL CONFORM TO CURRENT AWS BRIDGE WELDING CODE D1.5.

MATERIALS

- 1. GROUT PADS AND ANCHOR BOLT VOIDS SHALL BE GROUTED WITH SIKA 212, FLOWABLE GROUT OR APPROVED EQUIVALENT. GROUT SHALL HAVE A MINIMUM 28 DAYS COMPRESSIVE STRENGTH OF 45 MPa AND SHALL NOT BE DRY PACKED.
- 2. SOLE PLATE STEEL SHALL CONFORM TO CSA G40.21M-350 AT CATEGORY (4) WITH CHARPY V-NOTCH STRENGTH OF 27 JOULES AT -45°C. THE STEEL FOR BASE PLATE, KEEPER BARS, PINTELS AND SHIMS SHALL CONFORM TO THE REQUIREMENTS OF CSA G40.21 GRADE 300W.
- 3. STAINLESS STEEL PLATES AND STUDS FOR ABUTMENT SEAT SHEAR BLOCKS SHALL CONFORM TO THE REQUIREMENTS OF THE AMERICAN IRON AND STEEL INSTITUTE (AISI) TYPE 304. THE CHEMICAL AND MECHANICAL PROPERTIES SHALL CONFORM TO THE REQUIREMENTS OF ASTM A240/A240M.

GALVANIZING

1. GALVANIZING SHALL BE HOT DIP METHOD AFTER FABRICATION IN ACCORDANCE WITH THE CURRENT EDITION OF ASTM A123/A123M STANDARD SPECIFICATION FOR ZINC (HOT-DIP GALVANIZED) COATINGS ON IRON AND STEEL PRODUCTS AND ASTM F2329 STANDARD SPECIFICATION FOR ZINC COATING (HOT-DIP).

JACKING LOADS

PERMANENT DEAD LOAD: 825 kN (UN-FACTORED)

PERMANENT DEAD LOAD +
1 LANE OF CL 800 TRUCK

(AT CENTER OF ROADWAY): 990 kN (UN-FACTORED)

(ABOVE JACKING LOADS PROVIDED BY ASSUMING NO BACKFILL SOIL FRICTION ON DIAPHRAGMS AND WINGWALLS DURING THE JACKING)

BEARING SCHEDULE							
	DEADIA	10		G1, G4, G5		G2 & G3	
	BEARIN	iG		VALUE	LC	VALUE	LC
			MAX	1320		1320	
		VERT	PERM	825		825	
	SLS		MIZ	825		825	
		LONG		135		135	
DESIGN		TRANS		135		135	
BEARING REACTION	ULS		MAX	1840	ULS1	1840	ULS1
(kN)		VERT	PERM	910	ULS1	910	ULS1
			MIN	750	ULS1	750	ULS1
		LONG			455 (ULS 5)*		
		TRANS		455 (ULS 5)*			
	FLS	VERT		420		420	
DESIGN BEARING		LONG		±60		±60	
MOVEMENT (mm)		TRANS		±12		±2	
DESIGN BEARING ROTATION (rad)	SLS	LONG		0.025		0.025	
	JLJ	TRANS		_		_	

NOTE: *	BEARING TRAVERSE LOAD IS RESISTED BY EACH SHEAR BLOG	٦Ł
	LONGITUDINAL LOAD IS RESISTED BY BACKFILL SOIL BEHIND	
	EACH ABUTMENT DIAPHRAGM.	

GIRDER AND BEARIN	NG FLF		Ω TΔ	OF BF	ARING							
GIRDER AND BEARING ELEVATIONS AT Q OF BEARING												
ABUT 1 (SOUTH)	G1	G2	G3	G4	G5							
TOP OF GROUT PAD ELEVATION (m)												
BEARING HEIGHT (mm)												
UNDERSIDE OF GIRDER ELEVATION (m)	206.333	206.377	206.421	206.377	206.333							
ABUT 2 (NORTH)	G1	G2	G3	G4	G5							
TOP OF GROUT PAD ELEVATION (m)												
BEARING HEIGHT (mm)												
UNDERSIDE OF GIRDER ELEVATION (m)	206.823	206.867	206.911	206.867	206.823							

NOTE: TOP OF GROUT PAD ELEVATIONS AND BEARING HEIGHTS TO BE DETERMINED AFTER BEARING SHOP DRAWINGS ARE APPROVED AT CONSTRUCTION STAGE WHEN THE DESIGN BEARING HEIGHTS ARE KNOWN

	Government of Northwest Territories

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PROJECT No.

CE857700

Jacobs

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1	2024-12-06	ISSUED FOR CONSTRUCTION	YL
0	2024-07-19	ISSUED FOR TENDER	YL
		REVISIONS	

Government of Northwest Territories
HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2

JEAN MARIE RIVER BRIDGE BEARING LAYOUT

DESIGNED	YL	DATE	2024-12-06
CHECKED	JZ	DATE	2024-12-06
DRAWN	KK	DATE	2024-12-06
SCALE			
SCALE	AS	SHOWN	

PERMIT TO PRACTICE
6449506 CANADA INC. O/A
JACOBS CONSULTANCY CANADA INC.
Signature 10 Dec 2024

PERMIT NUMBER: P 1453
NT/NU Association of Professional
Engineers and Geoscientists

PATE

PREPARED UNDER THE DIRECTION OF

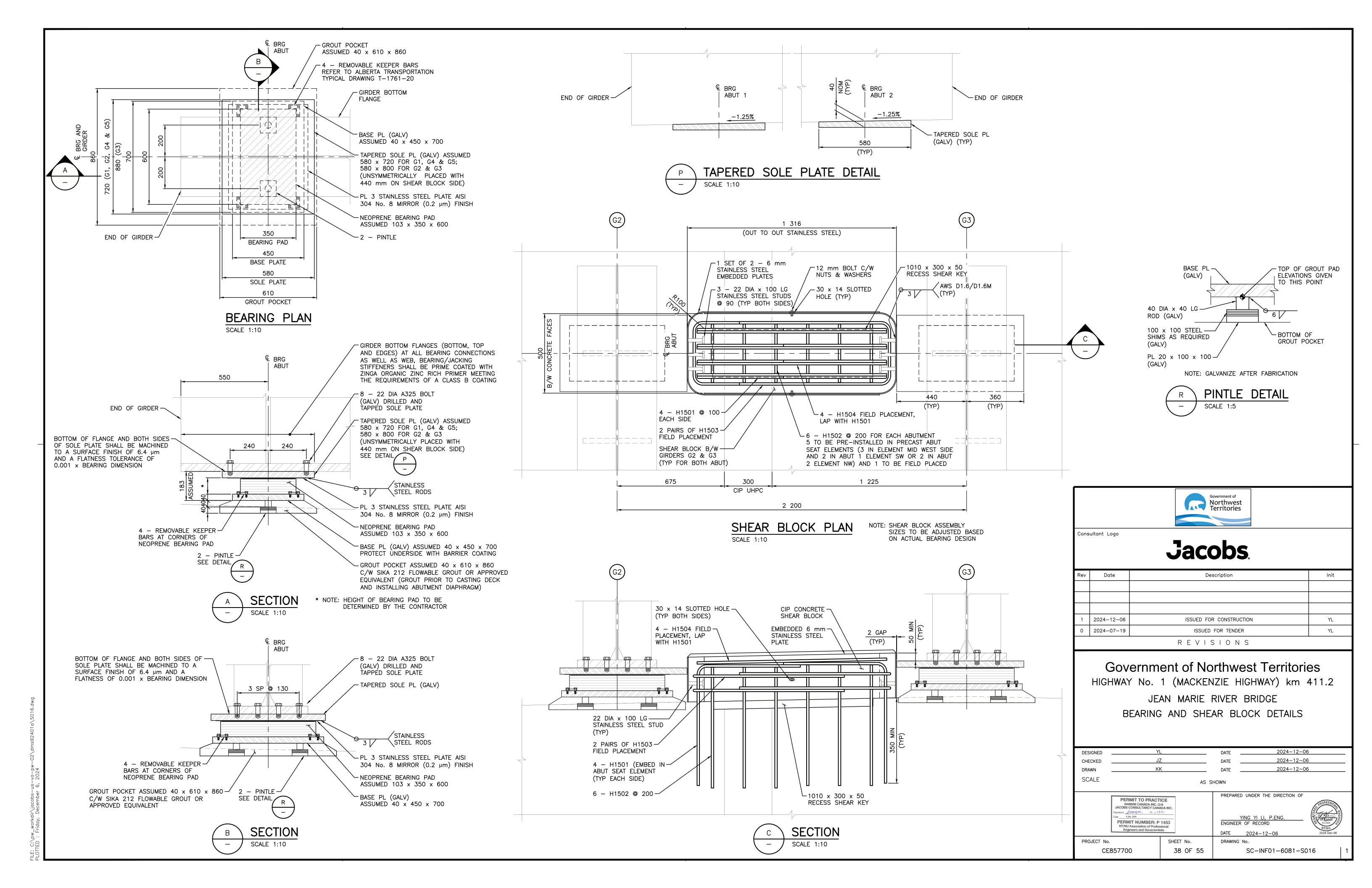
YING YI LI, P.ENG.

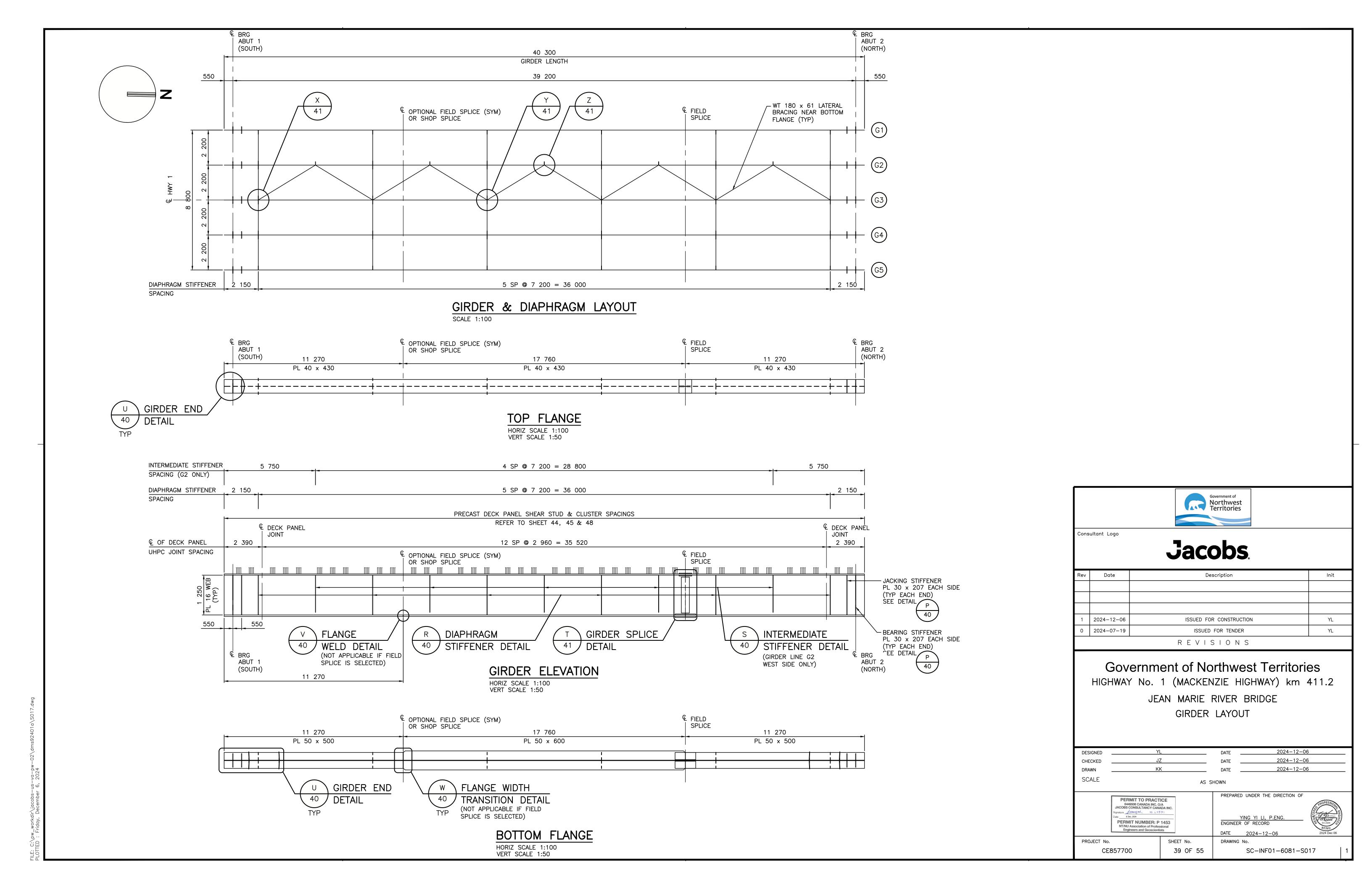
ENGINEER OF RECORD

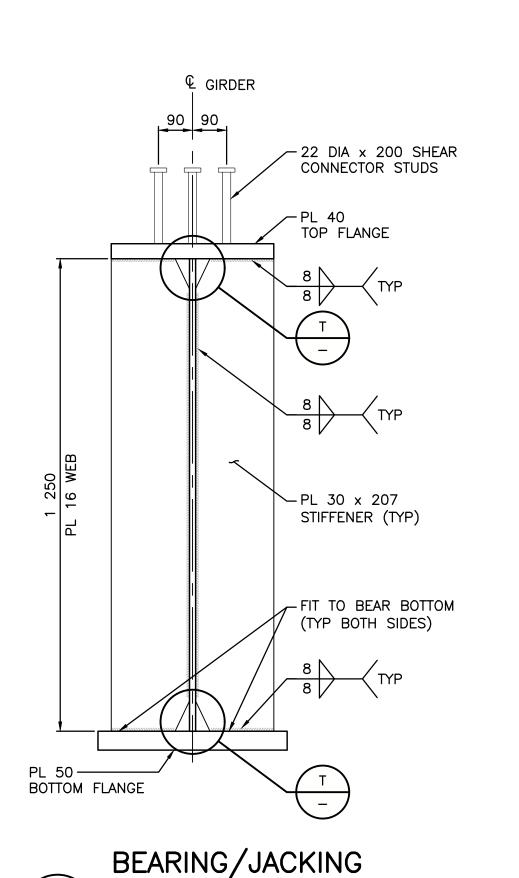
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 DRAWING No.

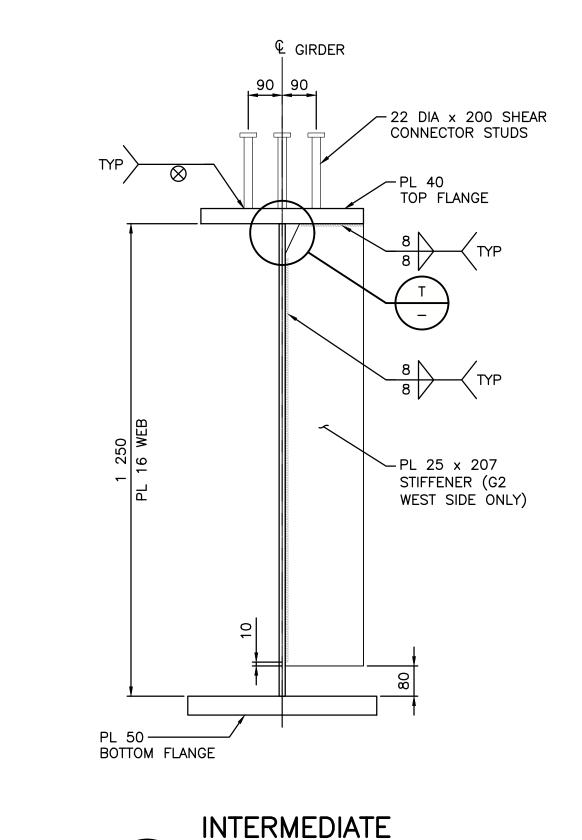
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 SC.

SC-INF01-6081-S015





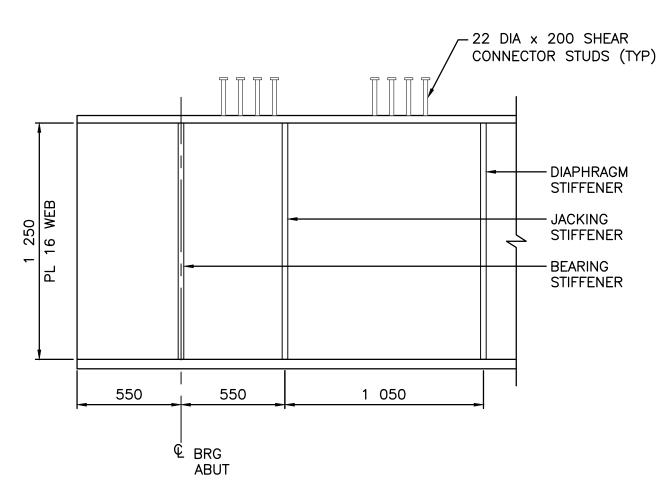




STIFFENER DETAIL

SCALE 1:10

39



1 600

1 050

- 22 DIA × 200 SHEAR

CONNECTOR STUDS (TYP)

└-PL 16 WEB

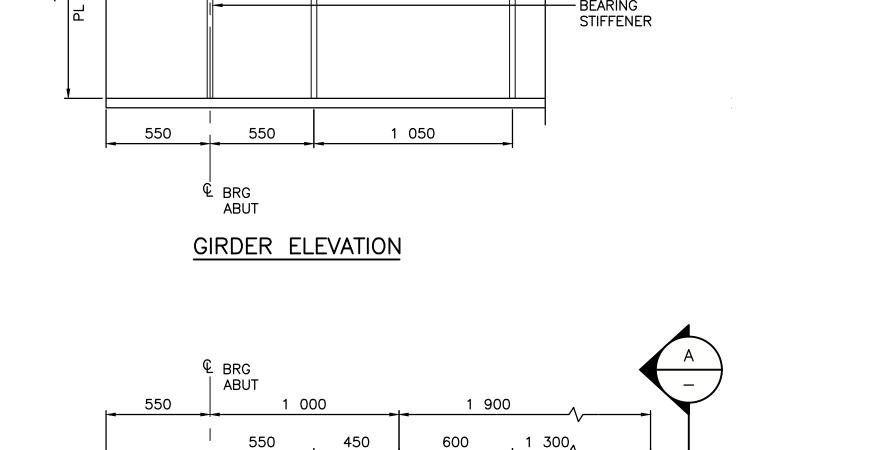
TOP FLANGE PLAN

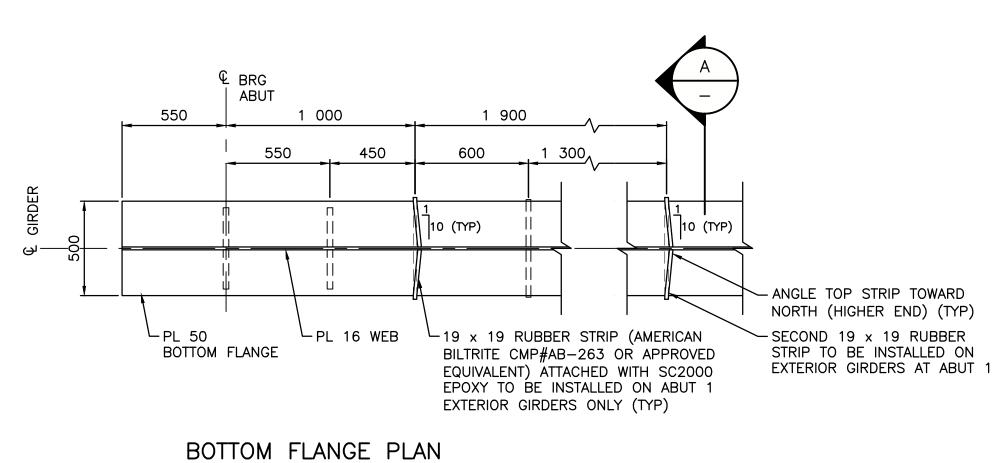
€ BRG | ABUT

TOP FLANGE

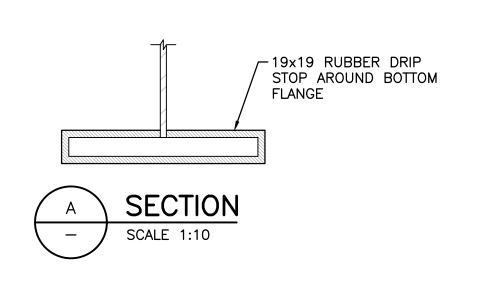
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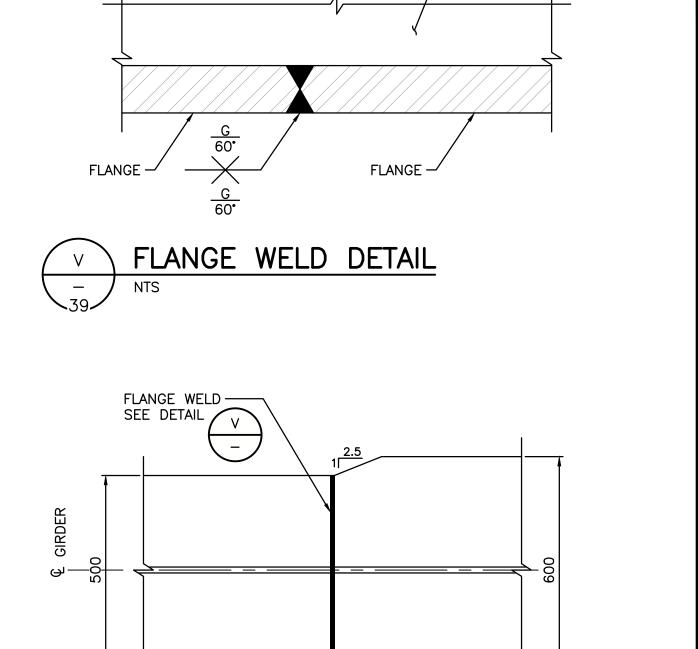
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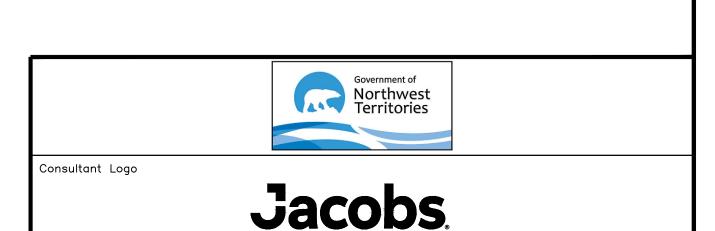






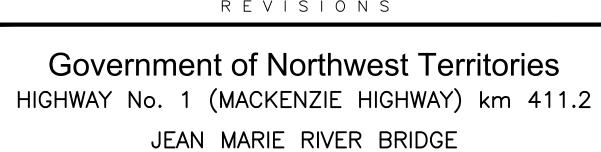


__ PL 16 WEB



FLANGE WIDTH TRANSITION DETAIL

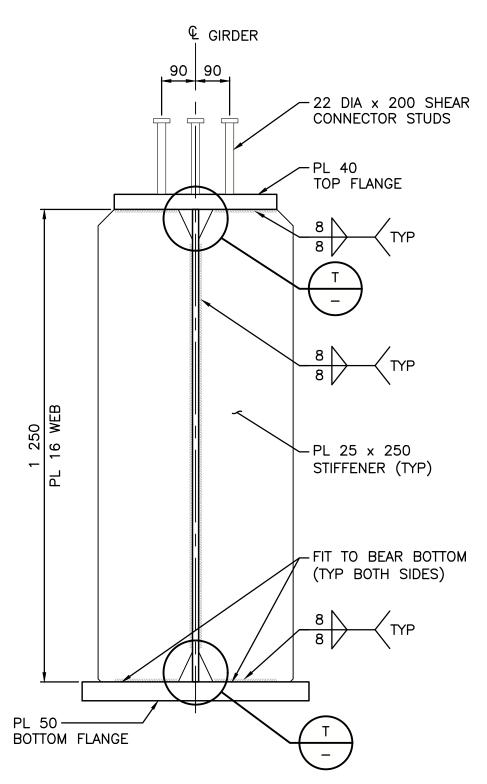
Rev	Rev Date Description								
1	2024-12-06	ISSUED FOR CONSTRUCTION	YL						
0	2024-07-19	ISSUED FOR TENDER	YL						
		R F V I S I O N S							



GIRDER DETAILS SHEET 1

DESIGNED	YL	DATE	2024-12-06
CHECKED	JZ	DATE	2024-12-06
DRAWN	KK	DATE	2024-12-06
SCALE	AS	SHOWN	

PERMIT TO PRACT 6449506 CANADA INC. O. JACOBS CONSULTANCY CANA Signature Assurdad. 20 L/3 Date 6 Dec. 2024 PERMIT NUMBER: P NT/NU Association of Profess Engineers and Geoscientis	ADA INC. ₹1 1453 stonal	PREPARED UNDER THE DIRECTION OF YING YI LI, P.ENG. ENGINEER OF RECORD DATE 2024–12–06	PROFESSION CONTROL OF THE PROPERTY OF THE PROP
PROJECT No. CE857700	SHEET No. 40 OF 55	DRAWING No. SC-INF01-6081-S	018

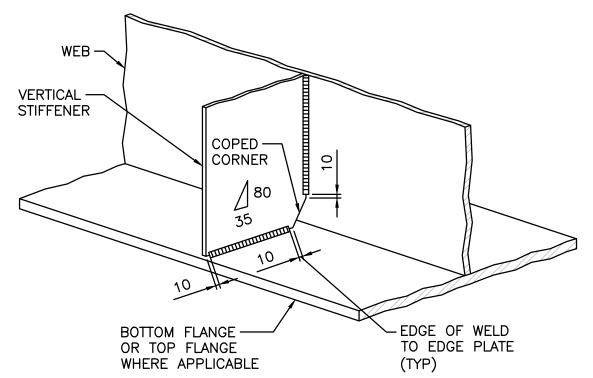


STIFFENER DETAIL

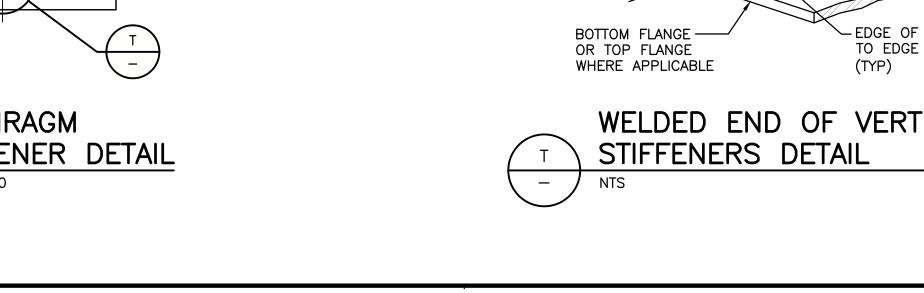
SCALE 1:10

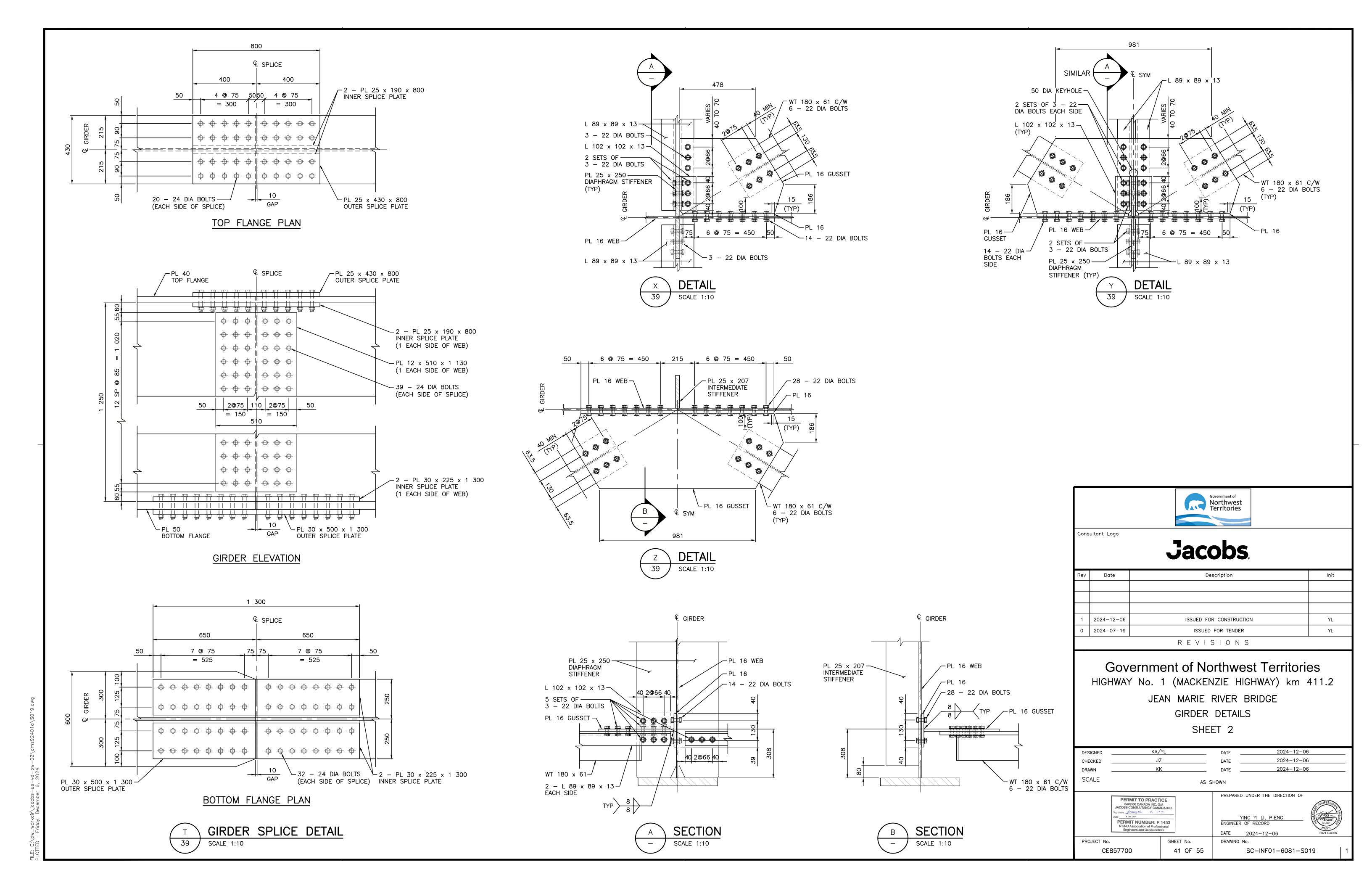
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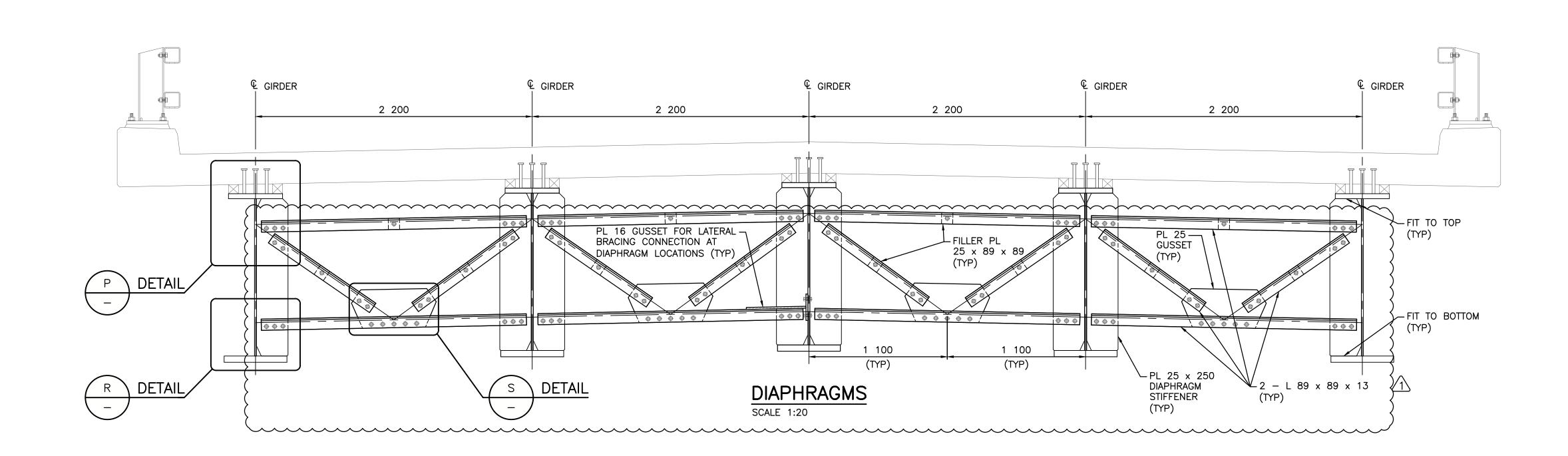


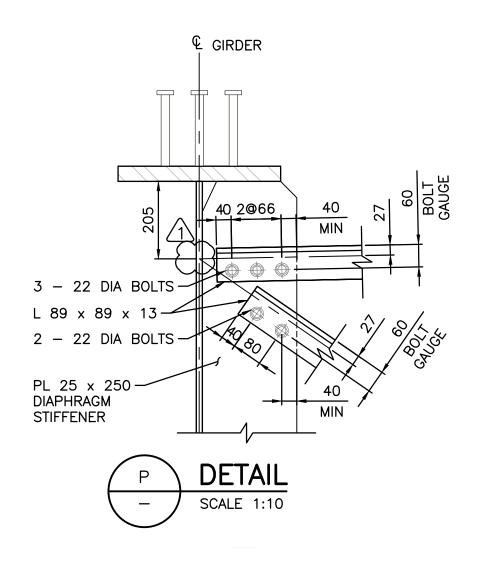


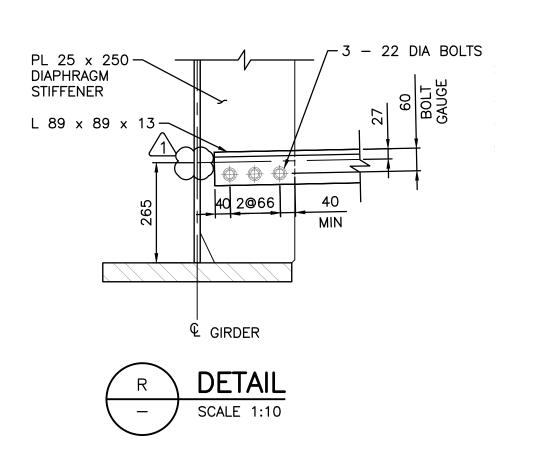


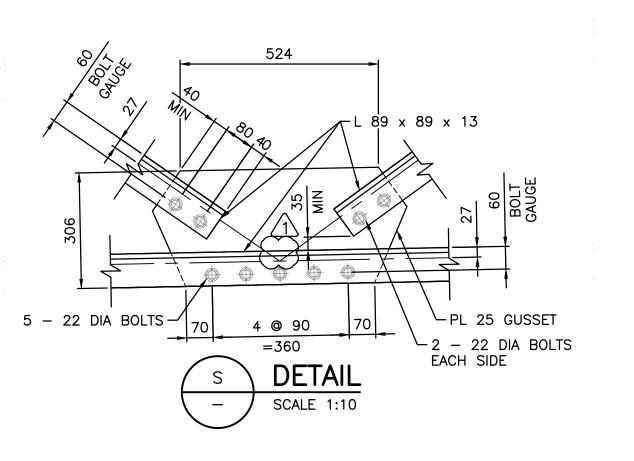














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0	2024-07-19	ISSUED FOR TENDER	YL
	•	REVISIONS	

Government of Northwest Territories

HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2 JEAN MARIE RIVER BRIDGE

> GIRDER DETAILS SHEET 3

2024-12-06 2024-12-06 SCALE

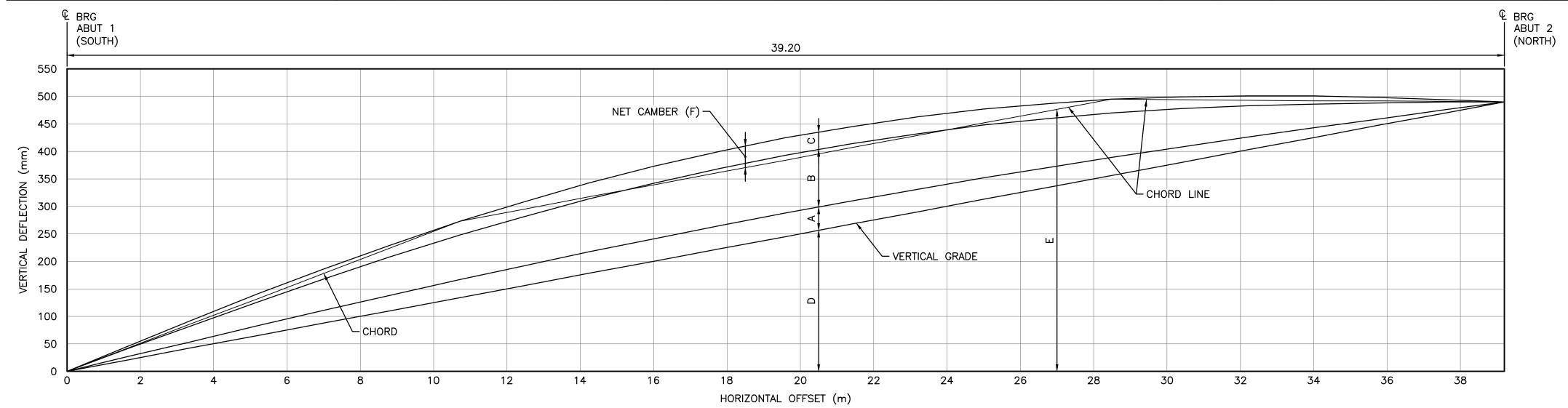
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Date	6 Dec. 2024			l <u> </u>
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42 OF 55

CE857700

REPARED UNDER THE DIRECTION OF SC-INF01-6081-S020

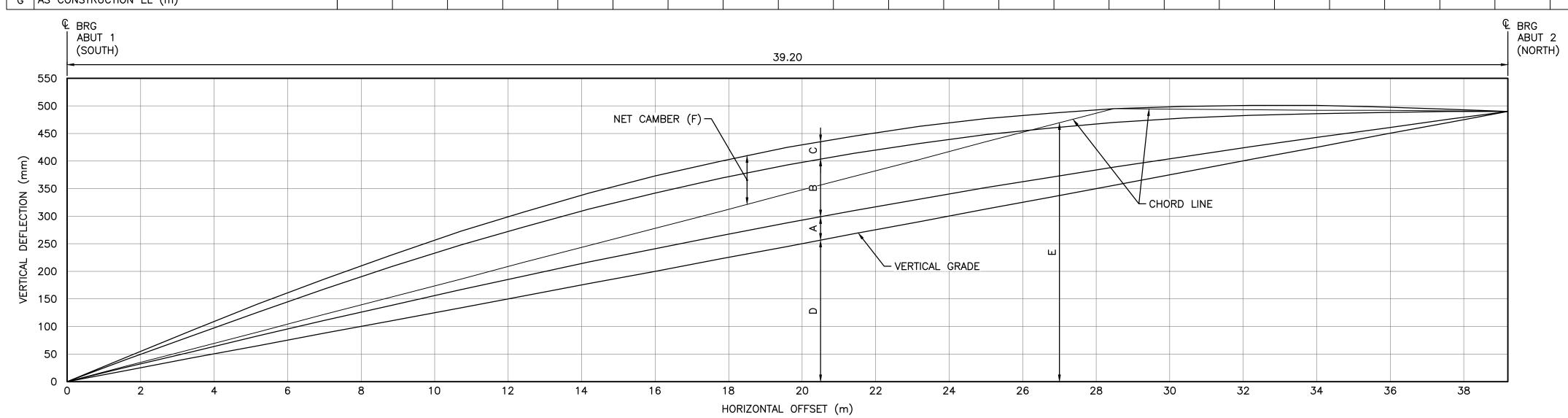
				C	AMBEF	R DIA	GRAM	TABL	E –	FOR	TWO	FIELD	SPL	ICES	OPTIO	N								
DIST	TANCE TO ABUT 1 & BRG (m)	0.00	1.60	3.40	5.20	7.00	8.80	10.72	12.40	14.20	16.00	17.80	19.60	21.40	23.20	25.00	26.80	28.48	30.40	32.20	34.00	35.80	37.60	39.20
LOC	CATION NOTE	€ BRG ABUT 1						FIELD SPLICE										FIELD SPLICE						© BRG ABUT 2
Α	DEAD LOAD - GIRDER (mm)	0	6	12	18	23	28	33	36	39	41	43	43	43	41	39	36	33	28	23	18	12	6	0
В	DEAD LOAD - DECK + HAUNCH + DIAPHRAGM (mm)	0	14	29	43	57	69	81	89	96	101	104	105	104	101	96	89	81	69	57	43	29	14	0
С	DECK SHRINKAGE + DEAD LOAD OF RAIL (mm)	0	5	10	14	18	22	25	27	29	30	31	31	31	30	29	27	25	22	18	14	10	5	0
D	VERTICAL GRADE (mm)	0	20	43	65	88	110	134	155	178	200	223	245	268	290	313	335	356	380	403	425	448	470	490
Ε	CHORD (mm)	0	41	87	132	178	224	273	294	317	339	362	384	407	429	452	474	495	494	493	492	492	491	490
F	NET CAMBER (mm)	0	4	7	8	8	5	0	13	25	34	39	41	39	34	25	13	0	5	8	8	7	4	0
G	AS CONSTRUCTION EL (m)																							



CAMBER DIAGRAM - FOR TWO FIELD SPLICES OPTION

HORIZ SCALE 1:75 VERT SCALE 1:5

				C	CAMBE	R DIA	AGRAM	1 TAB	LE -	FOR	ONE	FIELI) SPI	LICE	OPTIO	N								
DIS	TANCE TO ABUT 1 & BRG (m)	0.00	1.60	3.40	5.20	7.00	8.80	10.72	12.40	14.20	16.00	17.80	19.60	21.40	23.20	25.00	26.80	28.48	30.40	32.20	34.00	35.80	37.60	39.20
LOC	CATION NOTE	© BRG ABUT 1						SHOP SPLICE										FIELD SPLICE						© BRG ABUT 2
Α	DEAD LOAD - GIRDER (mm)	0	6	12	18	23	28	33	36	39	41	43	43	43	41	39	36	33	28	23	18	12	6	0
В	DEAD LOAD - DECK + HAUNCH + DIAPHRAGM (mm)	0	14	29	43	57	69	81	89	96	101	104	105	104	101	96	89	81	69	57	43	29	14	0
С	DECK SHRINKAGE + DEAD LOAD OF RAIL (mm)	0	5	10	14	18	22	25	27	29	30	31	31	31	30	29	27	25	22	18	14	10	5	0
D	VERTICAL GRADE (mm)	0	20	43	65	88	110	134	155	178	200	223	245	268	290	313	335	356	380	403	425	448	470	490
Ε	CHORD (mm)	0	28	59	90	122	153	186	216	247	278	309	341	372	403	435	466	495	494	493	492	492	491	490
F	NET CAMBER (mm)	0	17	34	50	65	76	87	92	95	95	91	84	73	59	42	21	0	5	8	8	7	4	0
G	AS CONSTRUCTION EL (m)																							



CAMBER DIAGRAM — FOR ONE FIELD SPLICE OPTION

HORIZ SCALE 1:75 VERT SCALE 1:5

NOTES

- 1. NET CAMBER (F) = A + B + C + D E
- 2. "NET CAMBER" FIGURES ARE FOR GIRDER SECTIONS IN ZERO LOAD CONDITIONS.
- FABRICATOR IS RESPONSIBLE FOR MAKING ALLOWANCES SUCH THAT CAMBER ON COMPLETED GIRDER SECTIONS WILL BE WITHIN ALLOWABLE TOLERANCES. ALL PROCEDURES FOR CAMBER ADJUSTMENT MUST BE APPROVED BY THE ENGINEER PRIOR TO USF.
- 4. LONGITUDINAL GIRDER DIMENSIONING (SHOWN HORIZONTAL) IS CORRECT ALONG THE BOTTOM FLANGE AT 20 °C.



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1	2024-12-06	ISSUED FOR CONSTRUCTION	YL
0	2024-07-19	ISSUED FOR TENDER	YL
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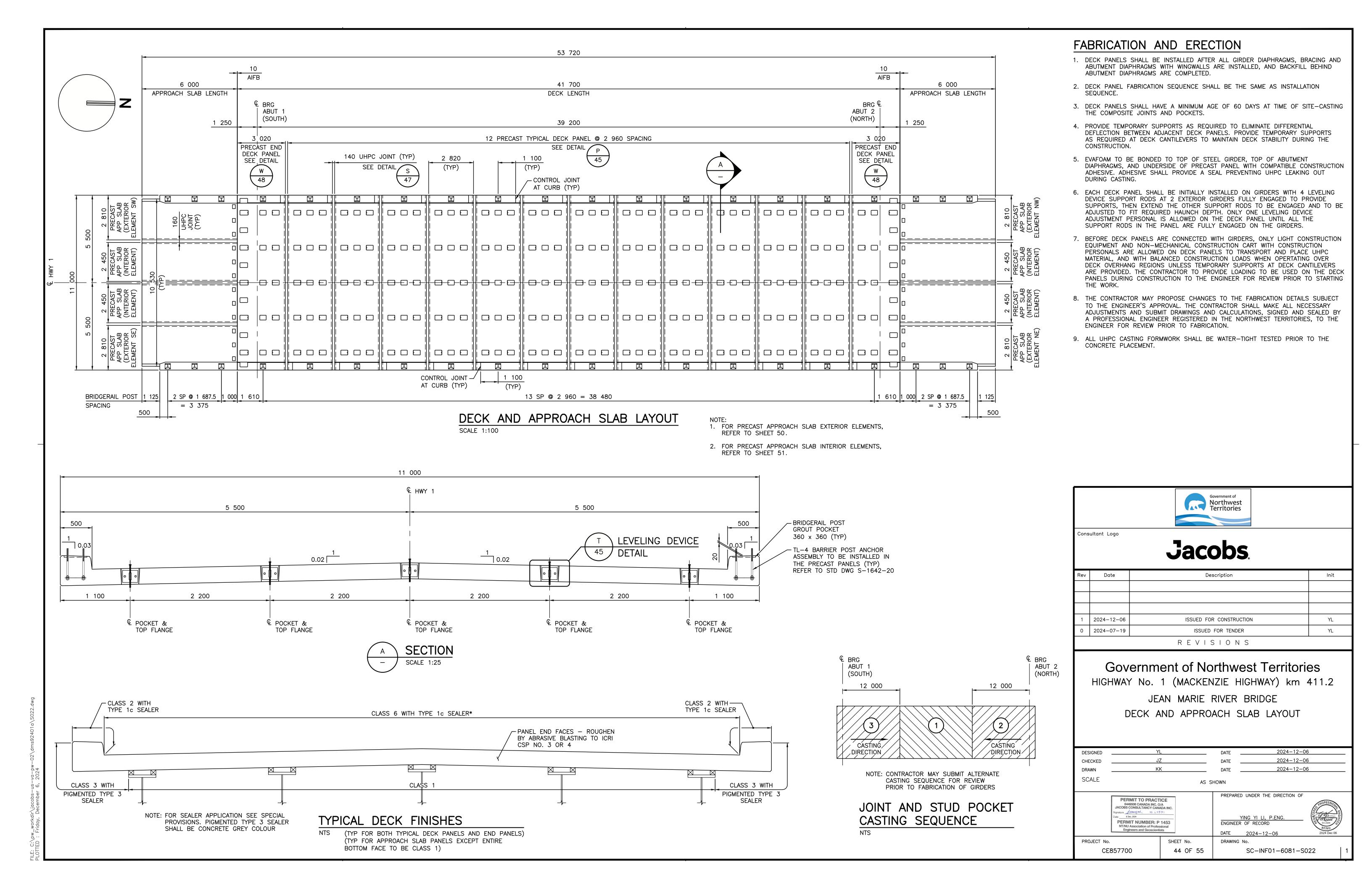
Government of Northwest Territories HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2

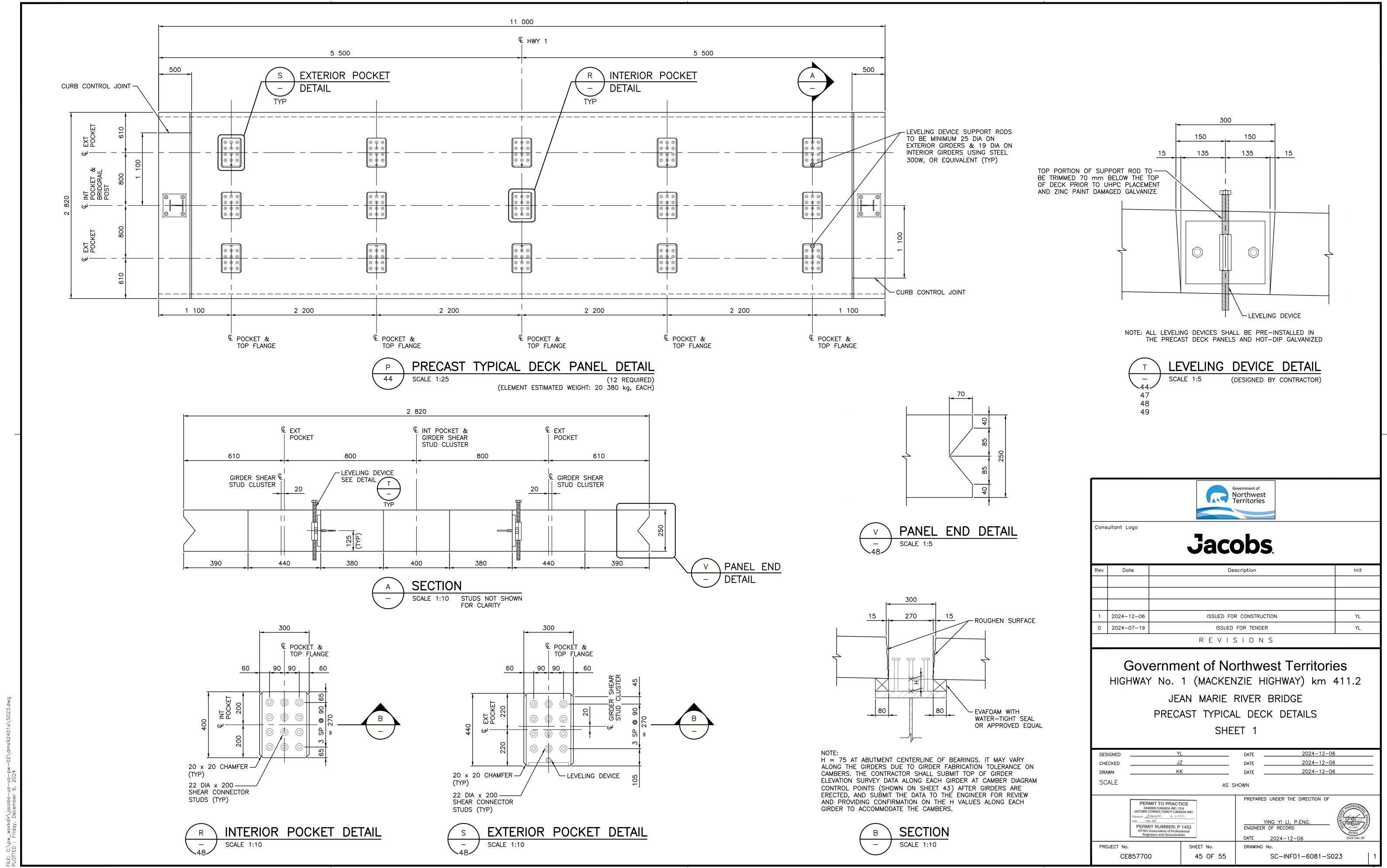
JEAN MARIE RIVER BRIDGE GIRDER CAMBER DIAGRAM

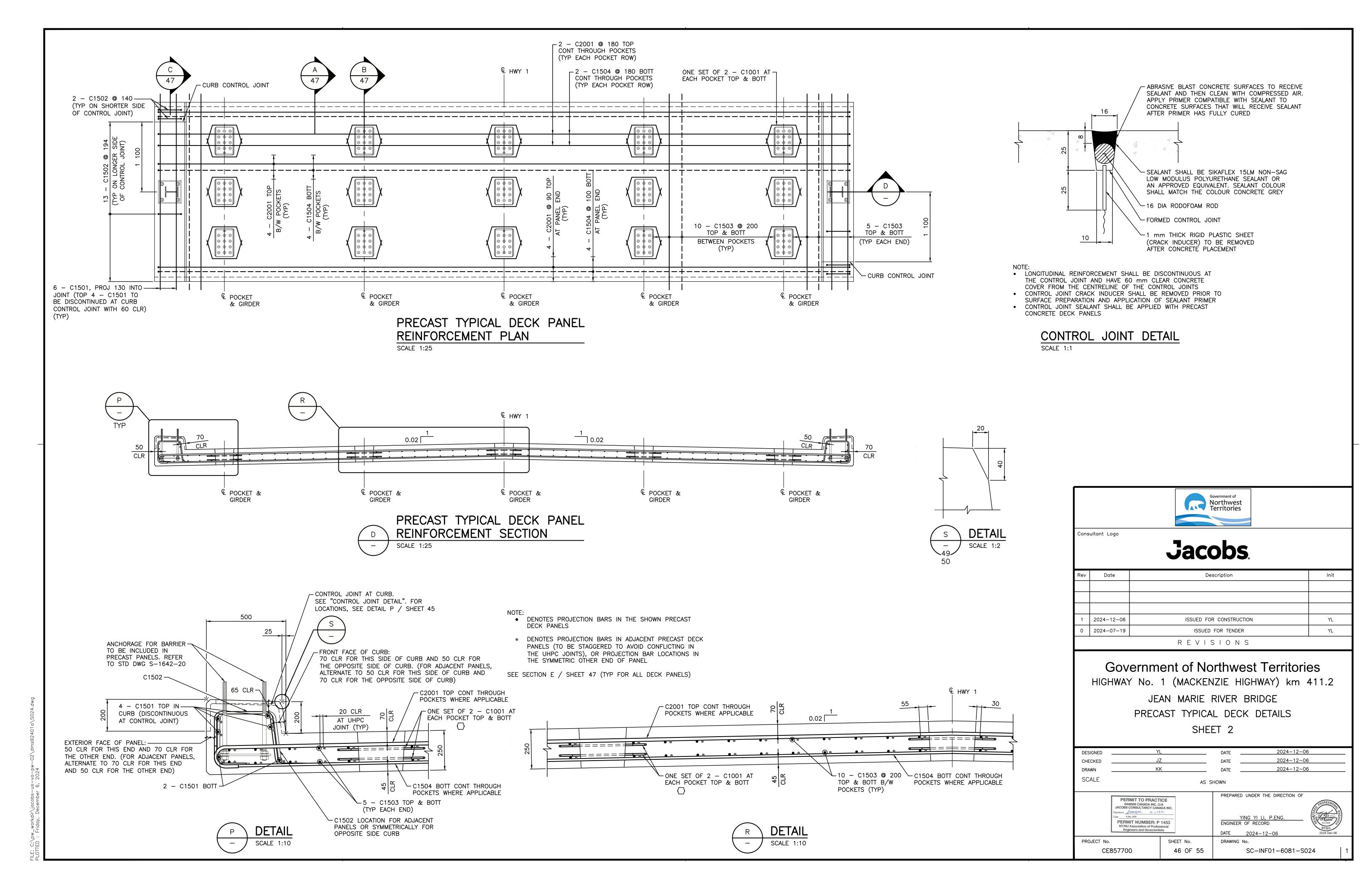
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CHECKED	JZ	DATE	2024-12-06
DRAWN	KK	DATE	2024-12-06
SCALE	AS	SHOWN	

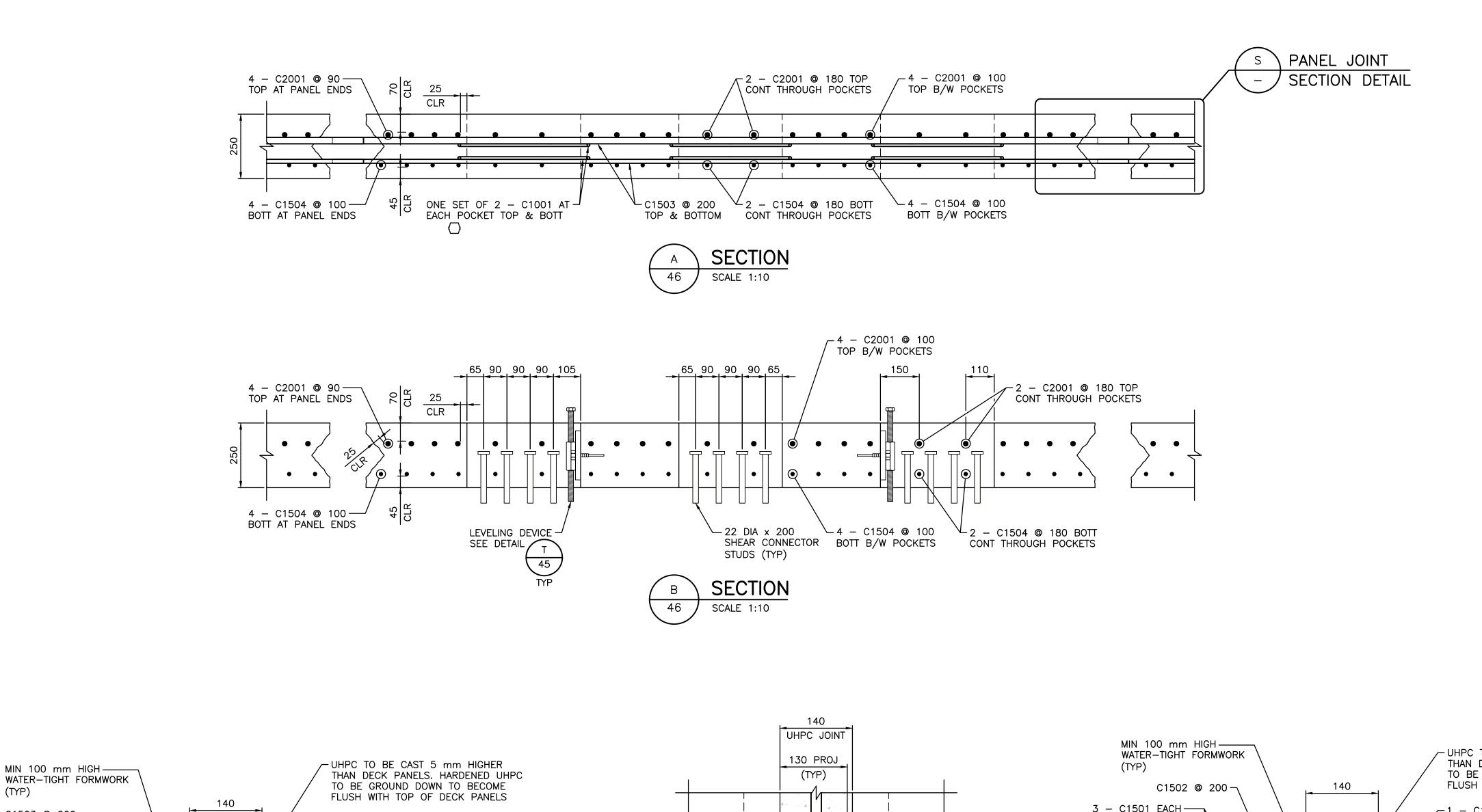
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PERMIT NUMBER: P NT/NU Association of Profes Engineers and Geoscienti	sional	YING YI LI, P.ENG. ENGINEER OF RECORD DATE 2024–12–06	NT/N 2024 E
PROJECT No.	SHEET No.	DRAWING No.	
CE857700	43 OF 55	SC-INF01-6081-S021	

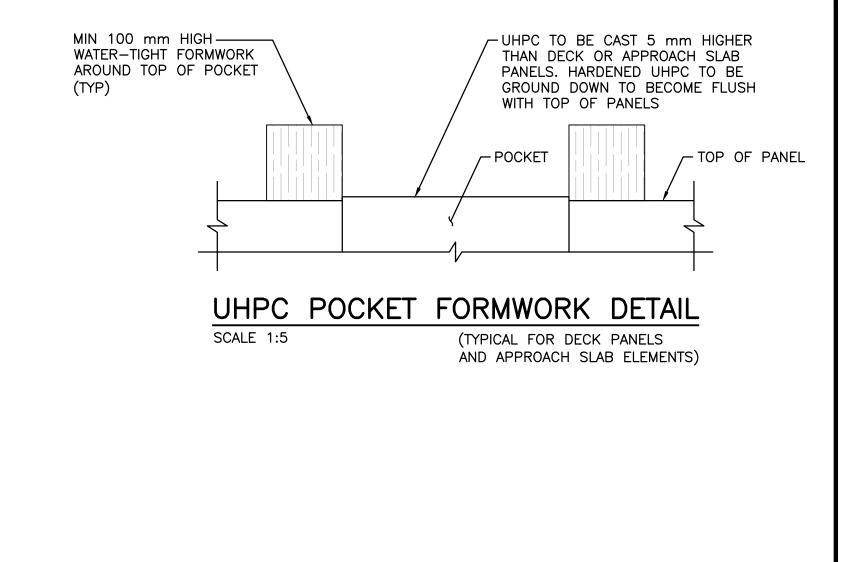
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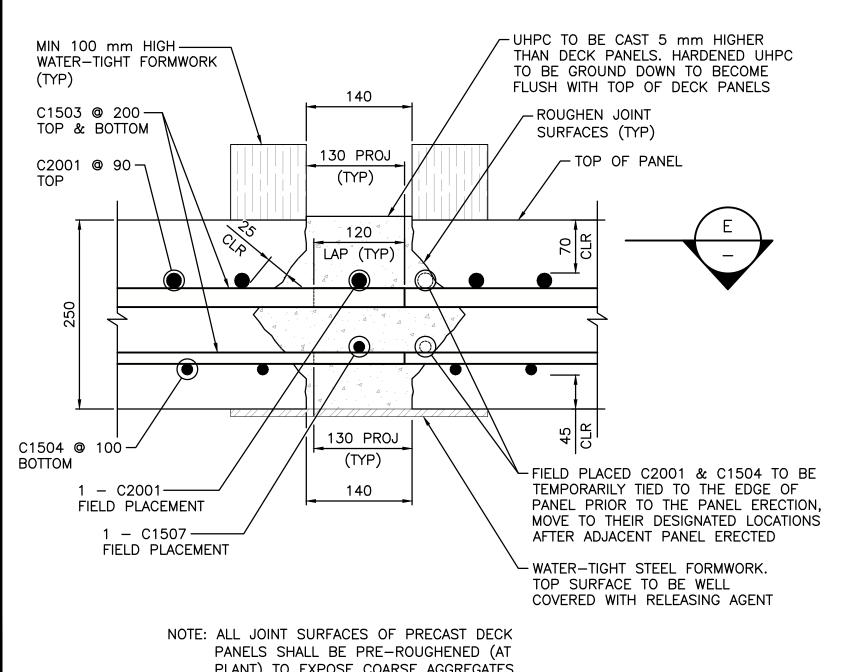


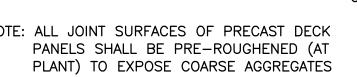




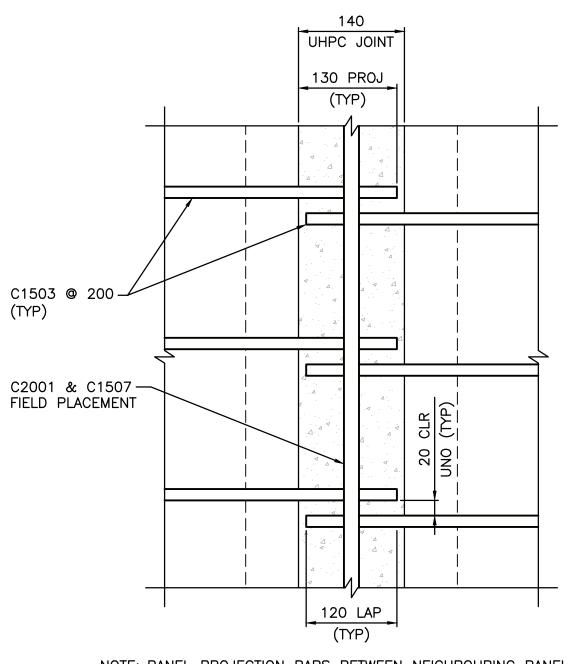






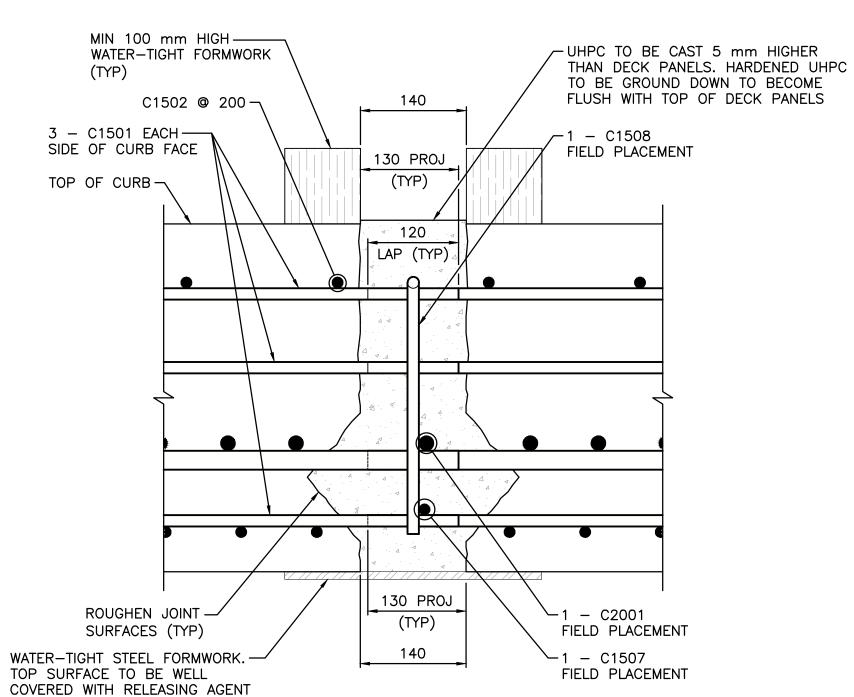


DECK PANEL JOINT SECTION DETAIL



NOTE: PANEL PROJECTION BARS BETWEEN NEIGHBOURING PANELS SHALL BE PLACED STAGGERED AS SHOWN TO MAINTAIN 20 mm SPLICE CLEARANCE IN THE UHPC JOINT. OTHER BARS NOT SHOWN FOR CLARITY

DECK PANEL JOINT PLAN SECTION SCALE 1:5



DECK PANEL JOINT SECTION AT CURB SCALE 1:5



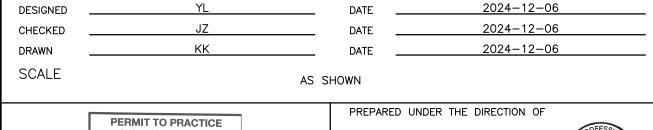
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Rev	Date	Description	Init
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0	2024-07-19	ISSUED FOR TENDER	YL
		REVISIONS	

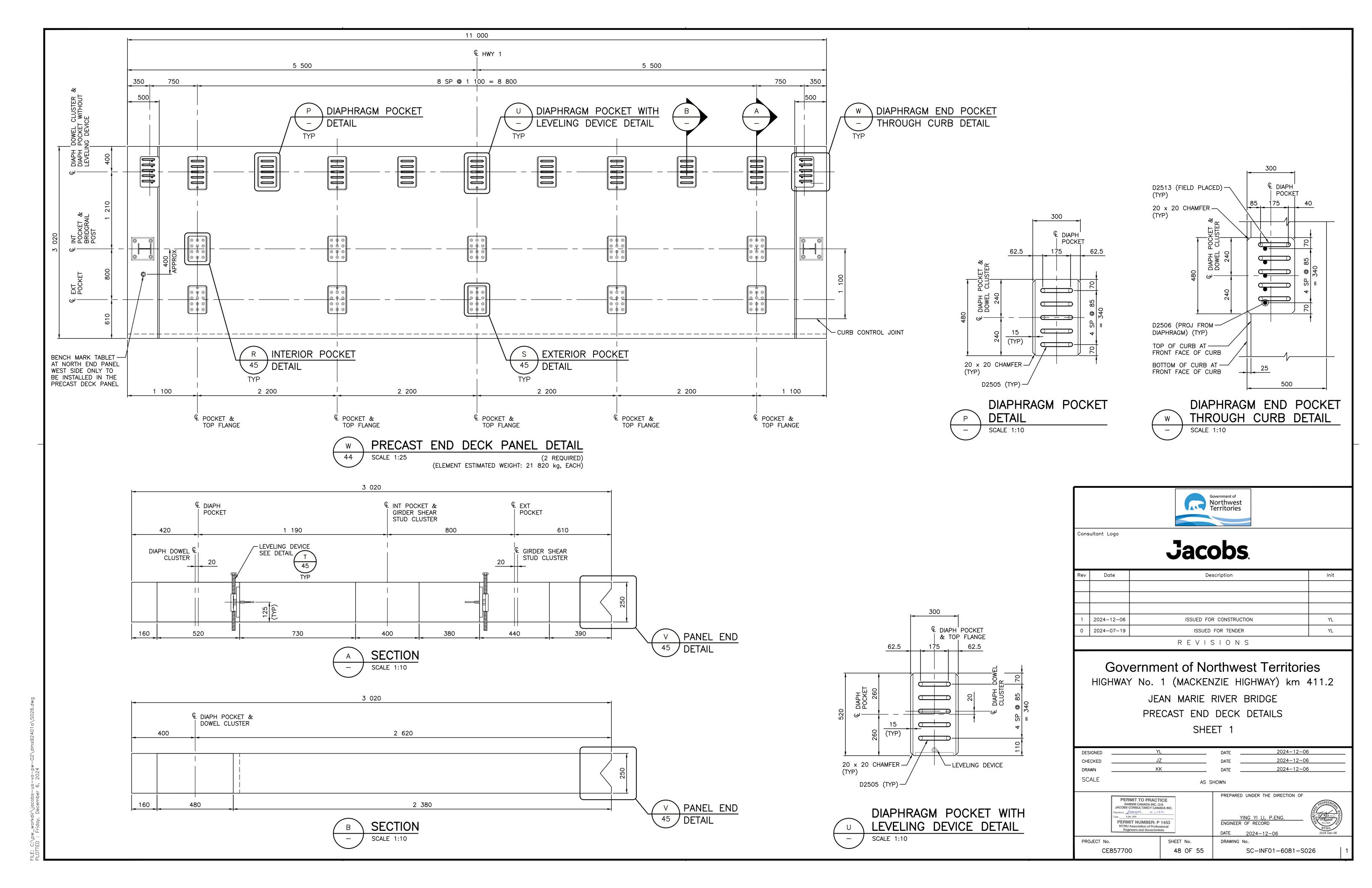
Government of Northwest Territories HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2

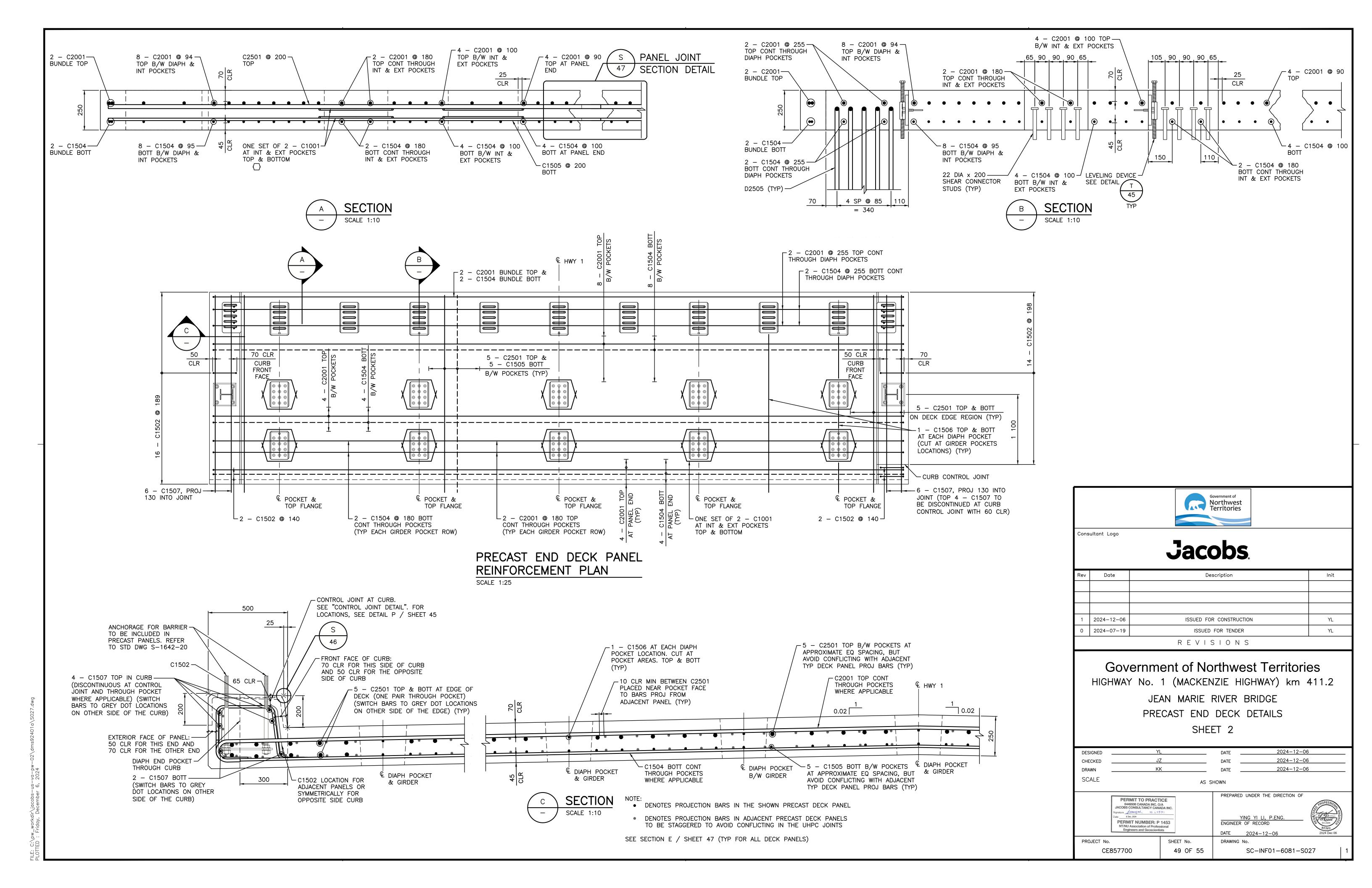
JEAN MARIE RIVER BRIDGE PRECAST TYPICAL DECK DETAILS

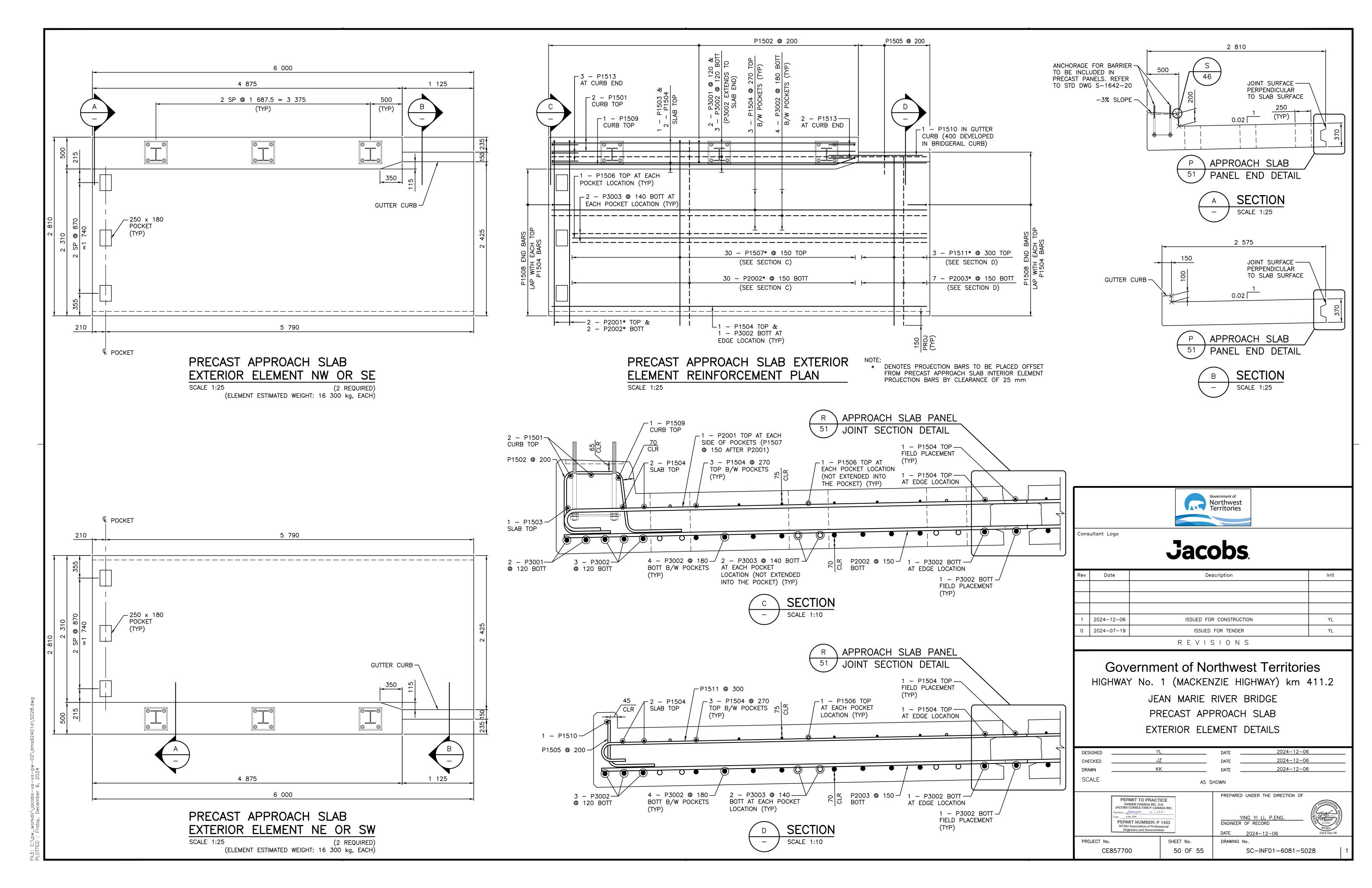
SHEET 3

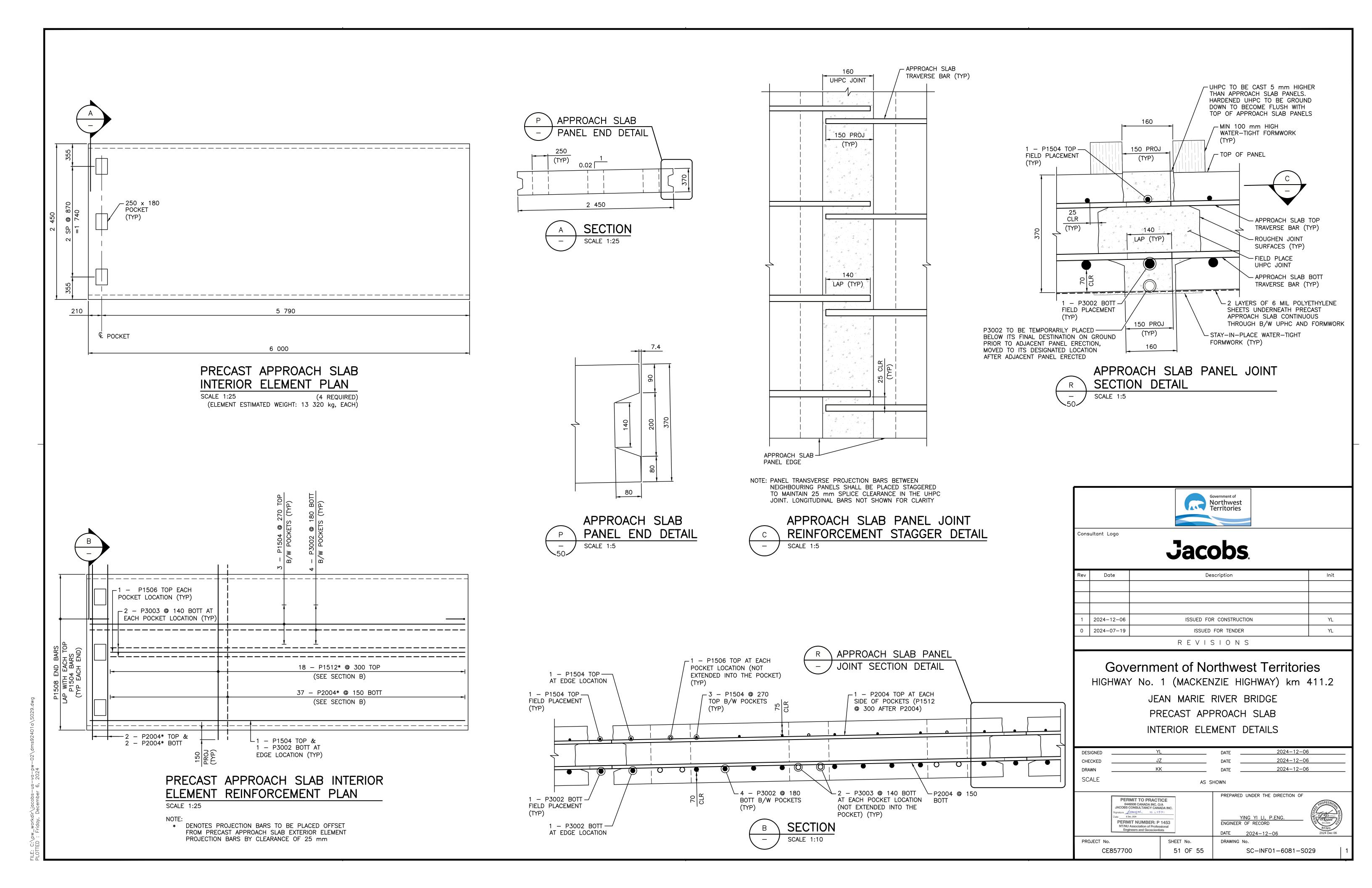


PERMIT TO PRACTICE YING YI LI, P.ENG ENGINEER OF RECORD PERMIT NUMBER: P 1453 NT/NU Association of Professional Engineers and Geoscientists PROJECT No. DRAWING No. SHEET No. 47 OF 55 SC-INF01-6081-S025 CE857700



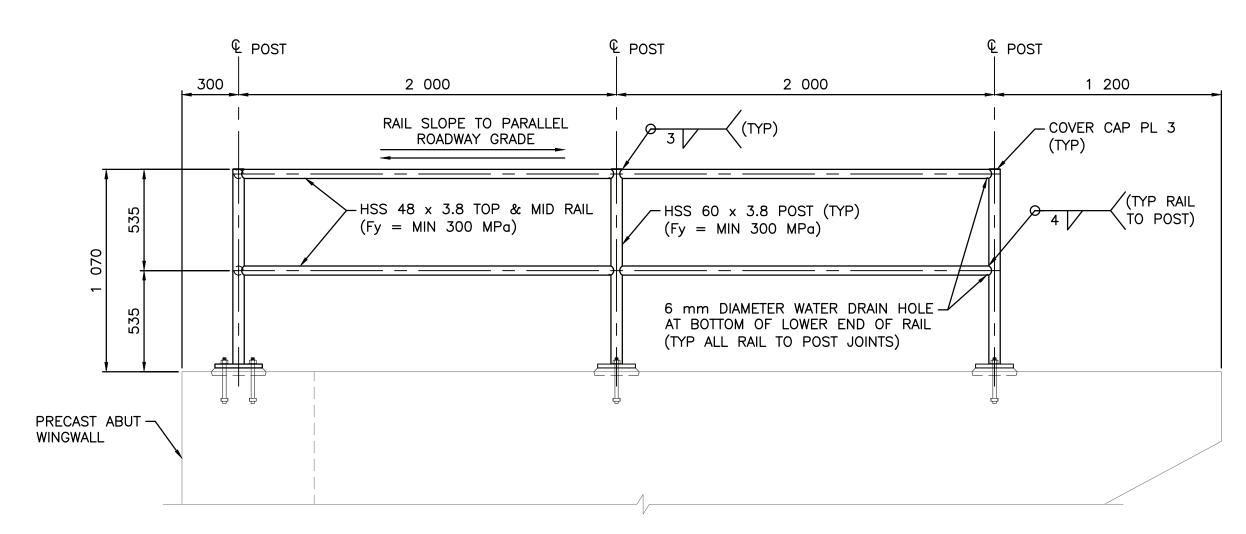




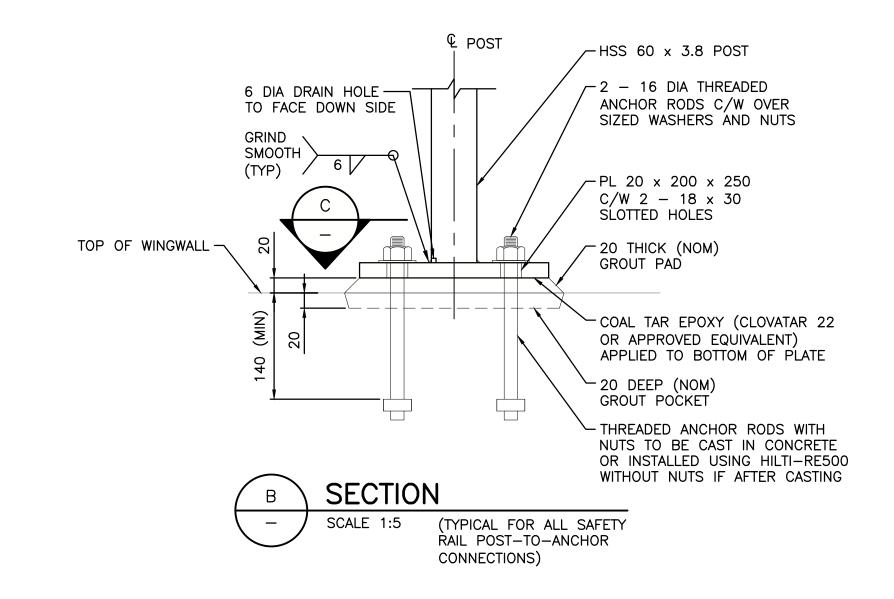


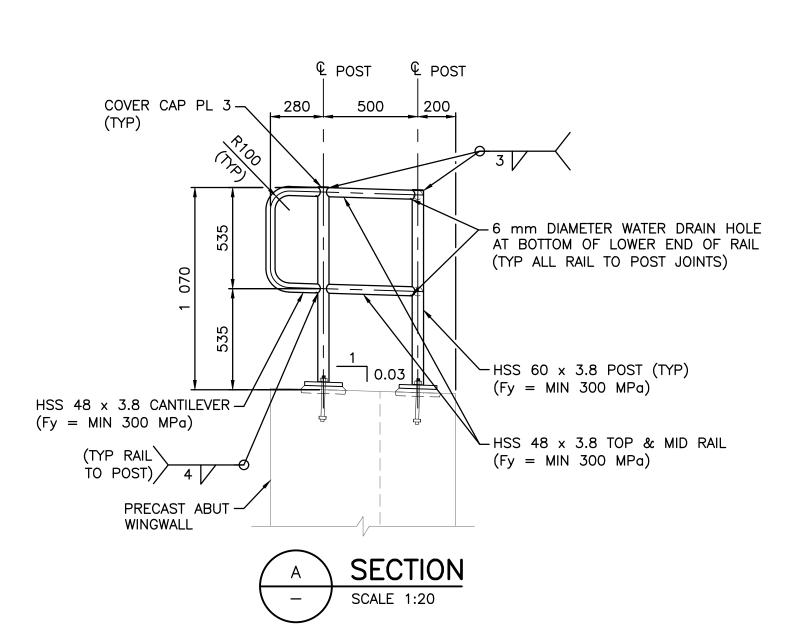
SAFETY RAIL ANCHOR PLAN

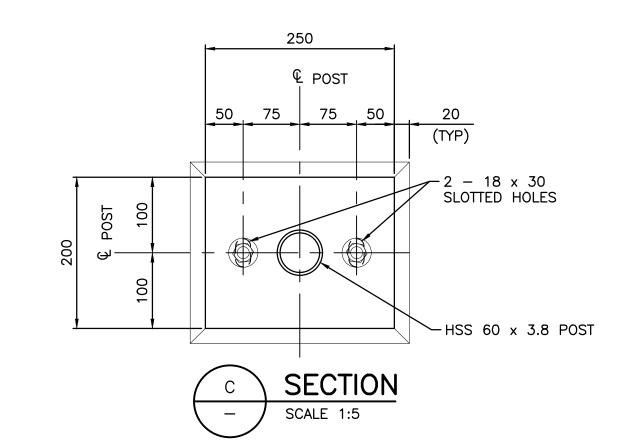
SCALE 1:20 (RAIL NOT SHOWN FOR CLARITY)
(SW AND NE SHOWN. NW AND SE OPPOSITE.)



SAFETY RAIL ELEVATION
SCALE 1:20







NOTES

- 1. POST SPACING SHOWN IS CORRECT AT 15°C. LOCATION OF THE POST ANCHOR ROD ASSEMBLIES SHALL BE ADJUSTED TO ACCOUNT FOR INSTALLATION TEMPERATURE.
- 2. ALL REQUIREMENTS OF GNWT SPECIFICATIONS FOR BRIDGE CONSTRUCTION (SSBC) SECTION 12 SHALL BE MET.
- 3. ALL STEEL SHALL CONFORM TO CSA SPECIFICATION G40.21M GRADE 300W. ANCHOR RODS SHALL CONFORM TO ASTM F1554 GRADE 55. ALL NUTS AND WASHERS SHALL CONFORM TO ASTM A563 AND ASTM F436 RESPECTIVELY.
- 4. ALL WELDING SHALL CONFORM TO CURRENT AWS SPECIFICATION D1.1 AND D1.5.
- 5. ALL MATERIALS SHALL BE HOT DIP GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123/A123M AND ASTM F2329 UNLESS NOTED OTHERWISE.
- 6. REPAIR OF GALVANIZED SHALL BE COMPLETED AS PER THE SSBC SECTION 6.2.7.3.3, REPAIR OF GALVANIZED AND METALLIZED MATERIAL.
- 7. ALL EXPOSED CUT TUBE ENDS SHALL BE GROUND SMOOTH.
- 8. THE BOTTOM SURFACE OF THE BASE PLATES SHALL BE COATED WITH CLOVATAR 22 OR AN APPROVED EQUIVALENT SUITABLE FOR APPLICATION ON GALVANIZED STEEL TO PREVENT CONTACT BETWEEN THE ZINC AND THE GROUT. THE COLOUR SHALL BE MEDIUM GREY.
- 9. ALL POSTS SHALL BE VERTICAL.
- 10. ALL ADDITIONAL HOLES PLACED FOR GALVANIZING PROCESS SHALL BE FILLED BY ALUMINUM PLUGS AFTER GALVANIZING.



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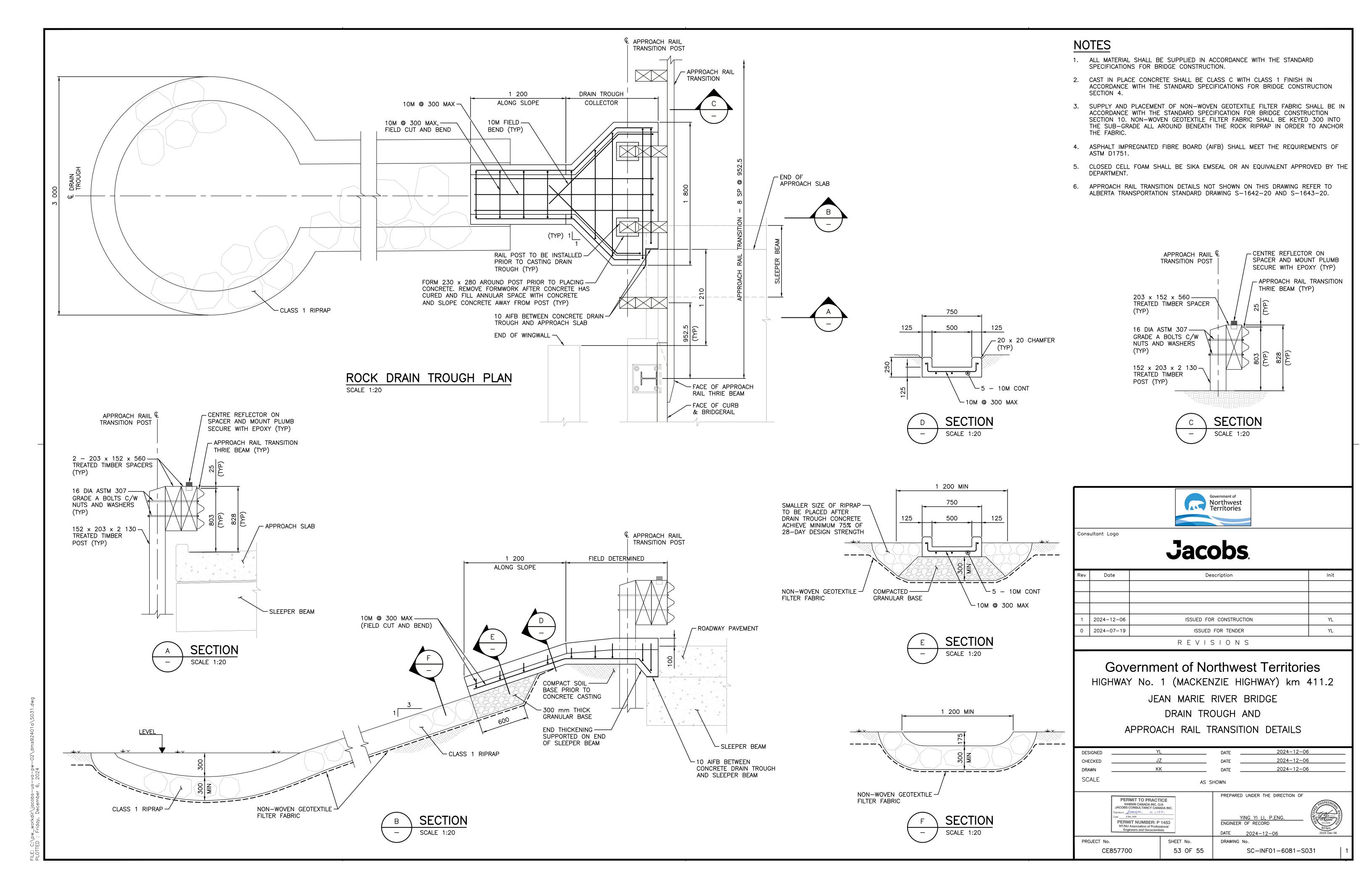
Rev	Date	Description	Init
1	2024-12-06	ISSUED FOR CONSTRUCTION	YL
0	2024-07-19	ISSUED FOR TENDER	YL
		REVISIONS	

Government of Northwest Territories
HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2

JEAN MARIE RIVER BRIDGE SAFETY RAIL DETAILS

DESIGNED	YL	DATE	2024-12-06
CHECKED	JZ	DATE	2024-12-06
DRAWN	KK	DATE	2024-12-06
SCALE	AS	SHOWN	

PERMIT TO PRACT 6449506 CANADA INC. O. JACOBS CONSULTANCY CANA Signature Assumption of Ly 3 Date 6 Dec. 2024 PERMIT NUMBER: P NT/NU Association of Profess Engineers and Geoscientis	ADA INC. ₹1 1453 sional	PREPARED UNDER THE DIRECTION OF YING YI LI, P.ENG. ENGINEER OF RECORD DATE 2024–12–06 PROFESSION 10.15794 10.15794 2024 Dec 06
PROJECT No.	SHEET No.	DRAWING No.
CE857700	52 OF 55	SC-INF01-6081-S030



BAR	LIST:	ABU	TMENT	Γ SEA	TS & SHE	AR BLOCKS									
MARK	SIZE	TYPE	x	Y	Z LENGTH	QTY IN 1 PRECAST UNIT IN ABUT 1 SE OR SW ELEMENT	QTY N 1 PRECAST UNIT ABUT 2 NE OR NW ELEMENT	. QTY IN 1 PRECAST UNIT MID ELEMENT ABUT 1 OR ABUT 2	LINITS	QTY FIELD PLACEMENT IN PILE POCKETS ABUT 1	QTY FIELD PLACEMENT IN PILE POCKETS ABUT 2	QTY FIELD PLACEMENT IN 4 CONNECTIONS	TOTAL QTY IN FIELD PLACEMENT	MASS IN 6 PRECAST UNITS (kg)	MASS IN FIELD PLACEMENT (k
A1501	15	Р	1 695		1 975	63	57	48	336			24	24	1 042	74
A1502	15	R	120	770	1 670 2 560	72	72	24	336					1 350	
A1503	15	G	322	1 220	2 762	36	24	12	144					624	
A1504	15	G	1 695	140	1 975	16	14	24	108					335	
A1505	15	G	1 655	200	2 055	4	4		16					52	
A1506	15	G	1 665	250	2 165	3	3		12					41	
A1507	15	Е	762	140	902						16		16		23
A1508	15	Е	720	140	860						16		16		22
A1509	15	G	1 756	140	2 036			16	32					102	
A1510	15	G	982	140	1 262		6		12					24	
A1511	15	G	1 024	140	1 304		6		12					25	
A1512	15	E	1 695	140	1 835					56	32		88		254
A2001	20	G	1 034	1 320	3 674	42	44	32	236			16	16	2 042	138
A2002	20	G	992	1 320	3 632	42	44	32	236			16	16	2 019	137
A2003	20	Р	1 695		2 095	36	48	12	192					947	
A2004	20	STR			4 315	14	14		56					569	
A2005	20	М	1 320		1 520		12		24					86	
A2006	20	Е	3 700	250	3 950	4	4		16					149	
A2007	20	STR			2 400			10	20					113	
A2008	20	STR			3 030			16	32					228	
A2009	20	STR			3 030			4	8					57	
A2501	25	E	4 315	400	4 715	8	8		32					592	
A2502	25	STR			4 315	8	8		32					542	
A2503	25	E	400	590	990	6	6		24					93	
A2504	25	E	510	400	910				0	8	8		16		57
A2505	25	STR			3 030			24	48					571	
A2506	25	STR			2 400			10	20					188	
A3001	30	STR			4 315	4	4		16					379	
A3002	30	STR			3 975	8	8		32					699	
A3003	30	Е	4 315	490	4 805	4	4		16					422	
A3005	30	STR			3 030			8	16					266	
A3006	30	STR			2 400			4	8					106	
H1501	15	G	470	800	2070			6	12					39	
H1502	15	G	370	800	1970			6	12					37	
H1503	15	Н	500	400	2080								8		26

PLAIN TOTAL IN 6 PRECAST ELEMENTS	(kg):	13 740
PLAIN TOTAL IN FIELD PLACEMENT	(kg):	731

BAR	LIST:	SLEE	PER	BEAM	S					
MARK	SIZE	TYPE	w	X	Y	Z	LENGTH	QTY IN 1 PRECAST UNIT	QTY IN 2 PRECAST UNITS	MASS (kg)
B1001	10	L		80	440 (AVE)	180	1 220	32	64	61
B1501	15	G		790	440 (AVE)		1 670	16	32	84
B1502	15	Н		790	450 (AVE)		2 760	45	90	390
B1503	15	G		790	500		1 790	8	16	45
B2001	20	Y		790			1 130	8	16	43
B2002	20	STR					1 700	8	16	64
B2003	20	STR					10 440	4	8	197
B2004	20	STR					10 440	2	4	98
B3001	30	J	5 220	10 440	104	104	10 440	4	8	459
B3002	30	STR					10 440	4	8	459

PLAIN TOTAL IN 2 PRECAST ELEMENTS (kg): 1 900

NOTE

- 1. BARS DENOTED AS 'SS' SHALL BE SOLID STAINLESS REINFORCING STEEL.
- 2. DIAMETERS OF ALL BENDS AND DETAILS OF ALL HOOKS, UNLESS NOTED OTHERWISE SHALL CONFORM TO THE RECOMMENDED SIZED DETAILED IN THE CURRENT EDITION OF THE REINFORCING STEEL STANDARD PRACTICE MANUAL PUBLISHED BY THE REINFORCING STEEL INSTITUTE OF CANADA.
- 3. WHERE SPLICES ARE SPECIFICALLY DETAILED ON THESE DRAWINGS THE MINIMUM SPLICE LENGTHS SHALL BE THE LONGER LENGTH SHOWN IN THE SPECIFIC DETAIL OR AS SUMMARIZED IN THE FOLLOWING TABLE. FOR ALL OTHER CASES APPROVAL IN WRITING FROM THE ENGINEER IS REQUIRED.

REINFORCING SPLICE LENGTHS (UNO)								
BAR SIZE SPLICE LENGTH								
10M	450							
15M	650							
20M	850							
25M	1300							
30M	1550							
35M	1800							

BAR	LIST:	ABUT	IMENT	DIAF	PHRAG	SMS							
MARK	SIZE	TYPE	V	W	х	Y	Z	LENGTH	QTY IN 1 PRECAST UNIT	QTY IN 2 PRECAST UNITS	TOTAL QTY IN FIELD PLACEMENT	MASS IN 2 PRECAST UNITS (kg)	MASS IN FIELD PLACEMENT (kg)
D1501	15	Р			1 422			1 702	24	48		128	
D2001	20	J		5 155	10 310	103	103	10 310	5	10		243	
D2002	20	Т	300	834	670	200	715	2 269	52	104		556	
D2003SS	20	S		300	160	750		2 260	12	24		128	
D2501	25	G			535	1 211		2 957			16		186
D2502	25	Н			585	1 422		4 574	53	106		1 903	
D2503	25	STR						6 000	10	20		471	
D2504	25	STR						3 200	10	20		251	
D2505	25	G			200	950		2 100	45	90		742	
D2506	25	С			400	95	750	1161	10	20		91	
D2507	25	J		3 900	7 800	78	78	7 800	6	12		367	
D2508	25	STR						4 600	12	24		433	
D2509	25	G			350	3 000		6 350	4	8		199	
D2510	25	STR						3 000	8	16		188	
D2511	25	G			535	3 000		6 535	18	36		923	
D2512	25	STR						3 500	14	28		385	
D2513	25	R			200	1 200	450	1 850			20		145
								•	•	PLAIN TOTAL I	N 2 PRECAST	ELEMENTS (kg):	6 881

PLAIN TOTAL IN 2 PRECAST ELEMENTS (kg): 6 881

SS TOTAL IN 2 PRECAST ELEMENTS (kg): 128

PLAIN TOTAL IN FIELD PLACEMENT (kg): 331

BAR	LIST:	ABU1	MENT	WIN	GWALL	_S					
MARK	SIZE	TYPE	W	X	Y	Z	LENGTH	QTY IN 1 PRECAST UNIT ABUT 1 SE OR SW ELEMENT	QTY IN 1 PRECAST UNIT ABUT 2 NE OR NW ELEMENT	TOTAL QTY IN 4 PRECAST UNITS	MASS IN 4 PRECAST UNITS (kg)
W1501	15	Р		585			865	6	6	24	33
W1502	15	Р		1 205			1 485	3	3	12	28
W1502	15	F	180	4 740	2 551	630	6 193	2		4	39
W1503	15	F	180	4 740	2 687	630	6 259		2	4	39
W1504	15	E		3 400 (AVE)	750		4 280	14	14	56	376
W1505	15	R		710	1 635	250	2 595	3	3	12	49
W1506	15	G		255	1 600 (AVE)		3 455	15	15	60	325
W1507	15	G		295	620		1 535	2	2	8	19
W1508	15	G		255	620		1 495	15	15	60	141
W1509	15	R		810	820	250	1 880	3	3	12	35
W2001	20	Н		535	832		3 134	2	2	8	59
W2002	20	Н		535	750		2 970	6	6	24	168
W2003	20	G		2 850	300		3 450	8	8	32	260
W2501	25	G		585	2 025		4 635	10	10	40	728
W2502	25	Н		535	1 210		4 290	8	8	32	539
W2503	25	Н		350	1 210		3 920	2	2	8	123
W2504	25	E		1 185	400		1 585	4	4	16	100
W2505	25	E		1 210	400		1 610	6	6	24	152
W2506	25	Н		585	1 742		4 150	1	1	4	65
W2507	25	E		3 400 (AVE)	400		3 800	14	14	56	835

PLAIN TOTAL IN 4 PRECAST ELEMENTS (kg): 4 113



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1	2024-12-06	ISSUED FOR CONSTRUCTION	YL			
0	2024-07-19	ISSUED FOR TENDER	YL			
	REVISIONS					

Government of Northwest Territories
HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2

JEAN MARIE RIVER BRIDGE BAR LIST

SHEET 1

DESIGNED	YL	. DATE	2024-12-06
CHECKED	JZ	DATE	2024-12-06
DRAWN	KK	DATE	2024-12-06
SCALE	46	SHOWN	
	24	>HOWN	

PERMIT TO PRACT 6449506 CANADA INC. O JACOBS CONSULTANCY CAN. Signature Activity and Control Land	/A ADA INC.	PREPARED UNDER THE DIRECTION OF				
PERMIT NUMBER: P NT/NU Association of Profes Engineers and Geoscienti	sional	YING YI LI, P.ENG. ENGINEER OF RECORD DATE 2024–12–06				
ROJECT No.	SHEET No.	DRAWING No.				
CE857700	54 OF 55	SC-INF01-6081-S032				

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BAR	LIST:	DECK	PAN	NELS										
MARK	SIZE	TYPE	٧	w	х	Y	Z	LENGTH	QTY IN 1 TYP DECK PANEL	QTY IN 1 END DECK PANEL	TOTAL QTY IN 14 PRECAST UNITS	TOTAL QTY IN FIELD PLACEMENT	MASS IN 14 PRECAST UNITS (kg)	MASS IN FIELD PLACEMENT (kg)
C1001	10	V			380	585	172	1 600	60	40	800		1 005	
C1501	15	STR						3 080	12		144		696	
C1502	15	U	44	250	362	350	340	1 652	30	32	424		1 100	
C1503	15	STR						3 080	100		1 200		5 803	
C1504	15	Х		5 450	10 880	109	109	11 380	22	24	312	13	5 574	232
C1505	15	STR						3 100		40	80		389	
C1506	15	STR						2 495		18	36		141	
C1507	15	STR						3 100		12	24		117	
C1508	15	Z			382	340	54	1 066				26		44
C2001	20	W		5 450	10 880	109	109	11 280	22	24	312	13	8 288	345
C2501	25	STR						3 100		20	40		487	

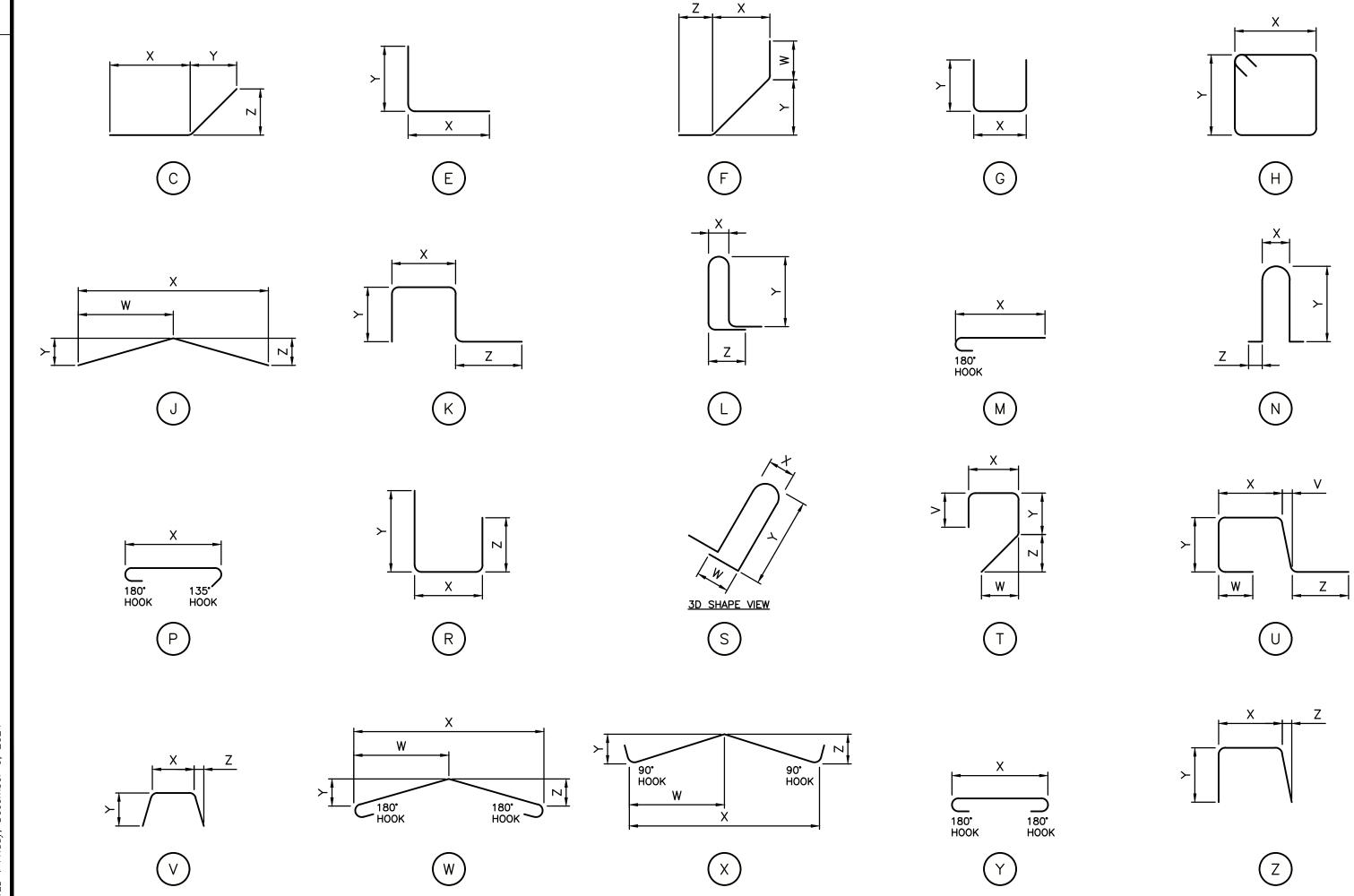
PLAIN TOTAL IN 14 PRECAST ELEMENTS (kg): 23 600
PLAIN TOTAL IN FIELD PLACEMENT (kg): 621

MARK	SIZE	TYPE	V	W	×	Y	Z	LENGTH	QTY IN 1 EXTERIOR ELEMENT	QTY IN 1 INTERIOR ELEMENT	TOTAL QTY IN 8 PRECAST ELEMENTS	TOTAL No. OF BARS FOR FIELD PLACEMENT	MASS IN PRECAST ALL ELEMENTS (kg)	MASS IN FIELD PLACEMENT (kg)
P1501	15	STR						4 775	2		8		60	()
P1502	15	U	50	250	362*	400	250	1 662	24		96		250	
P1503	15	STR						4 775	1		4		30	
P1504	15	STR						5 900	9	8	68	6	630	56
P1505	15	Е			300	300		600	6		24		23	
P1506	15	STR						5 625	3	3	24		212	
P1507	15	М			2 910			3 080	30		120		580	
P1508	15	G			210	300		810	18	16	136		173	
P1509	15	С			4 465	308	101	4 789	1		4		30	
P1510	15	STR						1 475	1		4		9	
P1511	15	М			2 675			2 845	3		12		54	
P1512	15	STR						2 750		18	72		311	
P1513	15	G			415	250		915	5		20		29	
P2001	20	М			2 910			3 110	2		8		59	
P2002	20	STR						2 910	32		128		877	
P2003	20	STR						2 675	7		28		176	
P2004	20	STR						2 750		41	164		1 062	
P3001	30	STR						4 775	2		8		210	
P3002	30	STR						5 900	12	10	88	6	2 853	195
P3003	30	STR						5 625	6	6	48		1 484	

NOTE: * ADJUST LAST 2 END BARS TO FIT CURB END CHAMFER

PLAIN TOTAL IN 8 PRECAST ELEMENTS (kg): 9 111

PLAIN TOTAL IN FIELD PLACEMENT (kg): 25





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1	2024-12-06	ISSUED FOR CONSTRUCTION	YL			
0	2024-07-19	ISSUED FOR TENDER	YL			
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Government of Northwest Territories

HIGHWAY No. 1 (MACKENZIE HIGHWAY) km 411.2

JEAN MARIE RIVER BRIDGE BAR LIST

SHEET 2

DESIGNED	YL	. DATE	2024-12-06
CHECKED	JZ	DATE	2024-12-06
DRAWN	KK	DATE	2024-12-06
SCALE	Δ0	SHOWN	

PERMIT TO PRACT 6449506 CANADA INC. O JACOBS CONSULTANCY CAN, Signature Atlanty and TD L 43	/A ADA INC.	PREPARED UNDER THE DIRECTION OF			
PERMIT NUMBER: P NT/NU Association of Profes Engineers and Geoscienti	sional	YING YI LI, P.ENG. ENGINEER OF RECORD DATE 2024–12–06			
ROJECT No.	SHEET No.	DRAWING No.			
CE857700	55 OF 55	SC-INF01-6081-S033			

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